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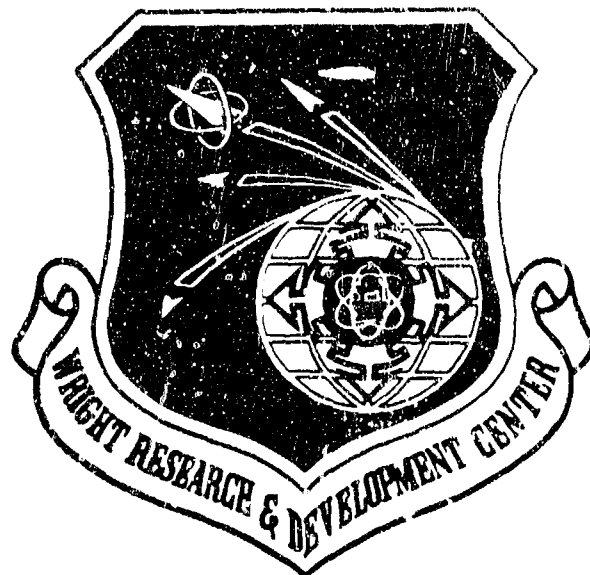
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## QUADA SEMINAR

Vipperla B. Venkayya  
Victoria A. Tischler

Analysis and Optimization Branch  
Structures Division

April 1989  
(Revised April 1993)



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FLIGHT DYNAMICS LABORATORY  
WRIGHT RESEARCH and DEVELOPMENT CENTER  
AIR FORCE SYSTEMS COMMAND  
WRIGHT-PATTERSON AIR FORCE BASE, OHIO 45433-6523

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## QUAD4 SEMINAR

Vipperla B. Venkayya

Victoria A. Tischler

Analysis and Optimization Branch  
Structures Division

April 1989

(Revised April 1993)



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This technical report has been reviewed and is approved for publication.

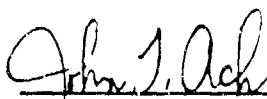


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# TABLE OF CONTENTS

|             | <u>TITLE</u>   | <u>PAGE</u> |
|-------------|--|-------------|
| 1.0         | INTRODUCTION   | 1           |
| 2.0         | SEMINAR VIEWGRAPHS   | 4           |
| 3.0         | THEORETICAL DEVELOPMENT OF THE QUAD4 ELEMENT   | 29          |
| APPENDIX A: | GUIDELINES FOR THE USE OF THE QUAD4 ELEMENT  | 66          |
| APPENDIX B: | SAMPLE PROBLEMS  | 80          |
|             | B.1 PROBLEM DESCRIPTIONS   | 82          |
|             | B.2 RESULTS AND COMPARISON   | 100         |
| APPENDIX C: | CASE CONTROL AND BULK DATA CARDS PERTAINING TO QUAD4 ELEMENT   | 115         |
| APPENDIX D: | NASTRAN INPUT DATA FOR SAMPLE PROBLEMS<br>1.6, 8, 9, 11, 13c   | 134         |
| APPENDIX E: | NASTRAN INPUT AND OUTPUT FOR SAMPLE PROBLEMS<br>7, 10, 12, 13a, 13b, 13d, 14, and 9 and 12<br>with TRIA3 | 171         |
| APPENDIX F: | NASTRAN INPUT AND OUTPUT FOR PROBLEM 15  | 248         |

## FOREWORD

This technical report is prepared for presentation at training seminars modeling plate elements in NASTRAN (NASA STructural Analysis Program). This training seminar is usually given at NASTRAN User Colloquiums or NASTRAN Applications Short Courses.

## INTRODUCTION

NASTRAN (NASA STRuctural ANalysis Program) is the most widely used general purpose structural analysis program in the world. This program was originally developed in the mid-sixties, and the first version was released in 1968.. The program developed was sponsored by the National Aeronautics and Space Administration (NASA). This development in the form of enhancements and maintenance was continued until 1972 as a single Government version under the sponsorship of NASA. Since then the Government version is being developed as COSMIC-NASTRAN, while a commercial version is being marketed by McNeal and Swendler Corporation (MSC) as MSC-NASTRAN. Even though the basic structure of these two versions remains the same, there are significant differences in capabilities and efficiencies. Around 1980 the MSC introduced a new plate bending element called the QUAD4 to its element library. It is one of the most versatile plate bending elements, even though its theoretical basis is somewhat controversial. The basic strength of this element is that it was subjected to exclusive numerical testing, and a number of empirical adjustments were made to conform the results to known solutions. The QUAD4 element embodies a number of improvements over the earlier elements (QDMEM, QDMEM1, QDMEM2, QDELT, Q1AD1 and Q1AD2).

- a. It is an isoparametric formulation.
- b. It models inplane (membrane) behavior more realistically.
- c. The layered composites modeling capability is extensive.
- d. Membrane-bending coupling can be modeled realistically.
- e. It is the only plate element with an offset feature.
- f. It is a convenient element for modeling laminate plates.
- g. The same element can be used in modeling sandwich plates; even though it is not as simple to model sandwich plates with composite face sheets.
- h. A single plate element replaced all other elements.

Until around 1987, COSMIC-NASTRAN did not have a similar (QUAD4) capability. The absence of this capability represented a significant inconvenience, in particular, for modeling layered composites. Between 1983-1986 a QUAD4 element was developed under the sponsorship of the Air Force (Flight Dynamics Laboratory) for use in the program ASTROS (Automated Structural Optimization System). At the same time this element was incorporated into COSMIC-NASTRAN. The ASTROS-QUAD4 element is similar to the MSC-QUAD4, but there are significant differences in the theoretical formulation between the two elements.

This report provides an informal background for modeling with the QUAD4 element. It contains over 15 problems to illustrate various options of the element. However, the report was intended only as background material to be used in conjunction with a one-day short course. The section on the theoretical formulation was

reproduced verbatim from one of the interim reports of the ASTROS contract.

## 2.0 SEMINAR VIEWGRAPHS

## QUAD4 ELEMENT - PURPOSE

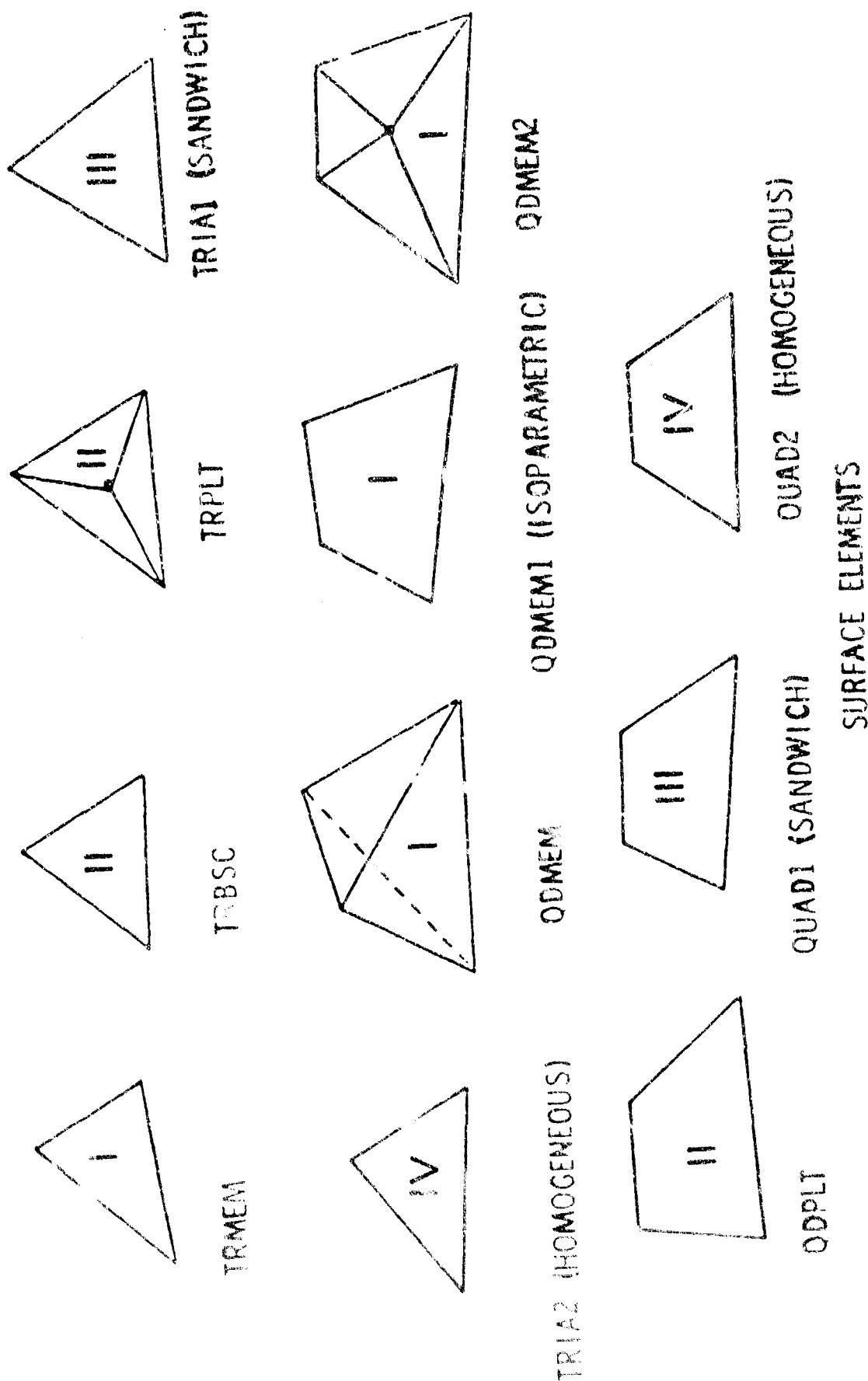
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- MODEL MEMBRANE BENDING PROBLEMS
- MODEL COMPOSITE PLATES
- MODEL MEMBRANE BENDING COUPLING
- MODEL SANDWICH PLATES IN MEMBRANE BENDING
- REPLACES QDMEM, QDMEM1, QDMEM2, QDPLT, QUAD1, QUAD2, SHEAR



# NASTRAN OVERVIEW

## STRUCTURAL ELEMENTS IN NASTRAN PLATE ELEMENTS (II)



## QUAD4 ELEMENT - FEATURES

---

- ISOPARAMETRIC ELEMENT
- BILINEAR VARIATION OF
  - GEOMETRY
  - DEFORMATION
- PROVIDES STIFFNESS FOR 5 DEGREES OF FREEDOM
- TRANSVERSE SHEAR FLEXIBILITY
- INTERLAMINAR SHEAR STRESSES
- FIVE FAILURE THEORIES

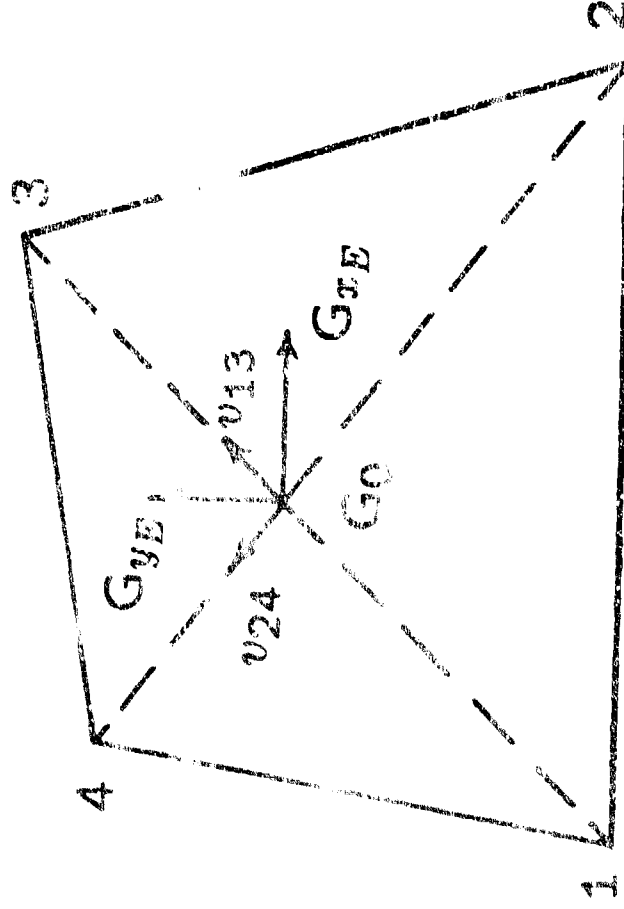
## QUAD4 ELEMENT - FEATURES

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- ALLOWS MODELING
  - THIN PLATES
  - THICK PLATES
  - HIGH ASPECT RATIO ELEMENTS
  - SKEWED ELEMENTS
  - OFF SET ELEMENTS
- NUMERICAL MODELING OF OUT-OF-PLANE SHEAR STRAINS
  - LUMPED MASS MODELS
  - CONSISTENT MASS MODELS (PLANNED)
  - DIFFERENTIAL STIFFNESS (PLANNED)

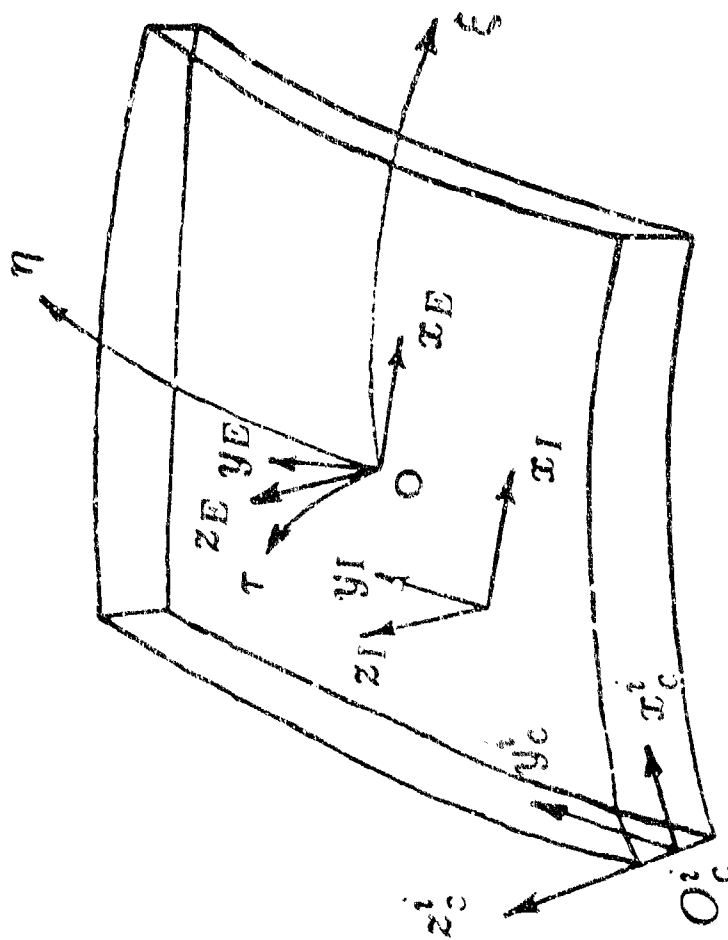
## QUAD4 ELEMENT - THEORETICAL OUTLINE

- TWO ELEMENT COORDINATE SYSTEMS
  - DEFINED BY ELEMENT CONNECTIVITY
  - INTERNAL ELEMENT COORDINATE SYSTEM
- FOR STIFFNESS FORMULATION



Internal Element Coordinate System

# QUAD4 ELEMENT - THEORETICAL OUTLINE



Isoparametric Quadrilateral 4-Node Plate & Shell Element

## QUAD4 ELEMENT - SHAPE FUNCTIONS

---

$$\tilde{x}_E(\xi, \eta) = N_1 \tilde{x}^1 + N_2 \tilde{x}^2 + N_3 \tilde{x}^3 + N_4 \tilde{x}^4$$

$$\tilde{u}_E(\xi, \eta) = N_1 \tilde{u}^1 + N_2 \tilde{u}^2 + N_3 \tilde{u}^3 + N_4 \tilde{u}^4$$

$N_i \Rightarrow N_i(\xi, \eta) \Rightarrow$  SHAPE FUNCTIONS

$\tilde{x}_i^i \Rightarrow$  GRID POINT COORDINATES

$$[x_i, y_i, z_i]^t$$

$\tilde{u}_i^i \Rightarrow$  GRID POINT DISPLACEMENTS

$$[u_i, v_i, w_i, \theta_{xi}, \theta_{yi}, \theta_{zi}]^t$$

## QUAD4 ELEMENT - SHAPE FUNCTIONS

---

$$N_i = \frac{1}{4}(1 + \xi\xi_i)(1 + \eta\eta_i)$$

$$\frac{\partial N_i}{\partial \xi} = \frac{1}{4}\xi_i(1 + \eta\eta_i)$$

$$\frac{\partial N_i}{\partial \eta} = \frac{1}{4}\eta_i(1 + \xi\xi_i)$$

# QUAD4 - STRAIN DISPLACEMENT RELATIONS

## • LINEAR STRAIN - DISPLACEMENT RELATIONS

$$\{e\} = \begin{pmatrix} \epsilon_x \\ \epsilon_y \\ \gamma_{xy} \\ \gamma_{yz} \\ \gamma_{zx} \end{pmatrix} = \begin{pmatrix} \frac{\partial u}{\partial x} & \frac{\partial u}{\partial y} & \frac{\partial v}{\partial x} & \frac{\partial v}{\partial y} & \frac{\partial v}{\partial z} \\ \frac{\partial u}{\partial y} & \frac{\partial u}{\partial x} & \frac{\partial w}{\partial y} & \frac{\partial w}{\partial x} & \frac{\partial w}{\partial z} \end{pmatrix}$$



# QUAD4 - STRAIN DISPLACEMENT RELATIONS

## MEMBRANE - BENDING STRAIN - DISPLACEMENT RELATIONS

$$\begin{pmatrix} \epsilon_M \\ \gamma_{TS} \\ \epsilon_B \end{pmatrix}_I = \begin{pmatrix} \frac{\partial N_i}{\partial x} & 0 & 0 & 0 \\ 0 & \frac{\partial N_i}{\partial y} & 0 & 0 \\ \frac{\partial N_i}{\partial y} & \frac{\partial N_i}{\partial x} & 0 & 0 \\ \hline \frac{\partial N_i}{\partial x} & 0 & \frac{\partial N_i}{\partial y} & 0 \\ 0 & \frac{\partial N_i}{\partial y} & \frac{\partial N_i}{\partial x} & 0 \\ \hline 0 & 0 & \frac{\partial N_i}{\partial y} & \frac{\partial N_i}{\partial x} \\ 0 & 0 & \frac{\partial N_i}{\partial x} & \frac{\partial N_i}{\partial y} \end{pmatrix} \begin{pmatrix} u_M \\ v_M \\ w_M \\ u_B \\ v_B \\ w_B \end{pmatrix}_I$$

## QUAD4 - STRAIN DISPLACEMENT RELATIONS

### • RELATION BETWEEN:

$$[u_M \ v_M \ w_M \ u_B \ v_B \ w_B] \quad \text{and} \quad [u \ v \ w \ \theta_x \ \theta_y \ \theta_z]$$

$$\begin{pmatrix} u_M \\ v_M \\ w_M \\ u_B \\ v_B \\ w_B \end{pmatrix}_I = \begin{pmatrix} T & 0 \\ 0 & \frac{Jt_A}{2} \end{pmatrix} \begin{pmatrix} u \\ v \\ w \\ \theta_x \\ \theta_y \\ \theta_z \end{pmatrix}_E$$

### • JACOBIAN (TRANSFORMATION FROM $\tilde{x}$ TO $\xi$ )

$$\begin{pmatrix} \frac{\partial N_i}{\partial \xi} \\ \frac{\partial N_i}{\partial \eta} \\ \frac{\partial N_i}{\partial \zeta} \end{pmatrix} = \begin{pmatrix} \frac{\partial x}{\partial \xi} & \frac{\partial y}{\partial \xi} & \frac{\partial z}{\partial \xi} \\ \frac{\partial x}{\partial \eta} & \frac{\partial y}{\partial \eta} & \frac{\partial z}{\partial \eta} \\ \frac{\partial x}{\partial \zeta} & \frac{\partial y}{\partial \zeta} & \frac{\partial z}{\partial \zeta} \end{pmatrix} \begin{pmatrix} \frac{\partial N_i}{\partial \tilde{x}} \\ \frac{\partial N_i}{\partial \tilde{y}} \\ \frac{\partial N_i}{\partial \tilde{z}} \end{pmatrix}$$

# QUAD4 ELEMENT - STRESS - STRAIN RELATIONS

\* STRESS AND STRAIN ARE RELATED BY:

$$\begin{pmatrix} \sigma_M \\ \sigma_B \\ \tau_{TS} \end{pmatrix} = \begin{pmatrix} G_1 & 0 & 0 \\ 0 & G_2 & 0 \\ 0 & 0 & G_3 \end{pmatrix} \begin{pmatrix} \epsilon_M \\ \epsilon_B \\ \gamma_{TS} \end{pmatrix} - \begin{pmatrix} \epsilon_M \\ \epsilon_B \\ 0 \end{pmatrix}_T$$

or

$$[\sigma]_i = [G]_i (\{\epsilon\}_{MEC} - \{\epsilon\}_T)_i$$

## QUAD4 ELEMENT - STRESS - STRAIN RELATIONS

---

where

- $\{\sigma_M\}$  - Membrane stress vector
- $\{\sigma_B\}$  - Bending stress vector
- $\{\tau_{TS}\}$  - Transverse shear stress vector
- $\{G_1\}$  - Membrane moduli matrix
- $\{G_2\}$  - Bending moduli matrix
- $\{G_3\}$  - Transverse shear moduli matrix
- $\{e_M\}$  - Membrane strain vector
- $\{e_B\}$  - Bending strain vector
- $\{\tau_{TS}\}$  - Transverse shear strain vector

# QUAD4 ELEMENT - STRESS - STRAIN RELATIONS

---

- MATERIAL CONSTANTS TRANSFORMATION

$$[G]_I = [U]^T [G]_M [U]$$



# FAILURE THEORIES

---

- 1) HILL
- 2) HOFFMAN
- 3) TSAI-WU
- 4) MAXIMUM STRESS
- 5) MAXIMUM STRAIN

## FAILURE THEORIES

$\sigma_t$  = Ultimate uniaxial tensile strength in the fiber direction  
 $\sigma_c$  = Ultimate uniaxial compressive strength in the fiber direction  
 $\eta$  = Ultimate uniaxial tensile strength perpendicular to the fiber direction

$\eta_c$  = Ultimate uniaxial compressive strength perpendicular to the fiber direction

$\tau$  = Ultimate planar shear strength under pure shear loading

$E_t$  = Ultimate uniaxial tensile strain in the fiber direction

$E_c$  = Ultimate uniaxial compressive strain in the fiber direction

$\eta_t$  = Ultimate uniaxial tensile strain perpendicular to the fiber direction

$E_c$  = Ultimate uniaxial compressive strain perpendicular to the fiber direction

$\bar{E}_s$  = Ultimate planar shear strain under pure shear loading



# FAILURE THEORIES

---

## HILLS'S THEORY

$$\frac{\sigma_1^2}{x_2} + \frac{\sigma_2^2}{y_2} - \frac{\sigma_1 \sigma_2}{x_2} + \frac{\tau_{12}^2}{s^2} = \text{FAILURE INDEX (FI)}$$

$x = x_t$  if  $\sigma_1 > 0$  ,  $x_c$  if  $\sigma_1 < 0$  ; similarly for  $y$

For the interaction term:

$$\frac{\sigma_1 \sigma_2}{s^2} , \quad x = x_t \text{ if } \sigma_1 \sigma_2 > 0 \\ x = x_c \text{ otherwise}$$

# FAILURE THEORIES

## HOFFMAN'S THEORY

$$\left(\frac{1}{x_t} - \frac{1}{x_c}\right)\sigma_1 + \left(\frac{1}{y_t} - \frac{1}{y_c}\right)\sigma_2 + \frac{\sigma_1^2}{x_t x_c} + \frac{\sigma_2^2}{y_t y_c} + \frac{\tau_{12}^2}{s^2} + \frac{\sigma_1 \sigma_2}{x_t x_c} = FI$$

Note that this theory takes into account the difference in the tensile and compressive allowable stresses by using linear terms in the failure equation.

# FAILURE THEORIES

## TSAI-WU THEORY

$$F_{10}\sigma_1 + F_{20}\sigma_2 + F_{11}\sigma_1^2 + F_{22}\sigma_2^2 + 2F_{12}\sigma_1\sigma_2 + F_{66}\tau_{12}^2 = FI$$

$$F_1 = \frac{1}{x_t} - \frac{1}{x_c}, \quad F_2 = \frac{1}{y_t} - \frac{1}{y_c},$$

$$F_{11} = \frac{1}{x_t x_c}, \quad F_{22} = \frac{1}{y_t y_c}, \quad F_{66} = \frac{1}{s^2}$$

$F_{12}$  needs to be determined experimentally from a biaxial test. However, satisfactory results may be obtained by setting it to zero.

Continued, Apr 21, 1988

Frame 6

# FAILURE THEORIES

## MAXIMUM STRESS

$$\sigma_1 \geq x_t, \quad \sigma_1 > 0; \quad \sigma_1 \leq -x_c, \quad \sigma_1 < 0$$

$$\sigma_2 \geq y_t, \quad \sigma_2 > 0; \quad \sigma_2 \leq -y_c, \quad \sigma_2 < 0$$

$$\tau_{12} \geq s, \quad \tau_{12} > 0; \quad \tau_{12} \leq s, \quad \tau_{12} < 0$$

# FAILURE THEORIES

---

## MAXIMUM STRAIN

$$\epsilon_1 \geq E_t, \quad \epsilon_1 > 0; \quad \epsilon_1 \leq E_c, \quad \epsilon_1 < 0$$

$$\epsilon_2 \geq F_t, \quad \epsilon_2 > 0; \quad \epsilon_2 \leq F_c, \quad \epsilon_2 < 0$$

$$\gamma_{12} \geq E_s, \quad \gamma_{12} > 0; \quad \gamma_{12} \leq E_s, \quad \gamma_{12} < 0$$

# SHEAR REDUCTION FACTORS

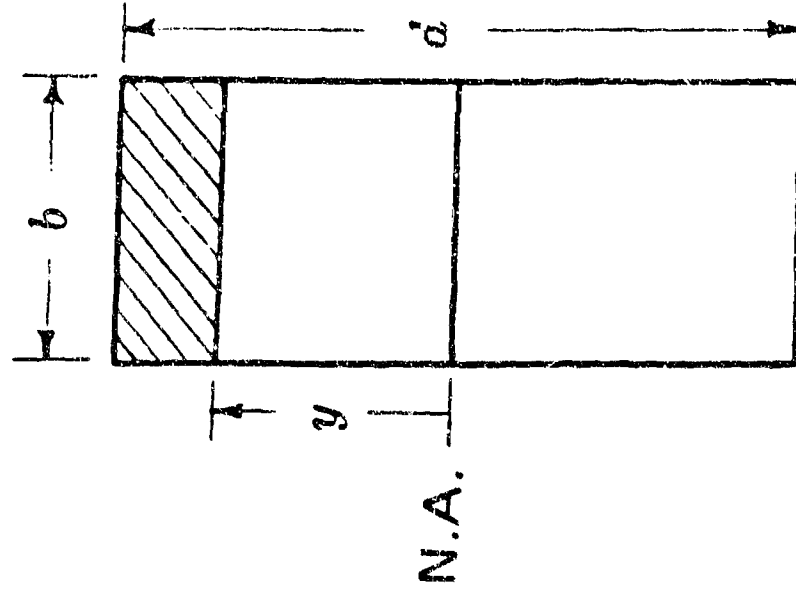
## 1. RECTANGULAR SECTION

$$I = \frac{bd^3}{12} \quad Q_y = \frac{b}{2} \left( \frac{d^2}{4} - y^2 \right)$$

$$\tau_y = \frac{vQ_y}{Ib} = \frac{v}{2I} \left( \frac{d^2}{4} - y^2 \right)$$

$v \Rightarrow$  Shear

$$\gamma_y = \frac{v}{2GI} \left( \frac{d^2}{4} - y^2 \right)$$



## SHEAR REDUCTION FACTORS

---

Strain energy density at a distance  $y$

$$= \frac{1}{2} \tau_y \gamma_y = \frac{1}{24GI^2} \left( \frac{d^2}{4} - y^2 \right)^2$$

$$\text{TOTAL STRAIN ENERGY} = \frac{v^2}{8GI^2} \int_{-d/2}^{d/2} \left( \frac{d^4}{16} - \frac{d^2 y^2}{2} + y^4 \right) dy$$

Carrying out the integration

$$\text{TOTAL STRAIN ENERGY} = \frac{1}{2} \frac{v^2}{G(5/6)A} = \frac{1}{2} \frac{v^2}{G(\alpha A)}$$

$$\text{where } \alpha = \frac{5}{6} = 0.8333333$$

### 3.0 THEORETICAL DEVELOPMENT OF THE QUAD4 ELEMENT



## APPENDIX A

### THE QUAD4 ELEMENT

This appendix provides the theoretical development for the QUAD4 element that has been installed into ASTROS. An overview of this element is given in Subsection 5.3.3, while this appendix provides detailed information on the element. This detail is necessary because, unlike the other elements, the ASTROS QUAD4 element has not been documented elsewhere.

#### A.1 DISPLACEMENT FUNCTIONS

The QUAD4 element has two distinct element coordinate systems. These are the "user defined" element coordinate system as defined by the element connectivity data and the "internal element" coordinate system, which is defined as having its origin at  $G_0 (X^0_E, Y^0_E, Z^0_E)$ . This origin is computed by taking the average of the grid point coordinates. The positive X- and Y-axes of the internal element coordinate system are defined with the aid of two points,  $G_{xE}$  and  $G_{yE}$  described below.

$\vec{V}_{13}$  and  $\vec{V}_{24}$  are defined as the unit diagonal vectors as illustrated in Figure A-1. Thus, the coordinates of points  $G_{xE}$  and  $G_{yE}$  are given by the following:

$$\begin{aligned} G_{xE} &= ((X^0_E + X'_E), (Y^0_E + Y'_E), (Z^0_E)) \\ G_{yE} &= ((X^0_E - Y'_E), (Y^0_E + X'_E), (Z^0_E)) \end{aligned} \tag{A-1}$$

where,  $X^0_E$ ,  $Y^0_E$  and  $Z^0_E$  are the coordinates of the origins of the internal coordinate system and  $X'_E$  and  $Y'_E$  are the components of the bisector vector of the unit diagonals  $V_{13}$  and  $V_{24}$ .

The coordinates of points  $G_0$ ,  $G_{xE}$  and  $G_{yE}$ , are used to define the transformation from the internal element coordinate system to the coordinate system in which the grid points are defined. The internal element coordinate system is necessary to correctly handle irregular-shaped and non-planar elements and is henceforth referred to as the "element" (E) coordinate system.

Using 2-D interpolation functions, the geometry field at any point  $(\xi, \eta)$  in the element cross-section (see Figure A-2) is defined, where the nodal curvilinear coordinates are related to the nodal cartesian coordinates system in the element coordinate system by the following relationship:

$$\{X_E(\xi, \eta)\} = \sum_{i=1}^4 N_i(\xi, \eta) \{X_E^i\}$$

where  $i$  refers to grid point  $i$ , and

$$\{X_E^i\} = (X_E, Y_E, Z_E) \text{ at node } i,$$

$N(\xi, \eta)$  are the interpolation (shape) functions which define the contribution of each node at a given point with the element. These functions and their derivatives are:

$$N_i = 1/4(1 + \xi\xi_i)(1 + \eta\eta_i)$$

$$\frac{\partial N_i}{\partial \eta} = \frac{1}{4}\xi_i(1 + \eta\eta_i) \quad (A-2)$$

The deformations of the element are also represented with the identical interpolation functions:

$$\{U_E(\xi, \eta)\} = \sum_{i=1}^4 N_i(\xi, \eta) \{U_E^i\} \quad (A-3)$$

where  $\{U_E^i\} = (U_E, V_E, W_E, \theta_{x_E}, \theta_{y_E}, \theta_{z_E})^T$  represents the vector of displacements at grid point  $i$  in the element coordinate system.

## A.2 STRAIN-DISPLACEMENT RELATIONSHIP

The QUAD4 element incorporates a reduced solid theory for thick shells. According to this theory, the element has five dof at each grid, defined in a coordinate system whose X-Y plane is tangent to the mid-surface of the shell at the given grid point. The z-axis, therefore, is the normal to mid-surface at that point. In our nomenclature, this is called the "C" system (Figure A-2 and A-3).

A generalization of the "C" system, called "I" system, incorporates the characteristics of the "C" system at a general point on the mid-surface of the shell element, normally the integration point (Figure A-2).

In order to establish a common definition for "I" and "C" systems, consider the following steps:

(A) The tangents to mid-surface at a given point  $(\xi, \eta)$  are:

$$\{V_{t1}\} = \frac{\partial \begin{Bmatrix} x \\ y \\ z \end{Bmatrix}_E}{\partial \xi} = \sum_{i=1}^4 \frac{\partial N_i}{\partial \xi} \begin{Bmatrix} x \\ y \\ z \end{Bmatrix}_E^i \quad (A-4)$$

$$\{V_{t2}\} = \frac{\partial \begin{Bmatrix} x \\ y \\ z \end{Bmatrix}_E}{\partial \eta} = \sum_{i=1}^4 \frac{\partial N_i}{\partial \eta} \begin{Bmatrix} x \\ y \\ z \end{Bmatrix}_E^i \quad (A-5)$$

where

$\begin{Bmatrix} x \\ y \\ z \end{Bmatrix}_E^i$  are the coordinates of grid points in "E" system.

(B) The axes of the new system then follow:

$$\{Z\}_{I/C} = \{V_n\} = \frac{\{V_{t1}\} \times \{V_{t2}\}}{|\{V_{t1}\} \times \{V_{t2}\}|}$$

$$\{X\}_{I/C} = \frac{\{Y\}_E \times \{Z\}_{I/C}}{|\{Y\}_E \times \{Z\}_{I/C}|} \quad (A-6)$$

$$\{Y\}_{I/C} = \{Z\}_{I/C} \times \{X\}_{I/C}$$

(C) Finally:

$$[TIE] = [(\{X\}_I \{Y\}_I \{Z\}_I)^T] \quad (A-7)$$

$$[TCE]^i = [(\{X\}_c^i \{Y\}_c^i \{Z\}_c^i)^T] \quad (A-8)$$

Note that the "C" system is not necessarily invariant when we go from one grid to the next. This is due to the possible warping of the element.

Since the ultimate goal of this discussion is to establish a relationship between the element strains (which are defined in the "I" system), and the nodal displacements (defined in the "E" system), it is necessary to develop a series of transformations along with the strain-displacement relationships.

Consider the five dof's in the "C" system at each grid point "i" to be arranged in the following manner (Figure A-3):

$$(U)_c^i = \begin{Bmatrix} u \\ v \\ w \end{Bmatrix}_c^i ; \quad (\theta)_c^i = \begin{Bmatrix} \alpha \\ \beta \end{Bmatrix}_c^i \quad (A-9)$$

In order to be compatible with the other dof's in the model, these are related to the six dof at that grid point, defined in the "E" system, by the relationship:

$$(U)_c^i = [TCE]^i (U)_E^i$$

$$(\theta)_c^i = \begin{bmatrix} 0 & 1 & 0 \\ -1 & 0 & 0 \end{bmatrix} [TCE]^i (\theta)_E^i \quad (A-10)$$

The extra transformation in the rotational case is a result of the difference in the definition of rotations for "C" and "E" systems (Figures A-3 and A-4).

The same five dof's are related to six dof's in the "I" system by using the transformations developed in Equations A-7 and A-8. Considering Equation A-10 and A-3:

$$(U)_I = [TIE] \sum_{i=1}^4 N_i [(TCE)^i]^T (U)_c^i = [TIE] \sum_{i=1}^4 N_i (U)_E^i$$

$$= \sum_{i=1}^4 N_i [T] (U)_E^i \quad (A-11)$$

and

$$(\theta)_I = [TIE] \sum_{i=1}^4 N_i [(TCE)^i]^T (\theta)_c^i = [TIE] \sum_{i=1}^4 N_i [(TCE)^i]^T$$

$$\begin{bmatrix} 0 & 1 & 0 \\ -1 & 0 & 0 \end{bmatrix} [TCE]^i (\theta)_E^i = \sum_{i=1}^4 N_i [A]^i (\theta)_E^i \quad (A-12)$$

Note that while [T] is invariant, [A] depends on the direction of the normal to mid-surface at each grid point.

At a point along the Z-axis of "I" system, at a level of  $Z = \zeta t_I/2$ , where,

$$t_I = \sum_{i=1}^4 N_i t_i$$

is the thickness of the element evaluated at this particular integration point, the dof's in "I" system may be written in the following form:

$$(u_M)_I = (u)_I ; (u_B)_I = \zeta t_I/2 (\theta)_I \quad (A-13)$$

The strain-displacement relationships can now be developed, using these rearranged dof's:

$$(\epsilon_M)_I = \begin{Bmatrix} \epsilon_x \\ \epsilon_y \\ \gamma_{xy} \end{Bmatrix}_I = \begin{Bmatrix} \partial u / \partial x \\ \partial v / \partial y \\ \partial u / \partial y + \partial v / \partial x \end{Bmatrix}_I = \begin{bmatrix} \partial / \partial x & 0 & 0 \\ 0 & \partial / \partial y & 0 \\ \partial / \partial y & \partial / \partial x & 0 \end{bmatrix} (u_M)_I \quad (A-14)$$

$$(\epsilon_B)_I = \begin{Bmatrix} \epsilon_x \\ \epsilon_y \\ \gamma_{xy} \end{Bmatrix}_I = \begin{Bmatrix} \partial u / \partial x \\ \partial v / \partial y \\ \partial u / \partial y + \partial v / \partial x \end{Bmatrix}_I = \begin{bmatrix} \partial / \partial x & 0 & 0 \\ 0 & \partial / \partial y & 0 \\ \partial / \partial y & \partial / \partial x & 0 \end{bmatrix} (u_B)_I \quad (A-15)$$

$$(\gamma_S)_I = \begin{Bmatrix} \gamma_{yz} \\ \gamma_{zx} \end{Bmatrix}_I = \begin{Bmatrix} \partial w / \partial y + \partial v / \partial z \\ \partial w / \partial x + \partial u / \partial z \end{Bmatrix}_I = \begin{bmatrix} 0 & 0 & \partial / \partial y & 0 & \partial / \partial z & 0 \\ 0 & 0 & \partial / \partial x & \partial / \partial z & 0 & 0 \end{bmatrix} \begin{Bmatrix} u_M \\ u_B \end{Bmatrix}_I \quad (A-16)$$

Inserting Equations A-11 through A-13 into Equations A-14 through A-16, and considering the following:

$$\frac{\partial}{\partial z} (u_B)_I = \frac{\partial}{\partial z} z (\theta)_I = (\theta)_I$$

and

(A-17)

$$\begin{Bmatrix} \partial / \partial x \\ \partial / \partial y \\ 1 \end{Bmatrix} = \sum_{i=1}^4 \begin{Bmatrix} \partial N_i / \partial x \\ \partial N_i / \partial y \\ N_i \end{Bmatrix}$$

we arrive at the following general relationships:

$$(\epsilon_M)_I = \sum_{i=1}^4 \begin{bmatrix} \partial N_i / \partial x & 0 & 0 \\ 0 & \partial N_i / \partial y & 0 \\ \partial N_i / \partial y & \partial N_i / \partial x & 0 \end{bmatrix} [T] (U)_E^I \quad (A-18)$$

$$(\epsilon_B)_I = \frac{\epsilon_I}{2} \sum_{i=1}^4 \begin{bmatrix} \partial N_i / \partial x & 0 & 0 \\ 0 & \partial N_i / \partial y & 0 \\ \partial N_i / \partial y & \partial N_i / \partial x & 0 \end{bmatrix} [A]^i \{\theta\}_E^i \quad (A-19)$$

$$(\gamma_S)_I = \sum_{i=1}^4 \begin{bmatrix} 0 & 0 & \partial N_i / \partial y & 0 & N_i & 0 \\ 0 & 0 & \partial N_i / \partial x & N_i & 0 & 0 \end{bmatrix} \begin{bmatrix} [T] & 0 \\ 0 & [A]^i \end{bmatrix} \begin{Bmatrix} U \\ \theta \end{Bmatrix}_E^i \quad (A-20)$$

or, collectively:

$$(\epsilon)_I = \begin{Bmatrix} \epsilon_M \\ \vdots \\ \epsilon_B \\ \vdots \\ \gamma_S \end{Bmatrix}_I = \begin{bmatrix} \partial N_1 / \partial x & 0 & 0 & & & \\ 0 & \partial N_1 / \partial y & 0 & & & \\ \partial N_1 / \partial y & \partial N_1 / \partial x & 0 & & & \\ \vdots & \vdots & \vdots & \ddots & \vdots & \vdots \\ 0 & 0 & \partial N_i / \partial y & 0 & N_i & 0 \\ 0 & 0 & \partial N_i / \partial x & N_i & 0 & 0 \end{bmatrix} \begin{bmatrix} \frac{\epsilon_I}{2} \begin{bmatrix} \partial N_1 / \partial x & 0 & 0 \\ 0 & \partial N_1 / \partial y & 0 \\ \partial N_1 / \partial y & \partial N_1 / \partial x & 0 \end{bmatrix} \\ \vdots \\ 0 & [A]^i \end{bmatrix} \begin{bmatrix} [T] & 0 \\ 0 & [A]^i \end{bmatrix} \begin{Bmatrix} u \\ v \\ w \\ \theta_x \\ \theta_y \\ \theta_z \end{Bmatrix}_E^i \quad (A-21)$$

Since the shape functions  $N_i$  are defined in terms of the curvilinear coordinates  $(\xi, \eta)$ , the shape function derivatives are related to the corresponding Cartesian derivatives in the element [E] coordinate system, by using the rules of partial differentiation, as:

$$\begin{Bmatrix} \partial N_i / \partial \xi \\ \partial N_i / \partial \eta \\ \partial N_i / \partial \zeta \end{Bmatrix} = \begin{bmatrix} \partial x / \partial \xi & \partial y / \partial \xi & \partial z / \partial \xi \\ \partial x / \partial \eta & \partial y / \partial \eta & \partial z / \partial \eta \\ \partial x / \partial \zeta & \partial y / \partial \zeta & \partial z / \partial \zeta \end{bmatrix} \begin{Bmatrix} \partial N_i / \partial x \\ \partial N_i / \partial y \\ \partial N_i / \partial z \end{Bmatrix} \quad (A-22)$$

The first and second rows of the transformation matrix (or Jacobian matrix [J]) are the tangent vectors to the surface  $r = \text{constant}$  and the third row is the interpolated values of the nodal normals. (Note the nodal normals are evaluated by carrying out the cross product of the two tangent vectors at the node point.)

From Equation A-7 the coordinates in the "I" system are related to the coordinates in the "E" system by the following:

$$\{U\}_I = [TIE]\{U\}_E$$

Therefore, the derivatives are given by:

$$\begin{Bmatrix} \partial N_1 / \partial x \\ \partial N_1 / \partial y \\ \partial N_1 / \partial z \end{Bmatrix} = [\phi] \begin{Bmatrix} \partial N_1 / \partial \xi \\ \partial N_1 / \partial \eta \\ \partial N_1 / \partial \zeta \end{Bmatrix}$$

where

(A-23)

$$[\phi] = [TIE] [J]^{-1} = \begin{bmatrix} \phi_{11} & \phi_{12} & 0 \\ \phi_{21} & \phi_{22} & 0 \\ \phi_{31} & \phi_{32} & \phi_{33} \end{bmatrix}$$

Note that  $\partial N_1 / \partial \zeta$  and  $\partial N_1 / \partial z$  will be zero when the interpolated normal at the integration point coincides with the normal to the mid-surface; e.g., in the case of the flat plate ( $\phi_{31}$  and  $\phi_{32}$  are zero). The zero terms in  $[\phi]$ , i.e.,  $\phi_{13}$  and  $\phi_{23}$ , result from dot products of perpendicular vectors.

### A.3 STRESS-STRAIN RELATIONSHIPS

Stresses are related to the previously defined strains by the elasticity matrix  $[G]$  (where  $[G]$  is partitioned to give separate membrane stresses).

$$\begin{Bmatrix} \sigma_M \\ \sigma_B \\ \tau_{TS} \end{Bmatrix} = \begin{bmatrix} G_1 & 0 & 0 \\ 0 & G_2 & 0 \\ 0 & 0 & G_3 \end{bmatrix} \begin{Bmatrix} \epsilon_M \\ \epsilon_B \\ \gamma_{TS} \end{Bmatrix}_{MEC} = \begin{Bmatrix} \epsilon_M \\ \epsilon_B \\ 0 \end{Bmatrix}_T$$

or

(A-24)

$$[\sigma]_I = [G]_I ((\epsilon)_{MEC} - (\epsilon)_T)_I$$

where

$(\sigma_M)$  Membrane stress vector

$(\sigma_B)$  Bending stress vector

$(\tau_{TS})$  Transverse shear stress vector

$[G_1]$  Membrane moduli matrix

$[G_2]$  Bending moduli matrix

$[G_3]$  Transverse shear moduli matrix

and subscripts "MEC" and "T" refer to mechanical and thermal, respectively.

The membrane-bending coupling moduli matrix [G4] will be incorporated into the [G] matrix following this discussion of the uncoupled matrices.

All anisotropic, orthotropic and isotropic material properties are supported. The elastic modulus matrix [G]<sub>M</sub> is defined in the material coordinate system and transformed into the user defined element coordinate system by means of a transformation angle,  $\theta_M$ , which references the user defined element X-AXIS or the material coordinate system ID (MCSID) specified by the user.  $\theta_M$  is in the X-Y plane of the element as shown in Figure A-5.

The elastic modulus matrix in the element coordinate system is:

$$[G]_I = [U]^T [G]_M [U] \quad (A-25)$$

(Note that since the projection of  $X_I$  onto the  $X_E$ - $Y_E$  plane is parallel to  $X_E$ , no extra transformations are required between the "E" and "I" systems.)

The transformation matrix for [G<sub>1</sub>], [G<sub>2</sub>] and [G<sub>4</sub>] is:

$$[U_1] = \begin{bmatrix} \cos^2 \theta_M & \sin^2 \theta_M & \cos \theta_M \sin \theta_M \\ \sin^2 \theta_M & \cos^2 \theta_M & -\cos \theta_M \sin \theta_M \\ -2 \sin \theta_M \cos \theta_M & 2 \sin \theta_M \cos \theta_M & \cos^2 \theta_M - \sin^2 \theta_M \end{bmatrix} \quad (A-26)$$

and the transformation matrix for [G<sub>3</sub>] is:

$$[U_2] = \begin{bmatrix} \cos \theta_M & \sin \theta_M \\ -\sin \theta_M & \cos \theta_M \end{bmatrix} \quad (A-27)$$

For isotropic materials:

(A) Membrane

$$[G_1] = \frac{E}{1-\nu^2} \begin{bmatrix} 1 & \nu & 0 \\ & \nu & 0 \\ \text{SYM} & & \frac{1-\nu}{2} \end{bmatrix} \quad (A-28)$$

(B) Bending

$$[G_2] = \frac{t^3}{12I} [G_1] \quad (A-29)$$



(C) Transverse Shear

$$[G_3] = \frac{t}{t} \begin{bmatrix} \beta_1 \frac{(1-\nu)}{2K} & 0 \\ 0 & \beta_2 \frac{(1-\nu)}{2K} \end{bmatrix} \quad (A-30)$$

where E is the Young's modulus; t is the element thickness at the corresponding integration point,  $\nu$  is the Poisson's ratio and  $t_s/t$  is the transverse shear factor.

Note that in matrix  $[G_3]$ , the factor "K" is introduced to compensate for the difference in shear distribution through the thickness, which is parabolic and not constant as indicated by the displacement function. The value of  $K=1.2$  is the ratio of the relevant strain energies. The  $\beta_i$  factors, which are derived numerically, are introduced to compensate for the "locking" of the element due to excessive shear stiffness.

For anisotropic materials:

(A) Membrane

$$[G_1] = \begin{bmatrix} G_{11} & G_{12} & G_{13} \\ & G_{22} & G_{23} \\ \text{SYM} & & G_{33} \end{bmatrix} \quad (A-31)$$

(B) Bending

$$[G_2] = \frac{t^3}{12I} [G_1] \quad (A-32)$$

(C) Transverse Shear

$$[G_3] = \frac{t}{t} \begin{bmatrix} G_{11} & G_{12} \\ G_{12} & G_{22} \end{bmatrix} \quad (A-33)$$

For orthotropic materials:

(A) Membrane

$$[G_1] = \frac{1}{1-\nu_{12}\nu_{21}} \begin{bmatrix} E_1 & \nu_{12}E_2 & 0 \\ & E_2 & 0 \\ \text{SYM} & & G_{12}(1-\nu_{12}\nu_{21}) \end{bmatrix} \quad (A-34)$$

(B) Bending

$$[G_2] = \frac{t^3}{12I} [G_1] \quad (A-35)$$

(C) Transverse shear

$$[G_3] = \frac{t_s}{t} \begin{bmatrix} G_{1z} & 0 \\ 0 & G_{2z} \end{bmatrix} \quad (A-36)$$

where  $E_1$  and  $E_2$  are the Young's moduli in the principal material axes,  $\nu_{12}$  is the major Poisson's ratio;  $G_{12}$  is the in-plane shear modulus,  $G_{1z}$  and  $G_{2z}$  are the out-of-plane shear moduli and  $t_s/t$  is the transverse shear factor.

The derivation of the  $[G_4]$  membrane-bending coupling matrix begins by denoting the strains at the mid-surface as:

$$(\epsilon_M^0) = \begin{Bmatrix} \epsilon_x^0 \\ \epsilon_y^0 \\ \gamma_{xy}^0 \end{Bmatrix} \quad (A-37)$$

and the out of plane curvatures as:

$$(K) = \begin{Bmatrix} K_x \\ K_y \\ K_{xy} \end{Bmatrix} \quad (A-38)$$

Therefore, the strains at a distance  $z$  above the mid-surface of the element are:

$$(\epsilon) = (\epsilon_M^0) - z(K) \quad (A-39)$$

The corresponding 2-D stresses are:

$$(\sigma) = [G]_I((\epsilon_M^0) - z(K)) \quad (A-40)$$

where  $[G]_I$  is a  $(3 \times 3)$  matrix of elastic moduli.

The forces and moments per unit length are therefore given by:

$$(F) = \int_{z_a}^{z_b} (\sigma) dz = \int_{z_a}^{z_b} [G]_I((\epsilon^0) - z(K)) dz \quad (A-41)$$

$$(F) = t[G_1](\epsilon^0) + t^2[G_4](K)$$

$$(M) = \int_{z_a}^{z_b} (\sigma) z dz = \int_{z_a}^{z_b} [G]_I (-z(\epsilon^0) + z^2(K)) dz \quad (A-42)$$

$$(M) = t^2[G_4](\epsilon^0) + I[G_2](K)$$

where  $t$  is the plate thickness and  $I$  is the bending inertia. Assuming a linear variation of elastic properties between top and bottom surface.

$$[G_1] = \frac{1}{t} \int_{-t/2}^{t/2} G dz = \frac{G_t + G_B}{2} \quad (A-43)$$

$$[G_2] = \frac{1}{I} \int_{-t/2}^{t/2} G dz = \left(\frac{t^3}{12I}\right)[G_1] \quad (A-44)$$

$$[G_4] = \frac{1}{t^3} \int_{-t/2}^{t/2} (-z) G dz = -\left[\frac{G_t - G_B}{12}\right] \quad (A-45)$$

Note that the membrane-bending stiffness coupling terms vanish for a element whose elastic properties are symmetric relative to the mean plane of the element.

By assuming that the elastic modulus has a linear variation between the top and bottom surfaces, define:

$$G = G_1 + \xi/2(G_T - G_B) \quad (A-46)$$

Therefore, from Equations A-31 and A-32:

(A) Membrane

$$G = G_1 + \xi/2(-12G_4) \quad (A-47)$$

$$G = G_1 - 6\xi G_4 \quad (A-48)$$

(B) Bending

$$G_2 = \frac{G_1 t^3}{12I} \quad (A-49)$$

$$G = \frac{12I}{t^3} G_2 - 6\xi G_4$$

Matrix  $[G_3]$  is not affected since transverse shears are assumed to have no coupling action.

Therefore, the stress-strain relationship, allowing for membrane, bending, transverse shear and membrane-bending coupling is:

$$\begin{Bmatrix} \sigma_M \\ \sigma_{TOT} \\ \tau_{TS} \end{Bmatrix} = \begin{bmatrix} G_1 & G_1 - 6\zeta G_4 & 0 \\ G_1 - 6\zeta G_4 & G_2 & 0 \\ 0 & 0 & G_3 \end{bmatrix} \begin{Bmatrix} \epsilon_M \\ \epsilon_B \\ \gamma_{TS} \end{Bmatrix} \quad (A-50)$$

where

$$(\sigma_M) = \begin{Bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{Bmatrix}, \quad \text{Membrane stresses}$$

$$(\sigma_{TOT}) = \begin{Bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{Bmatrix}_{TOT}, \quad \text{Total membrane and bending stresses}$$

$$(\sigma_{TS}) = \begin{Bmatrix} \tau_{yz} \\ \tau_{xz} \end{Bmatrix}, \quad \text{Transverse shear stresses}$$

$$(\epsilon_M) = \begin{Bmatrix} \epsilon_x \\ \epsilon_y \\ \gamma_{xy} \end{Bmatrix}, \quad \text{Membrane strain}$$

$$(\epsilon_B) = \begin{Bmatrix} \epsilon_x \\ \epsilon_y \\ \gamma_{xy} \end{Bmatrix}_B, \quad \text{Bending strains}$$

$$(\gamma_{TS}) = \begin{Bmatrix} \gamma_{yz} \\ \gamma_{xz} \end{Bmatrix}, \quad \text{Transverse shear strains}$$

#### A.4 STIFFNESS MATRIX

The element stiffness matrix is derived by minimizing the total potential energy and is given in numerical form by employing the Gauss-quadrature integration method:

$$[K]_E = \sum \xi \sum \eta \sum \zeta [B]^T [G] [B] W_\xi W_\eta W_\zeta \det [J] \quad (A-51)$$

where  $(\xi, \eta, \zeta)$  are the Gaussian integration point coordinates and  $W_\xi, W_\eta,$  and  $W_\zeta$  are the associated weight factors.  $\det [J]$  represents the physical volume of the element as calculated at this point,  $B$  is the strain displacement relationship of Equation A-21 and  $G$  is the stress strain relationship of Equation A-50.

Each element stiffness matrix partition in the element coordinate system,  $[K_{ij}]_{EE}$ , is transformed to the global coordinate system by the following transformation:

$$[K_{ij}]_G = [TEG]_i^T [K_{ij}]_{EE} [TEG]_i \quad (A-52)$$

where  $[TEG]_i$  is determined by relating the element coordinate system to the global coordinate system for grid  $i$  through the basic coordinate system:

$$[TEG]_i = [TEB]_i [TBG]_i \quad (A-53)$$

#### A.5 CONSISTENT AND LUMPED MASS MATRICES

The consistent mass matrix terms are evaluated, neglecting the rotational inertias associated with the  $\alpha$  and  $\beta$  degrees of freedom, by the following expression:

$$M_{ij} = \sum_{n=1}^4 N_i N_j \rho |J| t_n \quad (A-54)$$

where  $N_i$  is the shape function for node  $i$ ,  $\rho$  is the mass per unit volume,  $|J|$  is the physical area of the element and  $t_n$  is the element thickness at the integration point.

The lumped mass matrix, which is calculated at the pseudo center (i.e., the average of the element grid coordinates), is prorated to the edges based on the distance of the pseudo center from each edge.

The terms of the lumped mass matrix are evaluated using:

$$M_{ij} = \sum_{i=1}^4 N_i \rho |J| t_n \quad (A-55)$$

The transformation of the mass matrix to the global coordinate system is carried out using the same transformation matrices as used for the stiffness matrix in Equation A-52.

#### A.6 STRESS RECOVERY

The element stresses in partitioned form from Equation A-50 are

$$\begin{Bmatrix} \sigma_M \\ \sigma_{TOT} \\ \tau_{TS} \end{Bmatrix} = \begin{bmatrix} G_1 & G_1 - 6\zeta G_4 & 0 \\ G_1 - 6\zeta G_4 & G_2 & 0 \\ 0 & 0 & G_3 \end{bmatrix} \left\{ \begin{Bmatrix} \epsilon_M \\ \epsilon_B \\ \gamma_{TS} \end{Bmatrix}_{MEC} - \begin{Bmatrix} \epsilon_M \\ \epsilon_B \\ \gamma_{TS} \end{Bmatrix} \right\}$$

or

$$\begin{Bmatrix} \sigma_M \\ \sigma_{TOT} \\ \tau_{TS} \end{Bmatrix} = \begin{bmatrix} G_1 & G_4' & 0 \\ G_4' & G_2 & 0 \\ 0 & 0 & G_3 \end{bmatrix} \left\{ \begin{Bmatrix} \epsilon_M \\ \epsilon_B \\ \gamma_{TS} \end{Bmatrix}_{MEC} - \begin{Bmatrix} \epsilon_M \\ \epsilon_B \\ \gamma_{TS} \end{Bmatrix}_T \right\} \quad (A-56)$$

For a specified grid point temperature, the thermal strain vector is:

$$(\epsilon_M)_T = \begin{Bmatrix} \epsilon_x \\ \epsilon_y \\ \gamma_{xy} \end{Bmatrix}_T = (\alpha_I)(T_1 - T_0) \quad (A-57)$$

where  $(\alpha_I) = [U]^{-1}(\alpha_M)$  is a vector of thermal expansion coefficients in the element coordinate system.  $[U]$  is the strain transformation matrix given in Equation A-26 and  $(\alpha_M)$  is the vector of thermal expansion coefficients in the material axes.  $T_1$  and  $T_0$  are the specified grid point temperature and mid-surface (stress-free) temperature, respectively.

For a thermal gradient  $T'$ , the thermal strain vector  $(\epsilon_B)_T$  is:

$$(\epsilon_B)_T = (\alpha_I)\left(\frac{\zeta t}{2} T'\right) \quad (A-58)$$

For thermal moments  $(M)_T$ , the thermal strain vector  $(\epsilon_B)_T$  is:

$$(\epsilon_B)_T = \frac{-\zeta t}{2I} [G_2](M)_T \quad (A-59)$$

NOTE: ASTROS does not support thermal gradient or moments so that the above equations are provided for completeness only.

The in-plane stress vector  $(\sigma)_z$  at fiber distance  $z$  from the mid-surface is:

$$(\sigma)_z = \begin{Bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{Bmatrix}_z = \left(\frac{1}{2} - \frac{z}{t}\right) \begin{Bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{Bmatrix}_1 + \left(\frac{1}{2} + \frac{z}{t}\right) \begin{Bmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{Bmatrix}_2 \quad (A-60)$$

where the stress vectors  $(\sigma_x, \sigma_y, \tau_{xy})_1^T$  and  $(\sigma_x, \sigma_y, \tau_{xy})_2^T$  are the bottom and top fiber stress vectors, respectively.

If a temperature  $T_1$  is specified at the point where outer fiber stresses are to be calculated, the additional thermal stress due to the difference between the specified temperature and a temperature that would be produced by a uniform thermal gradient  $T'$  or thermal moments  $(M)_T$  is calculated using:

$$(\Delta\sigma)_T = [G_2](\alpha_1)(T_1 - T_0 - T'z) \quad (A-61)$$

for a thermal gradient  $T'$ , and

$$(\Delta\sigma)_T = -z \frac{(M)_T}{I} + [G_2](\alpha)T_1 \quad (A-52)$$

#### A.7 FORCE RESULTANTS

The forces at the mid-surface are evaluated by taking the average stress values over the element thickness:

##### (A) Forces

$$(F) = \begin{Bmatrix} F_x \\ F_y \\ F_{xy} \end{Bmatrix} = ((\sigma)_{z1} + (\sigma)_{z2}) \frac{t}{2} \quad (A-63)$$

##### (B) Moments

$$(P) = \begin{Bmatrix} M_x \\ M_y \\ M_{xy} \end{Bmatrix} = ((\sigma)_{z1} - (\sigma)_{z2}) \frac{t}{2} \quad (A-64)$$

##### (C) Transverse Shear Forces

$$(Q) = \begin{Bmatrix} Q_x \\ Q_y \end{Bmatrix} = ((\tau)_{z1} + (\tau)_{z2}) \frac{t}{2} \quad (A-65)$$

where stress vectors  $(\sigma)_{z1}$ ,  $(\sigma)_{z2}$  are stresses at the integration points (default option) or at grid points (if requested) and, similarly,  $(\tau)_{z1}$  and  $(\tau)_{z2}$  are the transverse shear stresses.

#### A.8 THERMAL LOAD VECTOR

The thermal load vector is computed as:

$$(P_T) = \int_V [B][G](\epsilon)_T dv \quad (A-66)$$

where the load vector  $(P_T)$  is defined as:

$$\{P_T\} = \begin{Bmatrix} F_T \\ \text{-----} \\ M_T \end{Bmatrix} \quad (A-67)$$

where  $\{F_T\}$  and  $\{M_T\}$  are the thermal forces and moments, respectively.

The thermal strain vector is:

$$\{\epsilon_T\} = \begin{Bmatrix} \epsilon_{M_T} \\ \text{-----} \\ \epsilon_{B_T} \end{Bmatrix} = \begin{Bmatrix} \alpha_M \\ \text{-----} \\ \alpha_B \end{Bmatrix} \Delta T \quad (A-68)$$

where  $\{\epsilon_M\}_T$  and  $\{\epsilon_B\}_T$  are the thermal membrane and bending strains, and correspondingly  $\{\alpha_M\}$  and  $\{\alpha_B\}$  are the thermal coefficients of expansion for membrane and bending.  $\Delta T$  is dependent on the temperature loading being specified.

- (A) For a specified grid point temperature the thermal membrane strain vector,  $\{\epsilon_M\}_T$ , is:

$$\{\epsilon_M\} = \{\alpha_M\}(T_1 - T_0) \quad (A-69)$$

$T_1$  = Grid point temperature

$T_0$  = Reference (stress-free) temperature

- (B) For a thermal gradient, the thermal bending strain vector,  $\{\epsilon_B\}_T$ , is:

$$\{\epsilon_B\} = \{\alpha_B\} \left( -\frac{z}{2} T' \right) \quad (A-70)$$

- (C) For thermal moments, the thermal bending strain vector,  $\{\epsilon_B\}_T$ , is:

$$\{\epsilon_B\} = [G_2]\{M\}_T \frac{z}{2I} \quad (A-71)$$

NOTE: ASTROS does not support thermal gradients or moments so that the above equations are provided for completeness only.

## A.9 LAMINATED COMPOSITE MATERIALS

The capability to model a stack of layers with a single QUAD4 element is detailed including the computation of equivalent "single layer" properties, i.e., membrane, bending transverse shear and membrane-bending coupling. The recovery of element forces, layer and interlaminar shear stresses and the computation of ply failure indices is also described in the following overview of theory.



### A.9.1 Overview of Theory

The calculation of the "overall" properties for the laminated composite elements is based on the classical lamination theory with the following assumptions:

- (A) Each of the lamina is in a state of plane stress.
- (B) The laminate is presumed to consist of perfectly bonded lamina.
- (C) The bonds are presumed to be infinitesimally thin and non-shear deformable. That is, the displacements are continuous across the lamina boundaries so that lamina can not slip relative to one another. Thus, the laminate behaves as a single layer with "special" properties.

The material properties of laminated composite materials are reflected in the following force-strain relationship:

$$\begin{Bmatrix} F \\ M \\ V \end{Bmatrix} = \begin{bmatrix} t G_1 & t^2 G_4 & 0 \\ t^2 G_4 & I G_2 & 0 \\ 0 & 0 & t_s G_3 \end{bmatrix} \begin{Bmatrix} \epsilon_M - \epsilon_M^T \\ \kappa - \kappa^T \\ \gamma \end{Bmatrix} \quad (A-72)$$

where

$$(F) = \begin{Bmatrix} F_x \\ F_y \\ F_{xy} \end{Bmatrix}, \quad \text{Membrane forces per unit length.}$$

$$(M) = \begin{Bmatrix} M_x \\ M_y \\ M_{xy} \end{Bmatrix}, \quad \text{Bending moments per unit length.}$$

$$(V) = \begin{Bmatrix} V_x \\ V_y \end{Bmatrix}, \quad \text{Transverse shear forces per unit length.}$$

and the remaining terms have been defined previously.

The  $G_1$ ,  $G_2$ , and  $G_4$  terms are defined by the following:

$$\begin{aligned} G_1 &= \frac{1}{t} \int [G_E] dz \\ G_2 &= \frac{1}{I} \int z^2 [G_E] dz \\ G_4 &= \frac{1}{t} \int -z [G_E] dz \end{aligned} \quad (A-73)$$

The limit on the integration are from the bottom surface to the top surface of the laminated composite. The elasticity matrix  $[G_E]$  has the following form for isotropic materials:

$$[G_E] = \begin{bmatrix} \frac{E}{1-\nu^2} & \frac{\nu E}{1-\nu^2} & 0 \\ \text{SYM} & \frac{E}{1-\nu^2} & 0 \\ & & G \end{bmatrix} \quad (\text{A-74})$$

$$G = \frac{E}{2(1+\nu)} \quad (\text{A-75})$$

For orthotropic materials, matrix  $[G_E]$  is:

$$[G_E] = \begin{bmatrix} \frac{E_1}{1-\nu_1\nu_2} & \frac{\nu_1 E_2}{1-\nu_1\nu_2} & 0 \\ \text{SYM} & \frac{E_2}{1-\nu_1\nu_2} & 0 \\ & & G_{12} \end{bmatrix} \quad (\text{A-76})$$

Equation A-73 may be rewritten as:

$$\begin{aligned} [G_{ij}]_1 &= \frac{1}{t} \sum_{K=1}^N [\bar{G}_{ij}]^K (Z_K - Z_{K-1}) \\ [G_{ij}]_2 &= \frac{1}{3I} \sum_{K=1}^N [\bar{G}_{ij}]^K (Z_K^3 - Z_{K-1}^3) \\ [G_{ij}] &= \frac{-1}{2t^2} \sum_{K=1}^N [\bar{G}_{ij}]^K (Z_K^2 - Z_{K-1}^2) \end{aligned} \quad (\text{A-77})$$

where  $[\bar{G}_{ij}]^K$  is the reduced moduli matrix evaluated for each lamina K after transforming the lamina property matrix from the fiber to the element material axes.

$Z_K$  and  $Z_{K-1}$  are the top and bottom distances of lamina K from the geometric middle plane of the laminate, as illustrated in Figure A-6, and N is the number of laminae (or plies). Note that the plies are numbered serially starting with 1 at the bottom layer. The bottom layer is defined as the surface with the largest -z value in the element coordinate system. If the

option to model membrane-only elements is exercised, matrices  $[G_2]$ ,  $[G_3]$ , and  $[G_4]$  are set to zero.

If the user defined element axis is not coincident with the element material axis, the user specified transformation angle  $\theta_M$ , which references the element X-axis, is added to the layer orientation angle. The property matrices  $[G_1]$ ,  $[G_2]$ , and  $[G_4]$  are then transformed to the user defined element axis using the following equation:

$$[G_E] = [U]^T [G_M] [U] \quad (A-78)$$

where

$$[U] = \begin{bmatrix} \cos^2\theta & \sin^2\theta & \cos\theta\sin\theta \\ \sin^2\theta & \cos^2\theta & -\cos\theta\sin\theta \\ -2\sin\theta\cos\theta & 2\sin\theta\cos\theta & \cos^2\theta - \sin^2\theta \end{bmatrix} \quad (A-79)$$

The transverse shear flexibility ( $G_3$ ) matrix is defined by:

$$[G_3] = \begin{bmatrix} G_{11} & G_{12} \\ G_{12} & G_{22} \end{bmatrix} \quad (A-80)$$

and the corresponding matrix transformed into the user-defined element coordinate system is given by:

$$[G] = [W]^T [G_M] [W] \quad (A-81)$$

where

$$[W] = \begin{bmatrix} \cos\theta & \sin\theta \\ -\sin\theta & \cos\theta \end{bmatrix} \quad (A-82)$$

The derivation of the transverse shear flexibility matrix  $[G_3]$  for the laminate is considered next.

The mean value of the transverse shear modulus,  $G$ , for the laminated composite is defined in terms of the transverse shear strain energy,  $U$ , through the depth as:

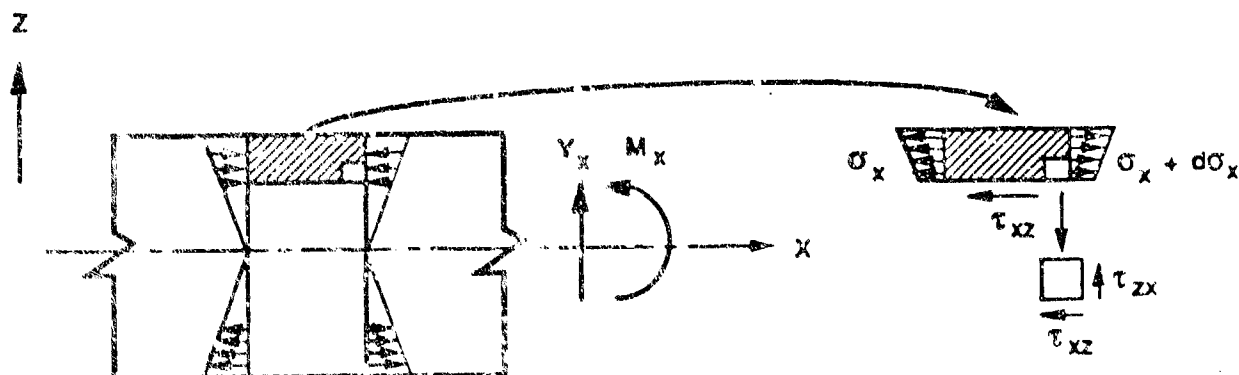
$$U = \frac{V^2}{2Gt} = \frac{1}{2} \int \frac{[x(z)]^2}{G(z)} dz \quad (A-83)$$

A unique mean value of transverse shear strain is assumed to exist for both the x- and y-components of the element coordinate system, but for ease of

discussion, only the evaluation of an uncoupled x-component of the shear moduli will be illustrated here. From Equation A-83, the mean value of transverse shear modulus is written in the following form:

$$\frac{1}{G_x} = \frac{E}{V^2} \sum_{i=1}^N \int_{z_{i-1}}^{z_i} \frac{(v_{zx}(z))^2}{(G_x)_i} dz \quad (A-84)$$

where  $G$  is an "average" transverse shear coefficient used by the element code and  $(G_x)_i$  is the local shear coefficient for layer  $i$ . To evaluate Equation A-84, it is necessary to obtain an expression for  $[v_{zx}(z)]$ . This is accomplished by assuming that the x- and y-components of stress are decoupled from one another. This assumption allows the desired equation to be deduced through an examination of a beam of unit cross-sectional width.



The equilibrium conditions in the horizontal direction and for total moment are:

$$\frac{\partial \tau_{xz}}{\partial z} - \frac{\partial \sigma_x}{\partial x} = 0 \quad (A-85)$$

$$V_x + \frac{\partial M_x}{\partial x} = 0 \quad (A-86)$$

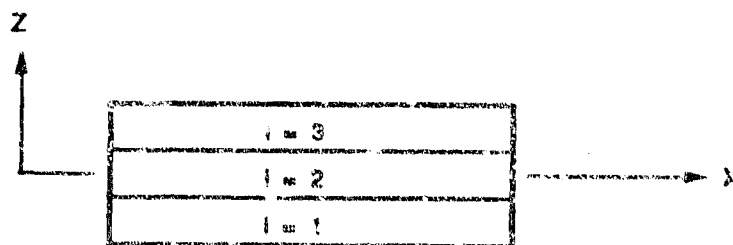
If the location of the neutral surface is denoted by  $x_N$  and  $\rho$  is the radius of curvature of the beam, the axial stress,  $\sigma_x$ , is expressed in the form:

$$\sigma_x = \frac{E(z - z_N)}{(EI)_x} M_x \quad (A-87)$$

Equation A-87 is differentiated with respect to  $x$  and combined with Equations A-85 and A-86. For constant  $E_x$ , the result is integrated to yield the following expression:

$$\tau_{xz} = C_1 + \frac{V}{(EI)_x} \left[ z_{xz} - \frac{z^2}{2} \right] E_{x1} \quad z_{1-1} < z < z_1 \quad (A-88)$$

Equation A-88 is used in the analysis of  $n$ -ply laminates because sufficient conditions exist to determine the constants  $C_i$  ( $i=1,2,\dots,n$ ) and the "directional bending center,"  $z_x$ . For example, consider the following laminated configuration:



At the bottom surface ( $i=1$ ,  $z=z_0$ , and  $\tau_{xz}=0$ ), therefore:

$$C_1 = \frac{V}{(EI)_x} \left[ z_x z_0 - \frac{z_0^2}{2} \right] E_{x1} \quad (A-89)$$

and for the first ply at the interface between plies  $i=1$  and  $i=2$  ( $z=z_1$ ):

$$(\tau_{xz})_1 = \frac{V}{(EI)} \left[ z_x(z_1 - z_0) - \frac{1}{2}(z_1^2 - z_0^2) \right] E_{x1} \quad (A-90a)$$

At this interface between plies  $i=1$  and  $i=2$ :

$$(\tau_{xz})_2 = C_2 + \frac{V}{(EI)_x} \left[ z_x z_1 - \frac{z_1^2}{2} \right] E_{x2} \quad (A-90b)$$

and since  $(\tau_{xz})_2 = (\tau_{xz})_1$  at  $z=z_1$ :

$$C_2 = (\tau_{xz})_1 - \frac{V}{(EI)_x} \left[ z_x z_1 - \frac{z_1^2}{2} \right] E_{x2} \quad (A-91)$$

Then, in the ply,  $z_1 < z < z_2$ , the shear is:

$$\tau_{xz}(z) = (\tau_{xz})_1 + \frac{V E_{x2}}{(\bar{EI})_x} [\bar{z}_x(z-z_1) - \frac{1}{2} (z^2-z_1^2)] \quad (A-92)$$

In general, for any ply  $z_{i-1} < z < z_i$ , the shear is:

$$\tau_{xz}(z)_i = (\tau_{xz})_{i-1} + \frac{V E_{xi}}{(\bar{EI})_x} [\bar{z}_x(z-z_{i-1}) - \frac{1}{2} (z^2-z_{i-1}^2)] \quad (A-93)$$

At any ply interface,  $z_i$ , the shear is therefore:

$$(\tau_{xz})_i = \frac{V}{(\bar{EI})_x} \sum_{j=1}^i E_{xj} T_j [\bar{z} - \frac{1}{2} (z_j + z_{j-1})] \quad (A-94)$$

where  $T_j = z_j - z_{j-1}$ .

Note that the shear at the top face,  $(\tau_{xz})_n$ , is zero and therefore:

$$(\tau_{xz})_n = \frac{V}{(\bar{EI})_x} \left[ \bar{z}_x \sum_{j=1}^n E_{xj} T_j - \sum_{j=1}^n E_{xj} T_j \frac{(z_j + z_{j-1})}{2} \right] = 0 \quad (A-95)$$

Equation A-95 proves that if  $\bar{z}_x$  is the bending center, the shear at the top surface must be zero.

A better form of Equation A-93, for this purpose, is:

$$[\tau_{xz}(z)]_i = \frac{V E_{xi}}{(\bar{EI})_x} \left[ f_{xi} + \bar{z}(z-z_{i-1}) - \frac{1}{2} (z^2-z_{i-1}^2) \right] \quad (A-96)$$

where

$$f_{xi} = \frac{1}{E_{xi}} \sum_{j=1}^{i-1} E_{xj} T_j [\bar{z}_x - \frac{1}{2} (z_j + z_{j-1})] \quad (A-97)$$

Substituting Equation A-96 into Equation A-84 yields:

$$\frac{1}{G_x} = \frac{T}{(\bar{EI})^2} \sum_{i=1}^n \frac{1}{E_{xi}} R_{xi} \quad (A-98a)$$

where

$$R_{x1} = (E_{x1})^2 T_1 \left[ (fx_1 + (\bar{z}_x - z_{1-1})T_1 - \frac{1}{3} T_1^2) fx_1 + (\frac{1}{3} (\bar{z}_x - 2z_{1-1}) - \frac{1}{4} T_1) \bar{z}_x T_1^3 + (\frac{1}{3} z_{1-1}^3 + \frac{1}{4} z_{1-1} T_1 + \frac{1}{20} T_1^3) T_1^3 \right] \quad (A-98b)$$

This expression for the inverse shear modulus for the x-direction is generalized to provide for the calculation of each term in the two-by-two matrix of shear moduli as:

$$[\bar{G}_{k1}] = \left[ \frac{T}{(EI)^2} \sum_{i=1}^n [G_{k1}]^{-1} R_{k1} \right]^{-1} \quad (A-99)$$

where

$$k = 1, 2$$

$$1 = 1, 2$$

Note that if no shear is given,  $[G^1]^{-1} = 0$ , and also that, in Equation A-99:

$$(\bar{EI})_{11} = 1, 1 \text{ term of } I \times [G_2]^*$$

$$(\bar{EI})_{22} = 2, 2 \text{ term of } I \times [G_2]^*$$

where  $[G_2]^*$  is calculated in the same manner as  $[G_3]$  except that Poisson's ratio is set to zero. The moduli for individual plies are provided through user input. Because  $G_{12} \neq G_{21}$ , in general, an average value is used for the coupling terms.

$$[G_3] = \begin{bmatrix} G_{11} & (\bar{G}_{12})_{AVG} \\ (\bar{G}_{12})_{AVG} & G_{22} \end{bmatrix} \quad (A-100)$$

#### A.9.2 Element Layer Stress Recovery

The linear strain variation is given by:

$$(\epsilon_x) = (\epsilon_M) - z(K) \quad (A-101)$$

where

- $(\epsilon_x)$  - Layer strain vector in the element coordinate system.
- $(\epsilon_M)$  - Reference surface strain in the element coordinate system.
- $(K)$  - Reference surface curvatures in the element coordinate system.
- $Z$  - Distance of the mid-surface of the layer  $k$  from the laminate reference surface.

The individual layer stress vector in the fiber coordinate system is:

$$(\sigma_L) = [G_L] [T] (\epsilon_x) \quad (A-102)$$

where

- $(\sigma_L)$  - Layer stress vector in the fiber coordinate system.
- $[G_L]$  - Stress-strain matrix in the fiber coordinate system.
- $[T]$  - Transformation matrix to transform strains from element coordinate system to fiber coordinate system.
- $(\epsilon_x)$  - Layer strain vector in the element coordinate system.

For element temperature and/or thermal gradients, the strain vector has to be corrected for thermal effects before applying Equation A-103:

$$(\epsilon_x) = (\epsilon'_x) - (\alpha) (T + zT') \quad (A-103)$$

and for thermal moments

$$(\epsilon_x) = (\epsilon_x) - (\epsilon_x)^T \quad (A-104)$$

where

- $(\epsilon'_x)$  - Mechanical strains.
- $(\alpha)$  - Thermal coefficients of expansion in the element coordinate system.
- $T$  - Element temperature.
- $T'$  - Element thermal gradient.
- $z$  - Distance from the middle of the layer to the laminate reference surface.
- $(\epsilon_x)^T$  - Layer strains due to thermal moments in the element coordinate system.



The thermal strain vector due to applied thermal moments is determined by substituting for  $\{M\}$  in Equation A-73 and solving for the reference surface strains and curvatures,  $\{\epsilon_M^T\}$  and  $\{K^T\}$ , respectively.

#### A.9.3 Interlaminar Shear Stresses

The interlaminar shear stress  $\tau_{yz}$ ,  $\tau_{xz}$  can be computed at any ply interface from Equation A-96.

#### A.9.4 Force Resultants

Forces and moments for the element are computed using:

$$(F) = \sum_{i=1}^N (\sigma_x) T_i$$

i=1, N (No. of layers) (A-105)

$$(M) = \sum_{i=1}^N -z_i T_i (\sigma_x)$$

where

(F) - In-plane force resultants.

(M) - Out-of-plane moments.

$(\sigma_x)$  - Stresses in the element coordinate system.

$T_i$  - Layer thickness.

$z_i$  - Distance from the middle of the layer to the laminate reference surface.

#### A.9.5 Failure Indices

Failure indices assume a value of one on the periphery of a failure surface in stress space. If the failure index is less than one, the lamina stress is interior to the periphery of the failure surface and the lamina is assumed "safe" and if it is greater than one the lamina is assumed to have "failed." The failure indices represent a phenomenological failure criterion, because only the occurrence of failure is predicted.

The analytical definition of a failure surface in stress space for a lamina subjected to biaxial (planar) states of stress is provided via the following failure theories.

- (1) HILL
- (2) HOFFMAN
- (3) TSAI-WU
- (4) MAXIMUM STRESS
- (5) MAXIMUM STRAIN

In the analysis of laminated composites, which are typically orthotropic materials (possibly exhibiting unequal properties in tension and compression), the strength of orthotropic lamina is a function of body orientation relative to the imposed stress. In order to determine the structural integrity of the lamina, a set of intrinsic strength properties (allowable stresses or allowable strains) in the principal material directions are defined as:

- $X_t$  - Ultimate uniaxial tensile strength in the fiber direction,
- $X_c$  - Ultimate uniaxial compressive strength in the fiber direction,
- $Y_t$  - Ultimate uniaxial tensile strength perpendicular to the fiber direction,
- $Y_c$  - Ultimate uniaxial compressive strength perpendicular to the fiber direction,
- $S$  - Ultimate planar shear strength under pure shear loading,
- $E_t$  - Ultimate uniaxial tensile strain in the fiber direction,
- $E_c$  - Ultimate uniaxial compressive strain in the fiber direction,
- $F_t$  - Ultimate uniaxial tensile strain perpendicular to the fiber direction,
- $F_c$  - Ultimate uniaxial compressive strain perpendicular to the fiber direction, and
- $E_s$  - Ultimate planar shear strain under pure shear loading.

For most composite materials, the planar shear strengths and strains are equal for positive and negative shear loadings.

The five failure theories and a bonding failure index are now described:

### HILL'S THEORY

$$\frac{\sigma_1^2}{X^2} + \frac{\sigma_2^2}{Y^2} - \frac{\sigma_1 \sigma_2}{X^2} + \frac{\tau_{12}^2}{S^2} = \text{FAILURE INDEX (FI)} \quad (\text{A-106})$$

and  $X-X_t$  if  $\sigma_1$  is positive, and  $X_c$  if  $\sigma_1$  is negative; similarly for  $y$ . For the interaction term,  $(\sigma_1 \sigma_2)/X^2$ ,  $X-X_t$  if  $\sigma_1 \sigma_2$  is positive  $X-X_c$  otherwise.

### HOFFMAN'S THEORY

$$\left( \frac{1}{X_t} - \frac{1}{X_c} \right) \sigma_1 + \left( \frac{1}{Y_t} - \frac{1}{Y_c} \right) \sigma_2 + \frac{\sigma_1^2}{X_t X_c} + \frac{\sigma_2^2}{Y_t Y_c} + \frac{\tau_{12}^2}{S^2} + \frac{\sigma_1 \sigma_2}{X_t X_c} = FI \quad (\text{A-107})$$

Note that this theory takes into account the difference in the tensile and compressive allowable stresses by using linear terms in the failure equation.

### TSAI-WU THEORY

This quadratic interaction theory allows for the strength predictions wherein interaction among stress components can be considered in determining strengths in a biaxial field. Thus, in the case of an orthotropic lamina in a general state of planar stress:

$$F_1 \sigma_1 + F_2 \sigma_2 + F_{11} \sigma_1^2 + F_{22} \sigma_2^2 + 2F_{12} \sigma_1 \sigma_2 + F_{66} \tau_{12}^2 = FI \quad (\text{A-108})$$

$$F_1 = \frac{1}{X_t} - \frac{1}{X_c}, \quad F_2 = \frac{1}{Y_t} - \frac{1}{Y_c}, \quad (\text{A-109})$$

$$F_{11} = \frac{1}{X_t X_c}, \quad F_{22} = \frac{1}{Y_t Y_c}, \quad F_{66} = \frac{1}{S^2}$$

and  $F_{12}$  needs to be determined experimentally, from a biaxial test. However, satisfactory results may be obtained by setting it to zero.

### MAXIMUM STRESS

Failure is assumed to occur when any one of the stress components is equal to its corresponding intrinsic strength property. In mathematical form, the Maximum Stress theory is given by:

$$\begin{aligned}
\sigma_1 \geq X_T, \sigma_1 > 0 & ; \sigma_1 \leq -X_C, \sigma_1 < 0 \\
\sigma_2 \geq Y_T, \sigma_2 > 0 & ; \sigma_2 \leq -Y_C, \sigma_2 < 0 \\
\tau_{12} \geq S, \tau_{12} > 0 & ; \tau_{12} \leq -S, \tau_{12} < 0
\end{aligned}
\tag{A-110}$$

where the intrinsic strength properties are as defined previously.

#### MAXIMUM STRAIN

The Maximum Strain theory is analogous to the Maximum Stress theory. Failure is assumed to result when any one of the strain components is equal to its corresponding intrinsic ultimate strain. In mathematical form the Maximum Strain theory is given by:

$$\begin{aligned}
\epsilon_1 \geq E_T, \epsilon_1 > 0 & ; \epsilon_1 \leq E_C, \epsilon_1 < 0 \\
\epsilon_2 \geq F_T, \epsilon_2 > 0 & ; \epsilon_2 \leq F_C, \epsilon_2 < 0 \\
\gamma_{12} \geq E_S, \gamma_{12} > 0 & ; \gamma_{12} \leq E_S, \gamma_{12} < 0
\end{aligned}
\tag{A-111}$$

where the intrinsic ultimate strains are as defined previously.

#### FAILURE INDEX OF BONDING

The failure index of bonding material is calculated as the maximum interlaminar shear stress divided by the allowable bonding stress.

#### A.10 CORRECTION OF OUT-OF-PLANE SHEAR STRAIN

The typical formulation for a QUAD4 type finite element follows a standard bilinear isoparametric theory, with directional reduced integration for out-of-plane shear strain. However, this formulation has been found to be inadequate when the geometry of the element is irregular, and a correction defined herein has been implemented in ASTROS to correct this problem.

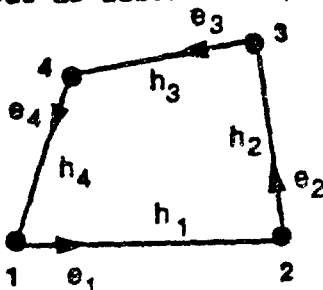
The modification is based upon the theory presented by Hughes and Tezdayar (Reference A-1), but is generalized to include non-planarity of the element, and special features to accommodate ASTROS's structure. The formulation enforces constant shear along each edge of the element, eliminating the need to perform reduced integration.

The formulation of this modification consists of establishing strain-displacement relationships in the element coordinate system. It involves six degrees of freedom (dof), the rotational part of which will be modified later to include the singularity about the normal to the mid-surface.

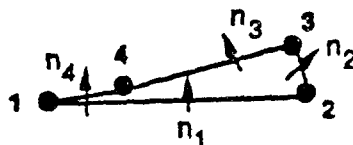
### A.10.1 Geometric Variables

The following terms are defined for each edge of an irregular-shaped, non-planar element:

A Unit Normal Vector ( $\vec{e}$ ) in the direction of the next node as illustrated;



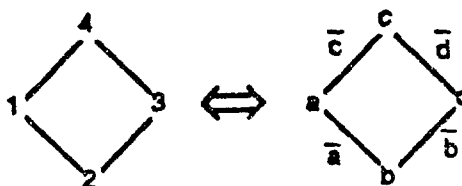
A Unit Normal Vector ( $\vec{n}$ ) which is a normalized average of the nodal normals to the mid-surface along that edge;



Length of each edge ( $h_i$ ); and cosine of the internal angle at each corner ( $\alpha_i$ ).

### A.10.2 Edge Shears and Shear Vectors

Given the following numbering sequence:



At the middle of each edge, the constant shears parallel to edges  $\bar{a}$ ,  $\bar{b}$ ,  $\bar{c}$  and  $\bar{d}$ , respectively, are:

$$g_a = \frac{1}{h_a} \vec{n}_a \cdot (\vec{U}_b - \vec{U}_a) - \frac{1}{2} \vec{e}_a (\vec{\theta}_b + \vec{\theta}_a)$$

$$g_b = \frac{1}{h_b} \vec{n}_b \cdot (\vec{U}_d - \vec{U}_b) - \frac{1}{2} \vec{e}_b (\vec{\theta}_d + \vec{\theta}_b)$$

(A-112)

$$g_c = \frac{1}{h_c} \vec{n}_c \cdot (\vec{U}_a - \vec{U}_c) - \frac{1}{2} \vec{e}_c (\vec{\theta}_a + \vec{\theta}_c)$$

$$g_d = \frac{1}{h_d} \vec{n}_d \cdot (\vec{U}_c - \vec{U}_d) - \frac{1}{2} \vec{e}_d (\vec{\theta}_c + \vec{\theta}_d)$$

where  $\vec{U}$  and  $\vec{\theta}$  are the vectors of translations and rotations at each node, respectively, in the element coordinate system.

The shear vector ( $\vec{\gamma}_b$ ) at node (b) is given by:

$$\vec{\gamma}_b = \frac{1}{1-\alpha_b^2} (g_b + g_a \alpha_b) \vec{e}_b + \frac{1}{1-\alpha_b^2} (g_a + g_b \alpha_b) \vec{e}_a \quad (A-113)$$

or

$$\begin{aligned} \vec{\gamma}_b = & \left[ \frac{1}{(1-\alpha_b^2)h_a} (\vec{e}_a + \alpha_b \vec{e}_b) (\vec{n}_a \cdot \vec{U}_a) \right] + \left[ \frac{1}{(1-\alpha_b^2)h_a} (\vec{e}_a + \alpha_b \vec{e}_b) (\vec{n}_a \cdot \vec{U}_b) \right] \\ & - \left[ \frac{1}{(1-\alpha_b^2)h_b} (\vec{e}_b + \alpha_b \vec{e}_a) (\vec{n}_c \cdot \vec{U}_b) \right] + \left[ \frac{1}{(1-\alpha_b^2)h_b} (\vec{e}_b + \alpha_b \vec{e}_a) (\vec{n}_b \cdot \vec{U}_d) \right] \\ & - \left[ \frac{1}{2(1-\alpha_b^2)} (\vec{e}_a + \alpha_b \vec{e}_b) (\vec{e}_a \cdot \vec{\theta}_a) \right] \\ & - \left[ \frac{1}{2(1-\alpha_b^2)} \left[ (\vec{e}_a + \alpha_b \vec{e}_b) (\vec{e}_a \cdot \vec{\theta}_b) + (\vec{e}_b + \alpha_b \vec{e}_a) (\vec{e}_b \cdot \vec{\theta}_b) \right] \right] \\ & - \left[ \frac{1}{2(1-\alpha_b^2)} (\vec{e}_b + \alpha_b \vec{e}_a) (\vec{e}_b \cdot \vec{\theta}_c) \right] \end{aligned} \quad (A-114)$$

and similarly for the other nodes, by permutations of the a, b, c and d subscripts.

#### A.10.3 Nodal Contributions of Shear Strain

The contribution of each node to the total shear strain ( $\vec{\gamma}_T$ ) evaluated at an integration point is:

$$\vec{\gamma}_T = \sum_{i=1}^4 N_i \vec{\gamma}_i \quad (\text{A-115})$$

The "pseudo-contribution" of each edge to the total shear strain (G) has the following form:

$$\begin{aligned} \vec{G}_a &= \frac{N_a}{1-\alpha^2} (\vec{e}_a + \alpha_a \vec{e}_c) + \frac{N_b}{1-\alpha^2} (\vec{e}_a + \alpha_b \vec{e}_b) \\ \vec{G}_b &= \frac{N_b}{1-\alpha^2} (\vec{e}_b + \alpha_b \vec{e}_a) + \frac{N_d}{1-\alpha^2} (\vec{e}_b + \alpha_d \vec{e}_d) \\ \vec{G}_c &= \frac{N_c}{1-\alpha^2} (\vec{e}_c + \alpha_c \vec{e}_d) + \frac{N_a}{1-\alpha^2} (\vec{e}_c + \alpha_a \vec{e}_a) \\ \vec{G}_d &= \frac{N_d}{1-\alpha^2} (\vec{e}_d + \alpha_d \vec{e}_b) + \frac{N_c}{1-\alpha^2} (\vec{e}_d + \alpha_c \vec{e}_c) \end{aligned} \quad (\text{A-116})$$

Hence, the columns of the [B] matrix partition for shear, corresponding to node b, [BS<sub>bj</sub>], are:

$$\left. \begin{aligned} (\overline{BS}_{b1}) &= \frac{n_a^1}{h_a} \vec{G}_a + \frac{n_b^1}{h_b} \vec{G}_b \\ (\overline{BS}_{bj}) &= \frac{e_a^j}{2} \vec{G}_a + \frac{e_b^j}{2} \vec{G}_b \end{aligned} \right\} \begin{array}{l} j=1,2,3 \\ j=4,5,6 \end{array} \quad (\text{A-117})$$

#### A.10.4 Transformations

The following transformations have to be performed before the preceding formulation can replace the existing [B] matrix generation for out-of-plane shear.

$$[BS_b]_{(3 \times 6)} = [TIE]_{(3 \times 3)} \overline{[BS_b]}_{(3 \times 6)} \begin{bmatrix} [I] & 0 \\ \hline & [TEE] \end{bmatrix}_{(6 \times 6)} \quad (A-118)$$

where

[TIE] - Is the orthogonal transformation between integration points and the element coordinate system, required since all the strains are calculated in the 1 system.

[I] - Is a 3x3 identity matrix.

[TEE] - Is the 3x3 transformation which takes into account the following facts

(A) Hughes' convention for rotations is different than the one implemented in STROS; and,

(B) The rotation about the normal to the mid-surface at each grid point is singular.

If NV is the normal vector at a given grid point, then.

$$[TEE] = \begin{bmatrix} 0 & -NV^3 & NV^2 \\ NV^3 & 0 & -NV^1 \\ -NV^1 & NV^1 & 0 \end{bmatrix} \quad (A-119)$$



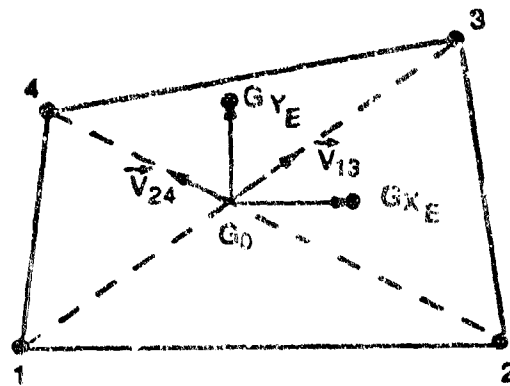


Figure A-1. Internal Element Coordinate System

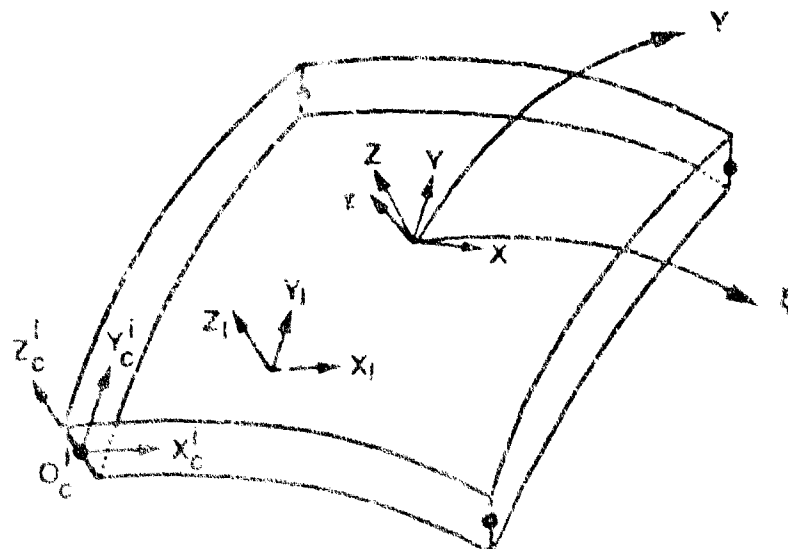


Figure A-2. Isoparametric Quadrilateral 4-Node Plate and Shell Element

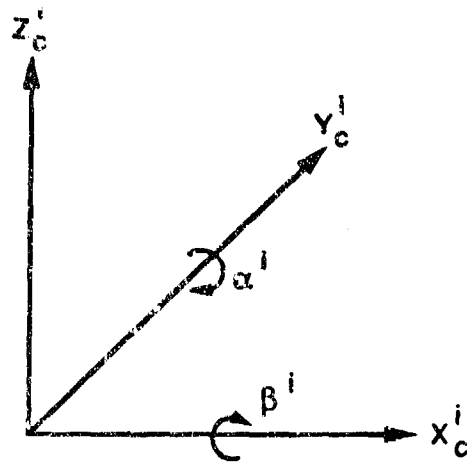


Figure A-3. Deformations at Grid Point  $i$

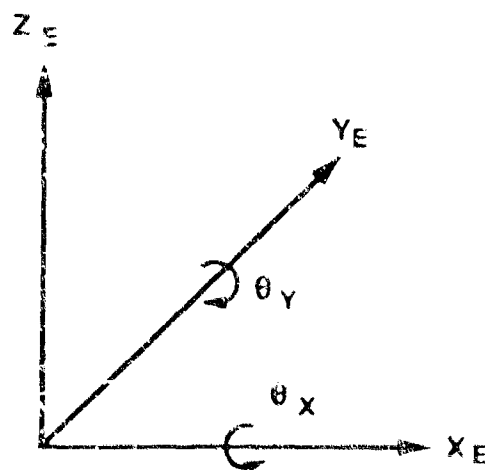


Figure A-4. Deformations in the Global Direction

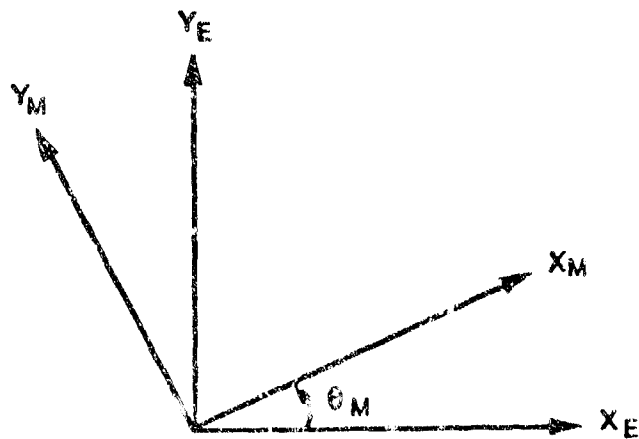


Figure A-5. Material and User Defined Element Axes

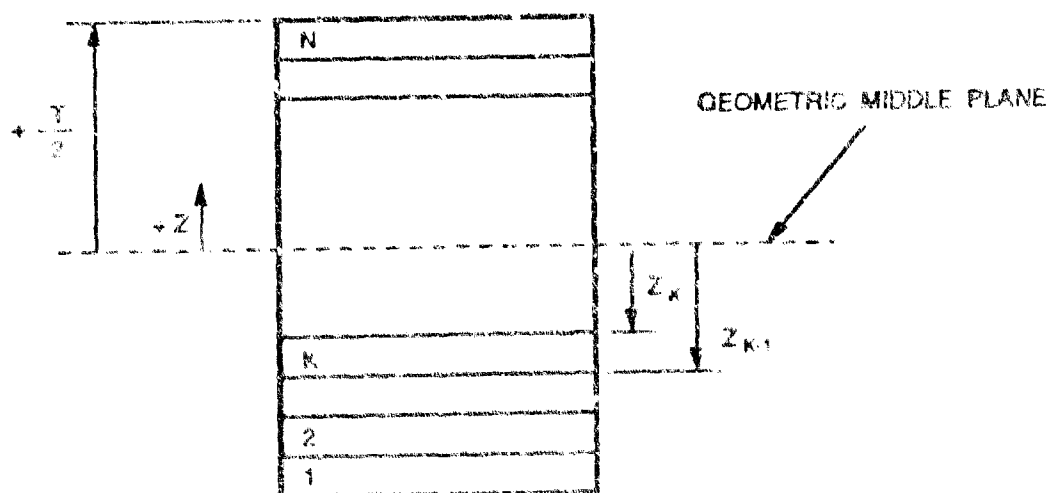


Figure A-6. Geometry of a N-Layered Element

## REFERENCES

- A-1. Hughes, T. J. R. and Tezduyar, T. F., "Finite Elements Based Upon Mindlin Plate Theory with Particular Reference to the Four-Node Bilinear Isoparametric Element," Transactions of ASME, Journal of Applied Mechanics, Volume 48, No. 3, September, 1981, pp 587-596.

APPENDIX A: GUIDELINES FOR MODELING WITH QUAD4 (TRIAB) ELEMENTS

## GUIDELINES FOR MODELING WITH QUAD4 (TRIA3) ELEMENTS

ASTROS (an Automated STRuctural Optimization System) and four versions of NASTRAN (COSMIC NASTRAN, UAI NASTRAN, CSA NASTRAN and MSC NASTRAN) provide QUAD4 and TRIA3 elements for the analysis of plates. They are basically flat plate elements, but they are often used for shell structures as well as approximations. These approximations become closer to reality with a finer mesh size. Much of the discussion in these guidelines is with reference to the QUAD4 element, but comments are equally applicable to the TRIA3 element. The original QUAD4 formulation in programs ASTROS, COSMIC NASTRAN and UAI NASTRAN is just about identical, and differences (if any) are the result of subsequent revisions. MSC NASTRAN and CSA NASTRAN are different, but nevertheless all five programs give comparable results for most well posed problems. Also the input requirements and the card structure are quite compatible in all five cases with minor differences in interpretation. The purpose of these seminar notes is to explain various QUAD4 modeling options and to point-out the differences in the five programs. An explanation of the input parameters on the various cards is considered the easiest way to accomplish this objective.

The QUAD4 is one of the most extensively used elements in NASTRAN as well as ASTROS. It is a very versatile element and can be used to model a variety of plate simulations such as (see Fig. 1).

- a. Membrane (inplane loading) behavior
- b. Bending (out of plane loading) behavior
- c. Membrane-bending (uncoupled)
- d. Membrane-bending (coupled-linear)
- e. Laminated plates
- f. Layered composites
- g. Sandwich plates with metal face sheets
- h. Sandwich plates with layered composite face sheets
- i. Isotropic materials
- j. Anisotropic (including orthotropic) materials

Application of the QUAD4 is often confusing because of the many options available for its use.

There are five cards which describe the input parameters for the QUAD4. They describe its geometry and properties along with some auxiliary information.

### Geometry and Property Cards

CQUAD4 - Connection card  
PSHELL - Property card for homogeneous and sandwich plates  
  
PCOMP  
or  
PCOMP1\* - Property cards for laminated or layered plates  
or  
PCOMP2\*

\*Not applicable to MSC NASTRAN

## Material Cards

MAT1 - Isotropic materials  
MAT2 - Anisotropic materials  
MAT8 - Orthotropic materials  
FLOAD4 - Pressure load definition on the QUAD4 element

The property cards PCOMP1 and PCOMP2 are not applicable to MSC NASTRAN. For a given element either the PSHELL or PCOMP card is applicable but not both. PSHELL cards are for homogeneous (nonlaminated) and sandwich plates with nonlayered face sheets. PCOMP cards are for laminated (layered) plates. In the case of sandwich plates with layered face sheets the honeycomb (sandwich) core will be treated as a laminate or layer.

A supplementary explanation of the parameters on each of these cards should aid in understanding the modeling nuances of the element.

## Notes on the CQUAD4

The format of the CQUAD4 is the same for all the NASTRANs and ASTROS with the exception of an additional parameter "TMAX" (field 3 on the continuation card) in ASTROS. "TMAX" is the maximum allowable thickness of the plate, applicable only in optimization.

The definitions of the parameters in fields 2 to 7 are self explanatory and need no further clarification. Similarly no additional explanation is necessary for the thickness parameters specified in fields 4 to 7 on the continuation card. However, the parameters TM (THETA) and ZOFF need a supplementary explanation or caution.

### Parameter TM (THETA)

The parameter TM defines the material property orientation. There are two options for this definition.

#### Option 1:

Define the angle between the side of the element (connecting G1 and G2) and the material axis. This is the least desirable option. It is prone to errors, because every time the sequence of the element connection changes, the angle must be changed. Also in a complex three dimensional model it is not easy to determine this angle without writing a preprocessor.

#### Option 2:

The integer option is preferable. An integer in field 8 refers to a separate coordinate system for defining the orientation of the material axis of the element. The material property definition is now independent of the connection sequence. The new coordinate system can be defined with a CORDZC card.

### Offset Parameter ZO (ZOFFS, ZOFF)

The offset parameter provision in the QUAD4 element constitutes a significant enhancement for plate elements. Before the QUAD4 the grid points of the structure could only be defined on the mid-surface of the plate elements. The Bar (beam or bend) was the only other element with an offset capability. However, some of the mass elements have the offset capability.

The offset, ZO, is shown for various cases in Fig. 3. Note the distinction between the grid point surface and mid-surface of the element. The definition of the offset ZO on the CQUAD4 is the same in all four NASTRAN and ASTROS. The offset implementation in COSMIC NASTRAN and ASTROS appears to give more consistent results than MSC and CSA NASTRAN. See Reference A1 for results on the benchmarking of offsets.

#### Notes on PSHELL

PSHELL or PCOMP are the property cards referenced on the CQUAD4 (in field 3). The PSHELL is to be used when the plate is not laminated (or layered), while the PCOMP is for laminated plates. Only one of these is applicable for a given element. A preprocessor in NASTRAN (ASTROS) generates equivalent PSHELL cards from the PCOMP cards before proceeding to the solution. NASTRAN (ASTROS) provides versatility to the QUAD4 element through the PSHELL card. It allows the modeling of membrane (inplane), bending, shear and membrane-bending coupling behavior. Fig. 2 illustrates key features of the elements described on the PSHELL card. It is a sandwich plate with two face sheets separated by a honeycomb core.

The first two fields of the PSHELL card are for the name and property identification called from the CQUAD4. The third field, MID1, is the material identification number for the face sheets in membrane behavior. The parameter T is the total thickness of the two face sheets. MID2 is the material identification number for bending behavior, MID3 for shear and MID4 for membrane-bending coupling. There are two types of membrane bending coupling. The coupling resulting from asymmetry in plate construction (non-symmetric laminates) is called linear coupling. Nonlinear coupling, on the other hand, is a result of the interaction of internal forces such as inplane and out of plane (beam-column effect) forces. The latter coupling can be accounted for only in differential stiffness and/or buckling analysis. The parameter  $12I/T^3$  (field 6) can be calculated by using the following definition for I.

$$I = 2 \left[ \frac{1}{12} \left( \frac{T}{2} \right)^3 + \frac{T}{2} \left( \frac{TS}{2} + \frac{T}{4} \right)^2 \right]$$

I is basically the moment of inertia of the face sheets about the neutral axis (centroidal). It is assumed that the face sheets are symmetric about the neutral axis. If they are not, the moment of inertia about the neutral axis can be calculated. For solid plates this parameter is simply 1.0

The definition of the parameter TS/T is obvious from the figure.

No further explanation is necessary for the parameters in the next three fields, NSM, Z1 and Z2.

The parameters MCSID and SCSID (not available in MSC and CSA NASTRAN) refer to the material coordinate system. As stated in the description of the card CQUAD4 there are two options for this definition. By leaving the field blank or a real value the first option is invoked. In this option the parameter represents the angle between the side of the element connecting the grid points G1 and G2 and the material axis. The second option is an integer which refers to a coordinate system defined on a COORD-card. The second option is the most desirable because the grid point sequence on the CQUAD4 card does not affect the material axis.



The offset parameter Z0 is the same as defined on the CQUAD4 (See Fig. 3). An important point to note is that there is no provision for the offset definition on the PSHELL card in MSC and CSA NASTRAN. There is some advantage in having this option on the PSHELL, because when the number of PSHELL cards is significantly fewer than the number of CQUAD4 cards, this parameter need not be repeated on all the CQUAD4 cards. The entry on CQUAD4, however, overrides that on the PSHELL card.

The PSHELL card provides the facility to model homogeneous as well as sandwich plates. However, the face sheets of the sandwich plates are assumed to be homogeneous (isotropic, orthotropic or anisotropic) plates. A discussion of sandwich plates with face sheets made of layered composites is deferred until the PCOMP cards description. The minimum thickness parameter TMIN is applicable only in ASTROS optimization.

Before leaving this discussion, it is worth pointing out some anomalies (idiosyncrasies) (as they exist at present, Feb 1993) in the application of the PSHELL card in various versions of NASTRAN (ASTROS).

Specifying MID1 and leaving the remaining three material identifications blank simulates the membrane behavior in all five programs with little or no differences to point out.

Specifying MID2 only invokes bending behavior only (as it should be) in MSC NASTRAN. It does not compute shear deformation. CSA NASTRAN on the other hand includes shear deformation, ostensibly with the material properties invoked by MID2. ASTROS and COSMIC NASTRAN go even further by including membrane, bending and shear deformations even though only MID2 is specified. Membrane and bending behavior are computed by the material properties called from MID2. This is not so bad, because these two behaviors are generally uncoupled. The most important point to note is that ASTROS (COSMIC NASTRAN) computes the shear deformation by assuming a material infinitely stiff in transverse shear when the MID3 field is blank. These results are, in general, not acceptable. The easiest way to avoid shear stiffness overestimation at present is not to leave MID3 blank when MID2 is specified. See results of the examples at the end of this Appendix. Future versions of ASTROS and COSMIC NASTRAN will uncouple these computations.

#### Notes on PCOMP, PCOMP1, PCOMP2

The purpose of PCOMP, PCOMP1 and PCOMP2 is to define element property parameters in modeling laminated plates including layered (fiber reinforced) composites. All three cards serve the same purpose except the options are different. If the layers are made of different materials and the thicknesses of the layers are all different, then the PCOMP card is appropriate. If all the layers are made of the same material and thickness, then PCOMP1 is appropriate. If the material is the same, but the thicknesses are different, then PCOMP2 is appropriate. The first two fields on the PCOMP cards need no further explanation.

#### Parameter Z0

The parameter Z0 refers to the distance from the grid point surface to the bottom of the plate. The plate bottom surface is defined in Fig. 4. It is the reference surface from which the stacking sequence of the laminates is defined. The parameters ZOFF defined on the (QUAD4 and PSHELL) are a source of confusion sometimes. However, figures 3 and 4 should provide better clarification.

### Parameter SBOND

The bonding material shear stress is indirectly related to the interlaminar shear and its value is generally empirical. A value of 400 to 500 psi for SBOND appears to be reasonable in the absence of a value obtained from experiments. Any approximation of this parameter will not affect the analysis results. It affects only the failure theory which is basically a postprocessing function.

### Parameters FT (Field 6)

This parameter simply identifies the desired failure theory. The five failure theories are discussed in detail in the theoretical section of the QUAD4 seminar notes. The possible values of FT are:

- "HILL" for the Hill Theory
- "HOFF" for the Hoffman Theory
- "TSAI" for the Tsai-Wu Theory
- "STRESS" for the Maximum Stress Theory
- "STRAIN" for the Maximum Strain Theory

### Parameter LOPT

This parameter description needs no further explanation than what is given in the PCOMP card descriptions. It should be pointed out, however, that no parameters are in fields 7 and 8 in COSMIC NASTRAN, while MSC and CSA NASTRAN define "TREF," a reference temperature for thermal stress analysis and "GE" the material damping coefficient for dynamic analysis.

### Parameters MIDi, Ti, THi, and SOUTi

These parameters pertain to the ith layer. MID1 is the material identification number of the first layer. The layer count goes up from the bottom surface of the plate. For the definition of the bottom surface see Fig. 4 for the definition of Z0. The MIDi refers to one of three material cards: MAT1 for isotropic materials, MAT2 for anisotropic materials and MAT8 for orthotropic materials. The parameter Ti defines the thickness of the first layer and TH1 refers to the orientation of the material axis with reference to the material axis defined on the CQUAD4. SOUT1 is the stress output parameter. Then the parameters are repeated for all the layers unless the symmetry option is used under the parameter LOPT. If any MIDi, Ti or THi are blank, then the last non-blank values specified for each will be used.

### Material Cards

#### Isotropic & Anisotropic

The parameters on MAT1 and MAT2 are self explanatory from the card descriptions.

#### Orthotropic Material MAT8

Most of the parameters on MAT8 are self explanatory with the exceptions of G12 and G22 (fields 7 and 8). When these parameters are left blank, MSC NASTRAN and CSA NASTRAN do not calculate the transverse shear deformation in plate bending problems. ASTROS and COSMIC NASTRAN, on the other hand, assume that the material is infinitely stiff in transverse shear, and thus overestimate the stiffness of the element. To avoid such overestimation and to obtain comparable results to the other NASTRANS, transverse shear values have to be provided. Values of about two or three orders of magnitude less than the modulus of elasticity of the material are recommended.

These guidelines and the accompanying examples should clarify most of the questions arising in the use of the QUAD4 and TRIA3 elements.

#### REFERENCE

1. Pitrof, Stephen M. and Venkayya, Vipperi B., Benchmarking the QUAD4/TRIA3 Element, Twenty-first NASTRAN User's Colloquium, Tampa FL, April 1993.

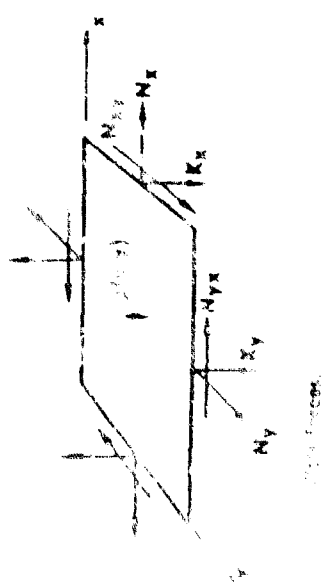


Plate forces.

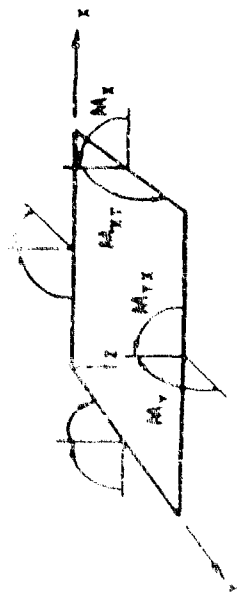
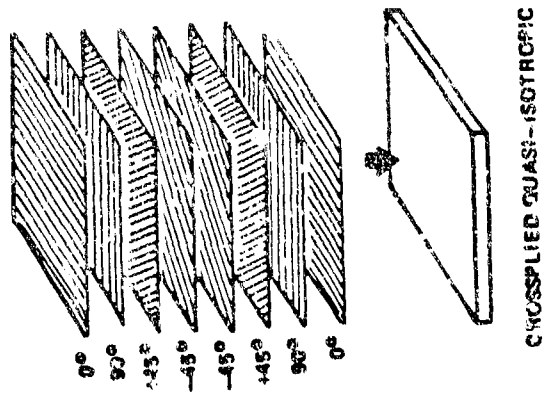
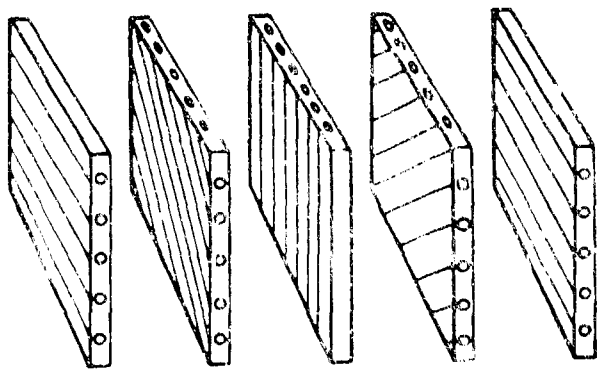


Plate moments.



Laminate construction.

Figure 1

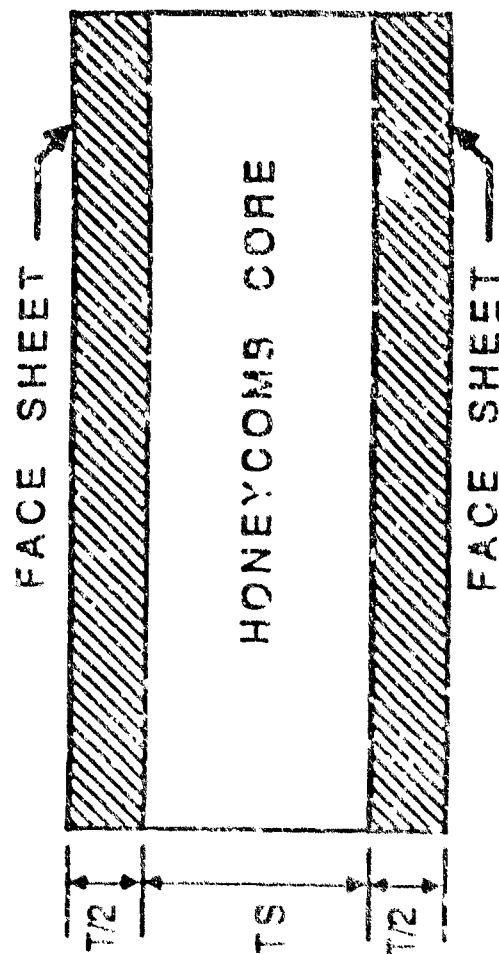


Figure 2

# QUAD4 - OFFSET DEFINITION

(CQUAD4 PSHELL)

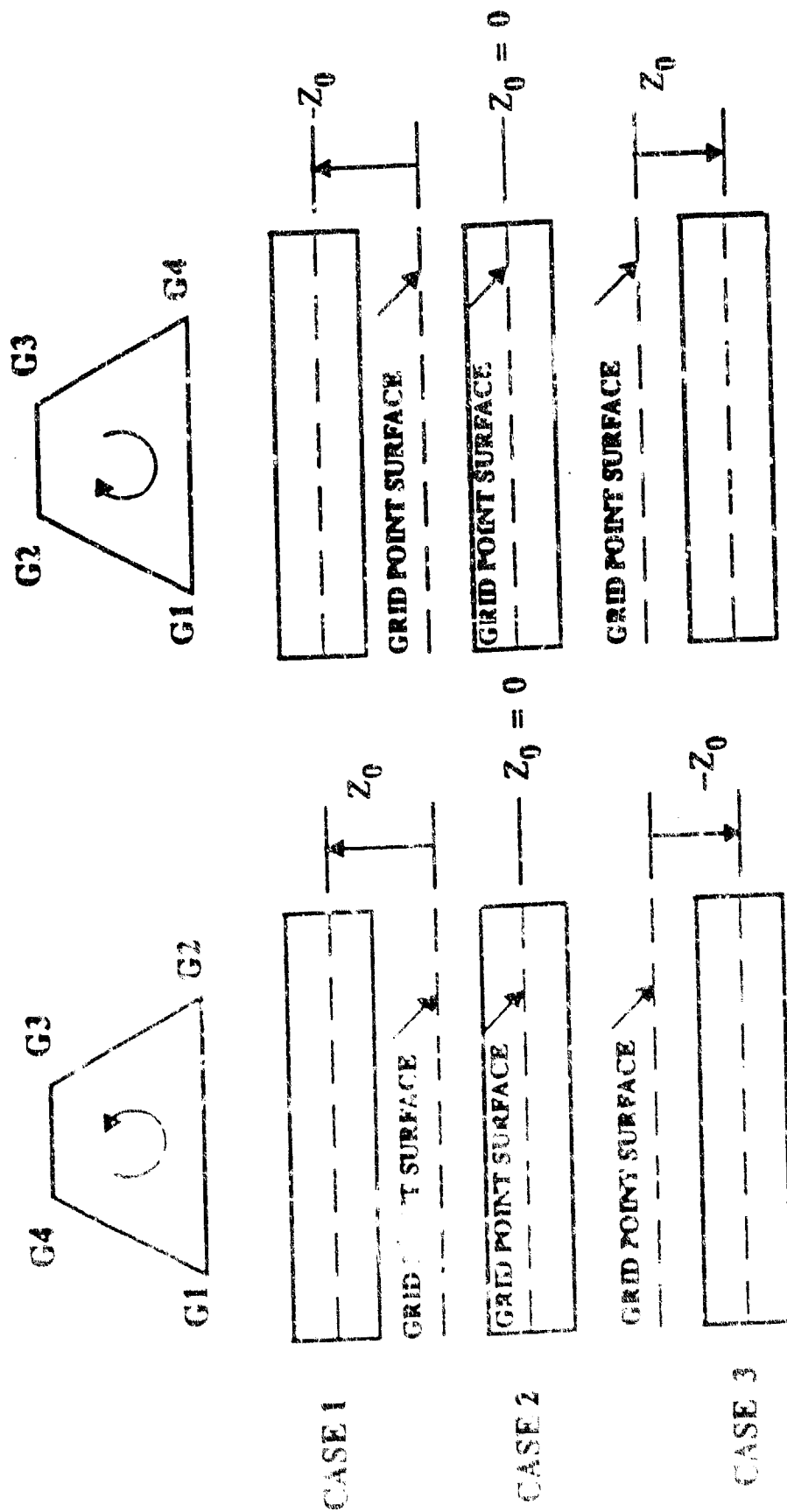


Figure 3

# QUAD4 - OFFSET DEFINITION

(PCOMP PCOMP1 PCOMP2)

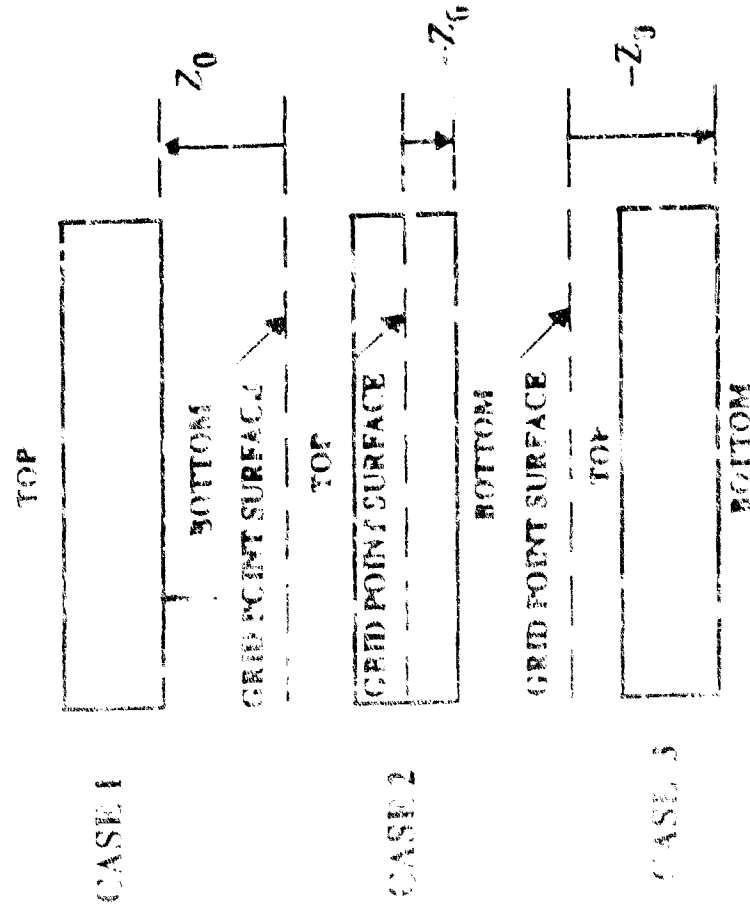
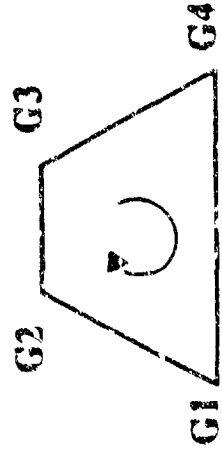
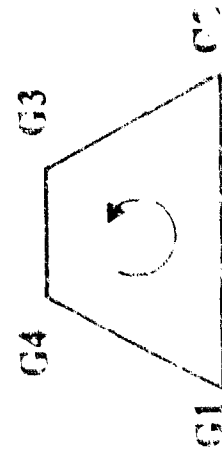


Figure 4

## PROBLEM #5

## RECTANGULAR PLATE

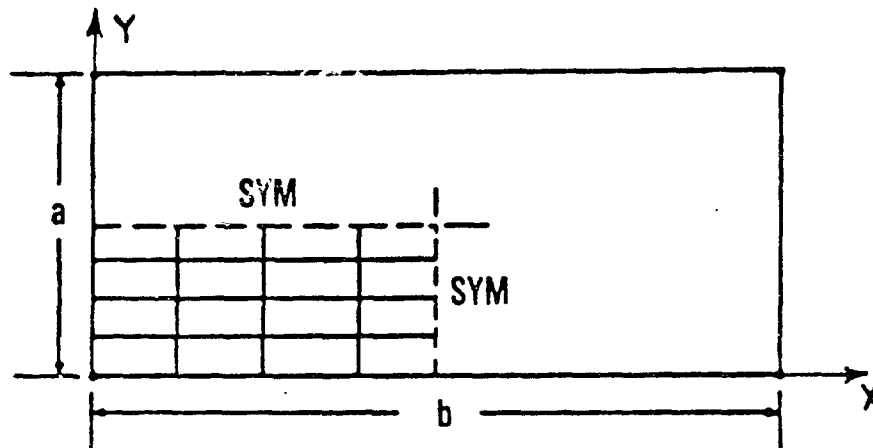


FIGURE 5

A finite element model of 1/4 of a rectangular plate is shown in Fig. 5. The length of the plate is 2 or 10, the width is 2, i.e. Aspect Ratios (AR) 1.0 and 5.0, and the thickness is .001. The material properties are given as  $E=1.7472 \times 10^6$  and  $\nu=0.3$ . The plate is subjected to two loading conditions and boundary conditions for each aspect ratio as follows:

CASE 1: Clamped Supports. Concentrated load of  $P = -4.0 \times 10^{-4}$  at the center of the plate.

CASE 2: Simple Supports. Uniform pressure load of  $q=10^{-4}$  over the plate.

Convergence is studied by varying the mesh size. Input data is given for a 6x6 mesh. Theoretical results for the lateral displacement at the center of the plate are given in Table 2.

| CASE 1   |          | CASE 2   |          |
|----------|----------|----------|----------|
| AR = 1.0 | AR = 5.0 | AR = 1.0 | AR = 5.0 |
| 5.60     | 7.23     | 4.062    | 12.97    |

TABLE 2

Example A4: The plate is modeled as a sandwich plate with composite face sheets and a honeycomb core. The thickness of the core is 2in. Two cases are considered. In Case 1 the MAT8 entries G1Z and G2Z are not specified. In Case 2 G1Z and G2Z are given the value  $4.0 \times 10^6$  psi.



Z DISPLACEMENT ( $\times 10^{-3}$ ) AT THE CENTER OF THE PLATE

| NASTRAN | MID2 ONLY | MID1, MID2 AND MID3 SPECIFIED |
|---------|-----------|-------------------------------|
| COSMIC  | -7.324    | -7.687                        |
| MSC     | -7.343    | -7.683                        |
| CSA     | -7.630    | -7.677                        |
| ASTROS  | -7.322    | -7.680                        |

EXAMPLE A1: QUAD4 - MID SPECIFICATIONS  
Metal Homogeneous Aluminum

Z DISPLACEMENT AT THE CENTER OF THE PLATE

| NASTRAN | FACE SHEETS AND CORE HAVE THE SAME MATERIAL PROP | CORE MATERIAL<br>$E = 2.5 \times 10^4$ , $G = 1.0 \times 10^4$ | CORE MATERIAL<br>$G = 1.0 \times 10^4$ |
|---------|--|--|--|
| COSMIC  | -0.000194  | -0.0150  | -0.0150                                |
| MSC     | -0.000193  | -0.0150  | -0.0150                                |
| CSA     | -0.000192  | -0.0150  | -0.0150                                |
| ASTROS  | -0.000193  | -0.0150  | -0.0150                                |

EXAMPLE A2: QUAD4 SANDWICH - HONEYCOMB PROPERTIES  
Sandwich Plate Isotropic Face Sheets

Z DISPLACEMENT ( $\times 10^{-2}$ ) AT THE CENTER OF THE PLATE

| NASTRAN | NO G1Z, G2Z | G1Z = G2Z = $4.0 \times 10^5$ |
|---------|-------------|-------------------------------|
| COSMIC  | -.803       | -1.071                        |
| MSC     | -.838       | -1.074                        |
| CSA     | -.945       | -1.029                        |
| ASTROS  | -.831       | -1.071                        |

**EXAMPLE A3: QUAD4 - SHEAR - G1Z, G2Z SPECIFIED**  
Composite Solid Plate

Z DISPLACEMENT ( $\times 10^{-3}$ ) AT THE CENTER OF THE PLATE

| NASTRAN | NO G1Z, G2Z | G1Z = G2Z = $4.0 \times 10^5$ |
|---------|-------------|-------------------------------|
| COSMIC  | -.199       | -2.691                        |
| MSC     | -6.464      | -6.526                        |
| CSA     | -5.067      | -5.109                        |
| ASTROS  | -.173       | -6.531                        |

**EXAMPLE A4: QUAD4 - SANDWICH - COMPOSITE FACE SHEETS**  
Composite Sandwich Plate with Honeycomb Core

## APPENDIX B: SAMPLE PROBLEMS

# QUAD4 SEMINAR

## APPENDIX B

### SAMPLE PROBLEMS

| <u>PROBLEM<br/>NUMBER</u> | <u>DESCRIPTION</u>  | <u>PAGE<br/>NUMBER</u> |
|---------------------------|---|------------------------|
| 1*                        | PATCH TEST FOR PLATES   | 83                     |
| 2                         | STRAIGHT CANTILEVER BEAM  | 85                     |
| 3                         | CURVED BEAM   | 86                     |
| 4                         | TWISTED BEAM  | 87                     |
| 5                         | RECTANGULAR PLATE   | 88                     |
| 6                         | SCORDELIS-LO ROOF   | 89                     |
| 7                         | SPHERICAL SHELL   | 90                     |
| 8*                        | COMPOSITE PLATE - PURE TWIST LOADING  | 91                     |
| 9                         | COMPOSITE PLATE - UNIFORM PRESSURE LOADING  | 92                     |
| 10                        | OPEN COMPOSITE TUBE   | 93                     |
| 11                        | STRAIGHT TEAM TEST - SIMULATION OF EQUIVALENT<br>ISOTROPIC PROPERTIES. LAMINATE CONFIGURATION<br>0/0/0/0. | 94                     |
| 12                        | COMPOSITE SHELL ROOF MODEL  | 96                     |
| 13                        | COMPOSITE RECTANGULAR PLATE   | 97                     |
| 14                        | COMPOSITE SANDWICH PLATE  | 99                     |

\* Problems 1 through 7 reference isotropic material and are defined in the MSC/NASTRAN APPLICATION MANUAL, SECTION 5.

+ Problems 8 through 12 are provided by UAI/NASTRAN.

## B.1 PROBLEM DESCRIPTIONS

## PROBLEM #1

## PATCH TEST

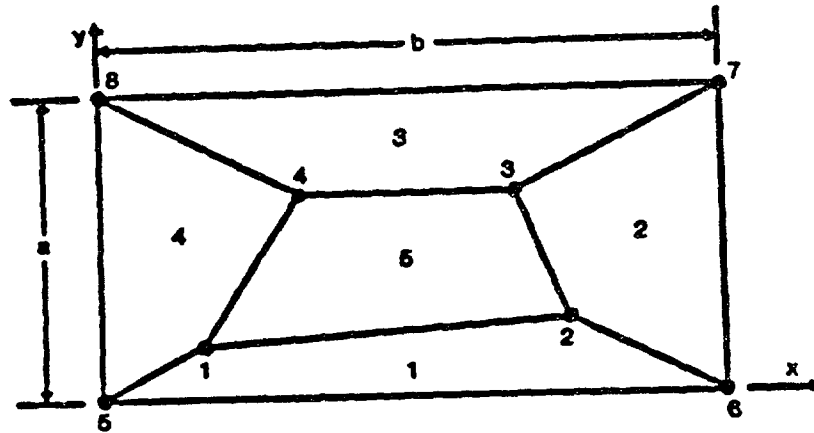


FIGURE 1

A model of a patch test for plates is shown in Fig. 1. The length of the plate is .24, the width is .12, and the thickness is .001. The location of the inner nodes is given in Table 1. The material properties are given as  $E=1.0 \times 10^6$  and  $\nu=0.25$ . Displacement boundary conditions are specified for the following cases:

## CASE 1. Membrane Plate

$$u = 10^{-3}(x+y/2)$$

$$v = 10^{-3}(y+x/2)$$

## CASE 2. Bending Plate

$$w = 10^{-3}(x^2+xy+y^2)/2$$

$$\theta_x = \frac{\partial w}{\partial y} = 10^{-3}(y+x/2)$$

$$\theta_y = -\frac{\partial w}{\partial x} = 10^{-3}(-x-y/2)$$

The theoretical solution for Case 1 is given by

$$\epsilon_x = \epsilon_y = \gamma = 10^{-3}; \sigma_x = \sigma_y = 1333.; \tau_{xy} = 400.$$

The theoretical solution for Case 2 is given by

Bending moments per unit length:

$$m_x = m_y = 1.111 \times 10^{-7}, m_{xy} = 10^{-7}$$

Surface stresses:

$$\sigma_x = \sigma_y = \pm .667; \tau_{xy} = \pm .200$$

| LOCATION OF INNER NODES |     |     |
|-------------------------|-----|-----|
| NODE                    | X   | Y   |
| 1                       | .04 | .02 |
| 2                       | .18 | .03 |
| 3                       | .16 | .08 |
| 4                       | .08 | .08 |

TABLE 1

## PROBLEM #2

## STRAIGHT CANTILEVERED BEAM

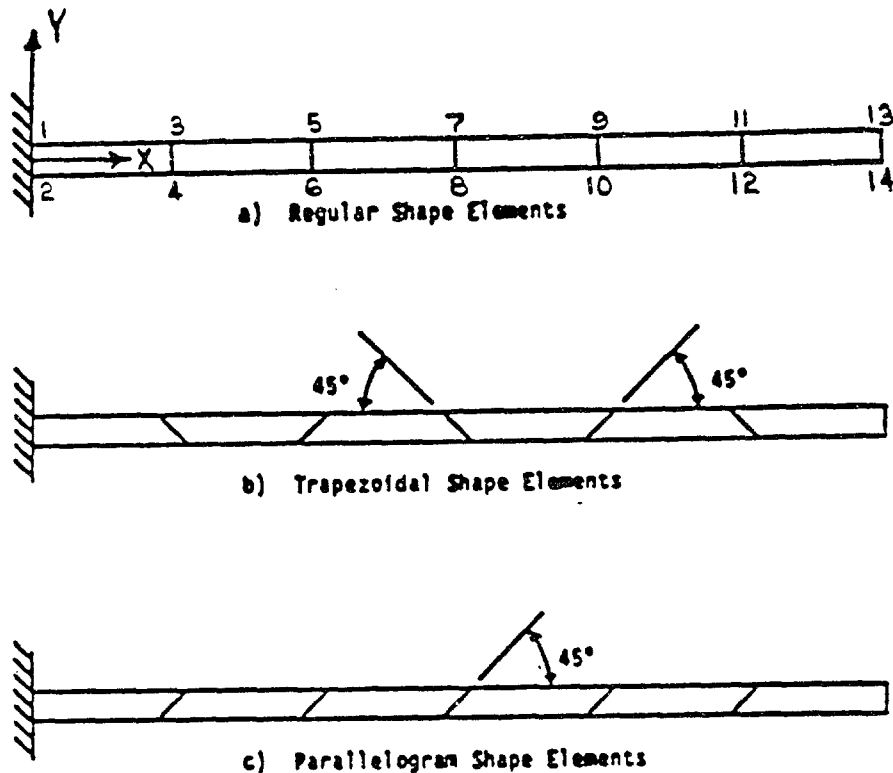


FIGURE 2

A finite element model of a straight cantilevered beam with rectangular, trapezoidal and parallelogram shape elements is shown in Fig. 2 (plane view). The length of the beam is 6, the width is 0.2 and the depth is 0.1. The beam has material properties  $E=1.0 \times 10^7$  and  $\nu=.30$ . All the elements have equal volume. The beam is subjected to 4 loading conditions as follows:

CASE 1: A unit force is applied at the tip in the X direction. (Extension)

CASE 2: A unit force is applied at the tip in the Y direction.  
(In-plane shear)

CASE 3: A unit force is applied at the tip in the Z direction.  
(Out-of-plane shear)

CASE 4: A unit force is applied at the tip in the  $\pm Z$  direction. (Twist)

The theoretical solution for the displacement at the tip in the direction of the load for Case 1 is  $3.0 \times 10^{-5}$ , for Case 2 0.1081, for Case 3 0.4321 and for Case 4  $.03208 \times 10^{-2}$ .



## PROBLEM #3

## CURVED BEAM

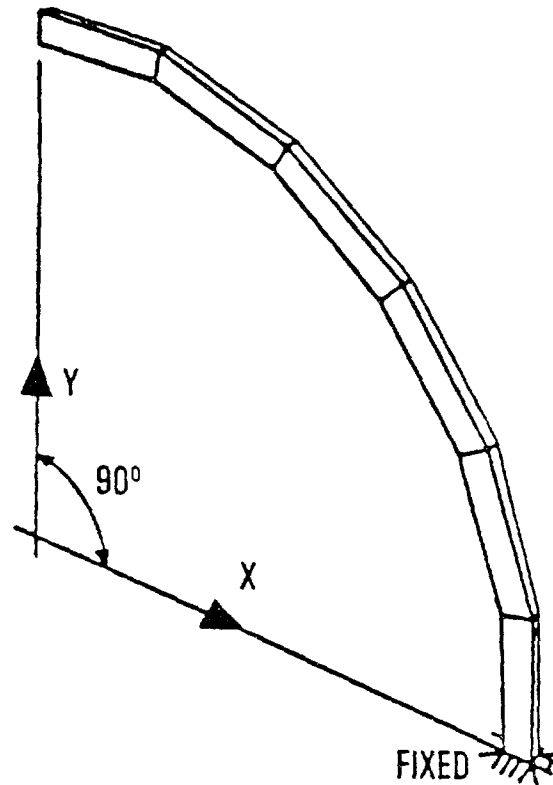


FIGURE 3

A finite element model of a curved cantilevered beam is shown in Fig. 3. The inner radius is 4.12, the outer radius is 4.32, the arc is  $90^\circ$ , and the thickness is 0.1. The beam has material properties  $E=1.0 \times 10^7$  and  $\nu=0.25$ . The beam is subjected to two loading conditions as follows:

CASE 1: A unit force is applied at the tip in the +Y direction.  
(In-plane shear)

CASE 2: A unit force is applied at the tip in the +Z direction.  
(Out-of-plane shear)

The theoretical solution for the deflection at the tip in the direction of the load for Case 1 is .08734 and for Case 2 is .5022.

PROBLEM #4

INPUT IS PROVIDED

TWISTED BEAM



FIGURE 4

A finite element model of a twisted beam is shown in Fig. 4. The length of the beam is 12, the width is 1.1 and the depth is .32. The beam is twisted  $90^\circ$  from root to tip. The material properties are given as  $E=29.0 \times 10^6$  and  $\nu=0.22$ , and the beam is subjected to two loading conditions as follows:

CASE 1: A unit force is applied at the tip in the +X direction.  
(In-plane shear)

CASE 2: A unit force is applied at the tip in the -Y direction.  
(Out-of-plane shear)

The theoretical solution for the deflection at the tip in the direction of the load for Case 1 is .005424 and for Case 2 is .001754.

## PROBLEM #5

## RECTANGULAR PLATE

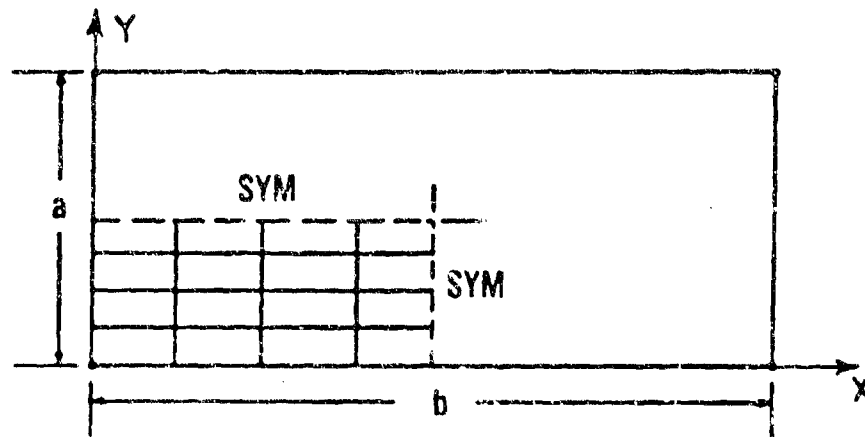


FIGURE 5

A finite element model of 1/4 of a rectangular plate is shown in Fig. 5. The length of the plate is 2 or 10, the width is 2, i.e. Aspect Ratios (AR) 1.0 and 5.0, and the thickness is .001. The material properties are given as  $E=1.7472 \times 10^6$  and  $\nu=0.3$ . The plate is subjected to two loading conditions and boundary conditions for each aspect ratio as follows:

CASE 1: Clamped Supports. Concentrated load of  $P = -4.0 \times 10^{-4}$  at the center of the plate.

CASE 2: Simple Supports. Uniform pressure load of  $q=10^{-4}$  over the plate.

Convergence is studied by varying the mesh size. Input data is given for a 6x6 mesh. Theoretical results for the lateral displacement at the center of the plate are given in Table 2.

| CASE 1   |          | CASE 2   |          |
|----------|----------|----------|----------|
| AR = 1.0 | AR = 5.0 | AR = 1.0 | AR = 5.0 |
| 5.60     | 7.23     | 4.062    | 12.97    |

TABLE 2

## PROBLEM 6

## SCORDELIS - LO ROOF

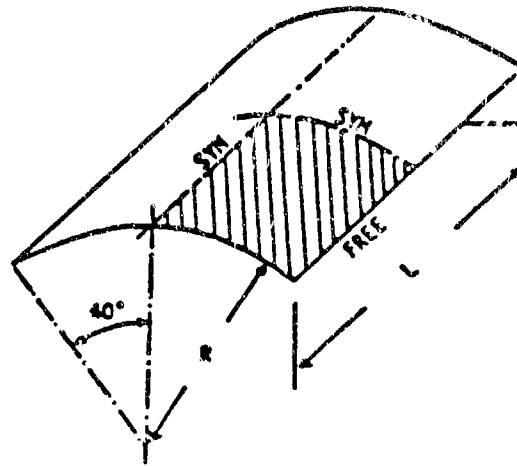


FIGURE 6

A model of a Scordelis-Lo Roof i.e. a singly curved shell, is shown in Fig. 6. The roof has a radius of 25', a length of 50', and a thickness of 0.25'. The material properties are given as  $E=4.32 \times 10^8$  lbs/ft<sup>2</sup> and  $\nu=0.0$ . The roof is loaded by its own weight, and the weight of the roof is 90 lbs/ft<sup>2</sup>. Only 1/4 of the roof is modeled. Symmetry boundary conditions are imposed on the interior edges and  $U_x=U_y=0$  on the curved edges. Convergence is studied by varying the mesh size. Input data is given for an 8x8 mesh. The theoretical solution for the midside vertical displacement is .3086'. A value of .3024' was used for normalization of the results.

Reference: Zienkiewicz, O.C., The Finite Element Method, Third Edition, McGraw-Hill Book Co, (UK) Limited, pp 349-351.

## PROBLEM #7

## SPHERICAL SHELL

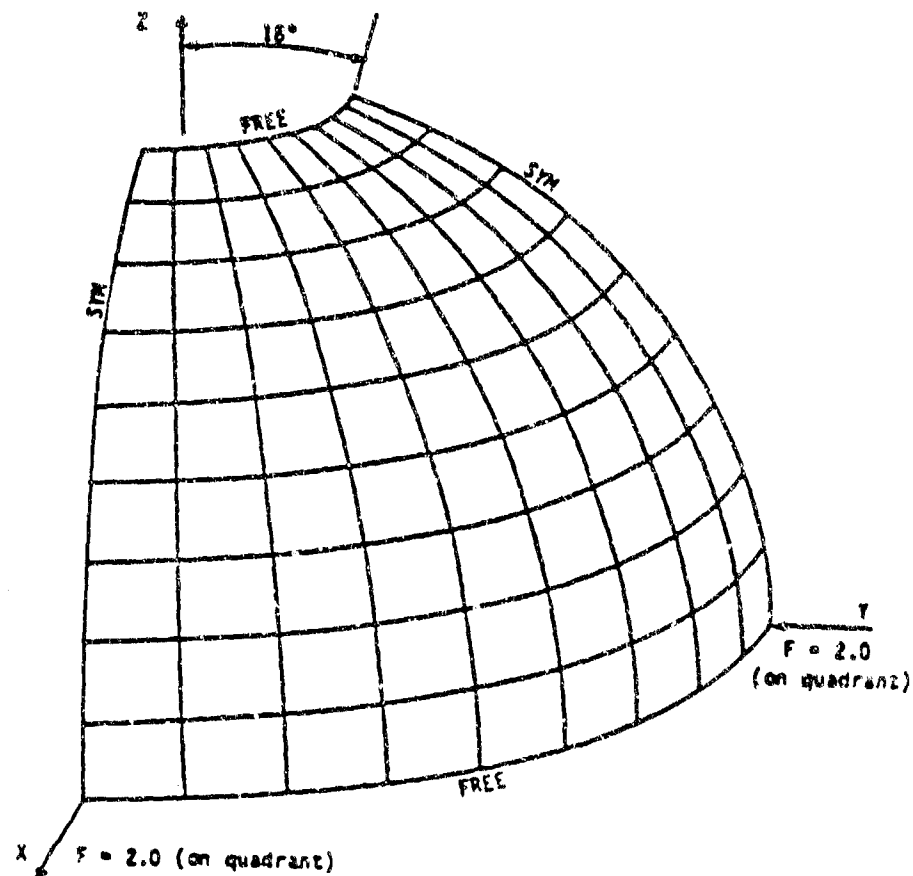


FIGURE 7

A model of a spherical shell, i.e. a doubly-curved shell, is shown in Fig. 7. The shell has a radius of 10 and a thickness of 0.4. The material properties are given as  $E=6.825 \times 10^6$  and  $\nu=0.3$ . The shell is subjected to concentrated forces as shown in Fig. 7. Only 1/4 of the shell is modeled and symmetry boundary conditions are imposed on two sides. In addition the \* node is constrained in the Z direction. Convergence is studied by varying the mesh size. Input and output are given for an 8x8 mesh. A theoretical lower bound for the radial deflection at the center in the case where the hole at the center is not present is .0924. A value of .0940 was used for normalization of the results.

Reference: Morley, L.S.D., and A.J. Morris, Conflict Between Finite Elements and Shell Theory, Royal Aircraft Establishment Report, London, 1978.

INPUT IS PROVIDED

PROBLEM #8

REGULAR SYMMETRIC CROSS-PLY LAMINATE

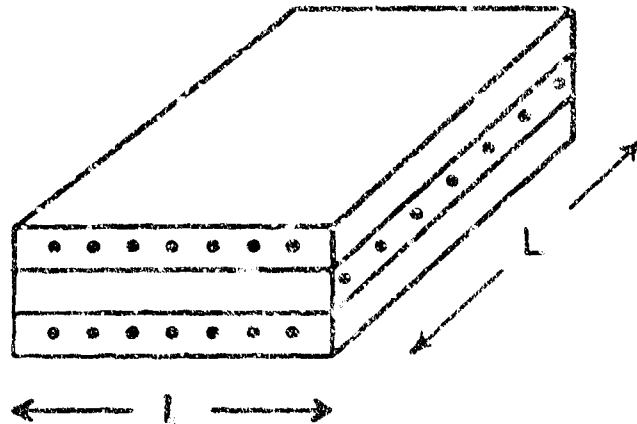


FIGURE 8

A composite square plate of length 5 modeled as a symmetric cross-ply laminate  $[0, 90, 0]$  with four elements is shown in Fig. 8. The thickness of each ply is .02222. The material properties are given as  $E_1=2.0 \times 10^6$ ,  $E_2=5.0 \times 10^5$ ,  $\nu_{12}=0.25$  and  $G_{12}=2.5 \times 10^5$  and are the same for each ply. Three corners of the plate are pinned and a unit force in the  $-Z$  direction is applied at the free corner to simulate a pure twist loading. The theoretical solution for the  $Z$  deflection at the free corner is  $-3.750 \times 10^{-3}$ . Theoretical results are also given for  $\tau$  for the element containing the free corner in Table 3.

| LAYER | $\tau$ |
|-------|--------|
| 1     | -50.0  |
| 2     | 0.0    |
| 3     | 50.0   |

TABLE 3

INPUT IS PROVIDED

PROBLEM #9

REGULAR SYMMETRIC CROSS-PLY LAMINATE

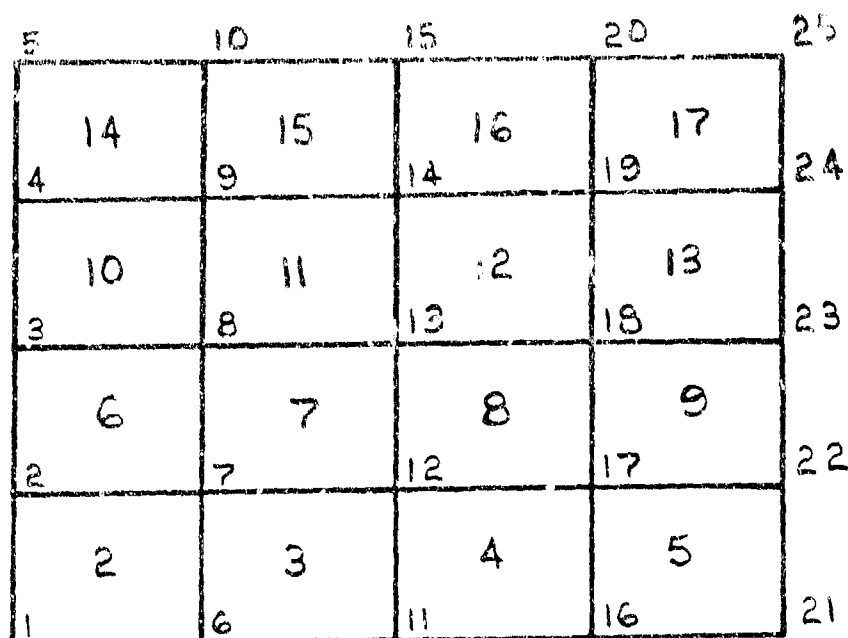


FIGURE 9

A quarter model of a composite square plate modeled as a symmetric cross-ply laminate is shown in Fig. 9. The length of each side is 1.0, and the thickness of each ply is .000666. The material properties are given as  $E_1=2.0 \times 10^7$ ,  $E_2=5.0 \times 10^5$ ,  $\nu_{12}=0.25$  and  $G_{12}=2.5 \times 10^5$  and are the same for each ply. The full plate is simply supported and is subjected to a uniform pressure load of  $-1.0 \times 10^{-4}$ . The finite element model contains 25 nodes and 16 elements. The theoretical solution for the  $z$  deflection at Grid 25 (center of the plate) is  $-1.836 \times 10^{-3}$ . Theoretical results are also given for the stresses in layers 1 and 3 in element 17 in Table 4.

| $\sigma_1$ | $\sigma_2$ | $\tau_{12}$ |
|------------|------------|-------------|
| 59.6       | 1.8        | -.06        |

TABLE 4

## PROBLEM #10

COMPOSITE OPEN TUBE

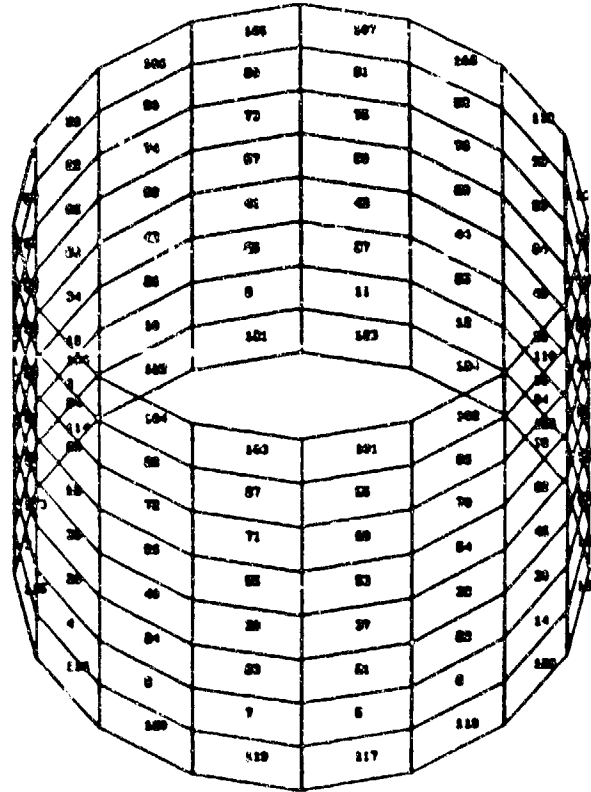


FIGURE 10

A finite element model of a composite open tube modeled with the symmetric lay-up  $[45, -45, 0, 90, 90, 0, -45, 45]$  is shown in Fig. 10. The model contains 144 nodes and 128 elements. The length of the tube is 80, the radius is 50, and the thickness of each layer is .24. The material properties are given as  $E_1=73.8 \times 10^3$ ,  $E_2=3.75 \times 10^3$ ,  $\nu_{12}=0.4$ , and  $G_{12}=1.74 \times 10^3$ . The tube is subjected to a uniform pressure load of 10.5. The theoretical solution for the hoop loading,  $F_Y$ , for element 8 is 525. Results from MSC/NASTRAN are given in Table 5 for the layer stresses in element 8.

| LAYER | $\sigma_1$ | $\sigma_2$ | $\tau_{12}$ |
|-------|------------|------------|-------------|
| 1     | 2.519E2    | 1.740E1    | 2.273E1     |
| 2     | 2.494E2    | 1.748E1    | -2.273E1    |
| 3     | -2.257E2   | 3.288E1    | 1.108E-2    |
| 4     | 7.268E2    | 2.651E0    | 2.221E-2    |
| 5     | 7.269E2    | 2.657E0    | 5.551E-2    |
| 6     | -2.255E2   | 3.231E1    | -8.881E-2   |
| 7     | 2.535E2    | 1.741E1    | -2.274E1    |
| 8     | 2.478E2    | 1.760E1    | 2.274E1     |

TABLE 5



## PROBLEM #11

## STRAIGHT BEAM TEST

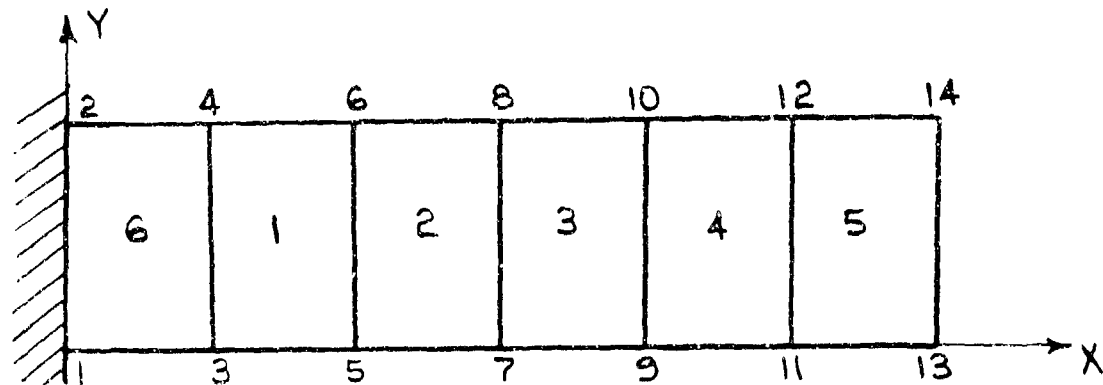


FIGURE 11

A finite element model of a cantilevered beam modeled as a laminate configuration  $[0, 0, 0, 0]$  in a simulation of equivalent isotropic properties is shown in Fig. 11. The length of the beam is 6, and the thickness of each layer is .25. The material properties are given as  $E=10.0 \times 10^6$  and  $\nu=.3$ . The beam is subjected to two loading conditions as follows:

CASE 1: A unit force is applied at the tip in the X direction. (Extension)

CASE 2: A unit force is applied at the tip in the Z direction.

For Case 1 the theoretical solution for the X deflection at Grids 13 and 14 is  $3.0 \times 10^{-5}$ . For Case 2 the theoretical solution for the Z deflection at Grids 13 and 14 is .4320. Theoretical results are given for the bending moment distribution from the free end to the cantilevered end in Table 6.

| ELEMENT NO. | $M_x$ |
|-------------|-------|
| 5           | 2.50  |
| 4           | 7.50  |
| 3           | 12.50 |
| 2           | 17.50 |
| 1           | 22.50 |
| 6           | 27.50 |

TABLE 6

For Case 2 results from MSC/NASTRAN are given in Table 7 for the direct layer bending stress in element 6.

| LAYER NO. | $\sigma_1$ |
|-----------|------------|
| 1         | 1.238E4    |
| 2         | 4.126E3    |
| 3         | -4.126E3   |
| 4         | -1.238E4   |

TABLE 7

## PROBLEM #12

## COMPOSITE SHELL ROOF

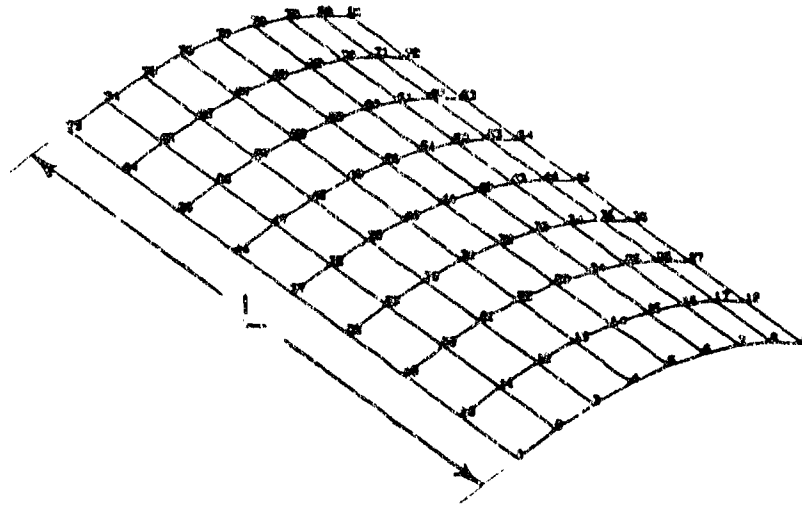


FIGURE 12

A finite element model of a composite shell roof modeled with the symmetric lay-up  $[45, -45, 15, -15, -15, 15, -45, 45]$  is shown in Fig. 12. The length and radius of the shell are 25, and the thickness of each layer is .03125. The material properties are given as  $E_1=2.0 \times 10^8$ ,  $E_2=0.5 \times 10^8$ ,  $\nu_{12}=0.25$ ,  $G_{12}=2.5 \times 10^6$ , and  $G_{17}=G_{27}=2.5 \times 10^6$ . The shell is subjected to a uniform pressure of 90.0. Results from MSC/NASTRAN are given in Table 8 for the radial deflection at selected nodes.

| GRID | T1      |
|------|---------|
| 34   | -1.0662 |
| 35   | -1.3441 |
| 36   | -1.6074 |
| 43   | -1.3267 |
| 44   | -1.6739 |
| 45   | -2.0079 |

TABLE 8

PROBLEM #13

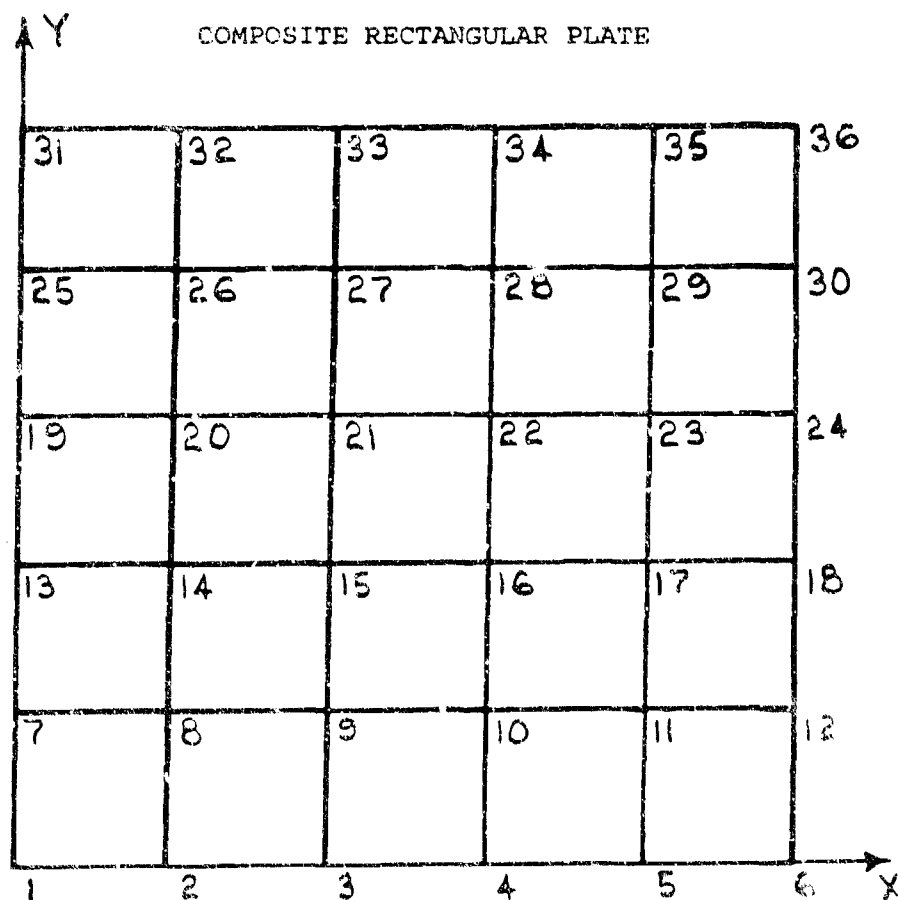


FIGURE 13

A composite rectangular plate modeled with the symmetric lay-up  $[45, -45, 90, 0, 0, 90, -45, 45]$  is shown in Fig. 13. The model contains 36 nodes and 25 elements. The length of the plate is 20, the width is 15, and the thickness of each layer is .0236. The material properties are given as  $E_1=18.5 \times 10^6$ ,  $E_2=1.6 \times 10^6$ ,  $\nu_{12}=0.25$ ,  $G_{12}=.65 \times 10^6$  and  $G_{12}=G_{22}=4.0 \times 10^6$ . Four variations of the plate are considered.

CASE 1: The plate is modeled as a membrane subjected to a compression load in the -X direction at the  $X=15.0$  edge.

CASE 2: The plate is modeled with bending properties and subjected to a uniform pressure load of 2 in the -Z direction.

CASE 3: The plate is modeled in membrane and bending and is subject to the loads defined in Case 1 and Case 2.

CASES 1 through 3 are defined for a static analysis, RF #1.

CASE 4: Normal modes analysis using RF #3. The first ten natural frequencies are found.

# PROBLEM #14

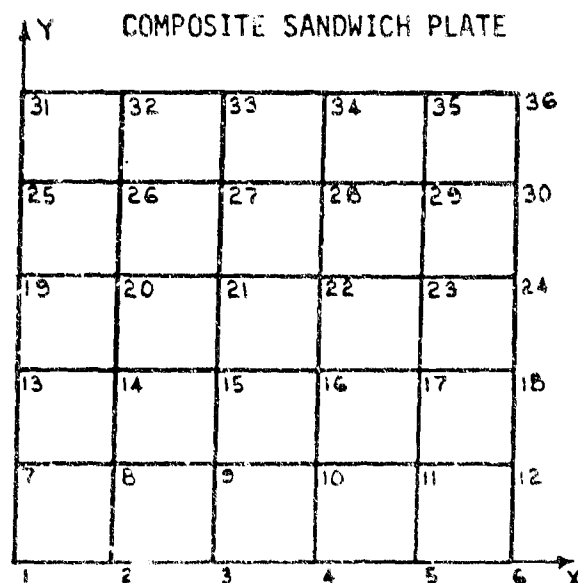
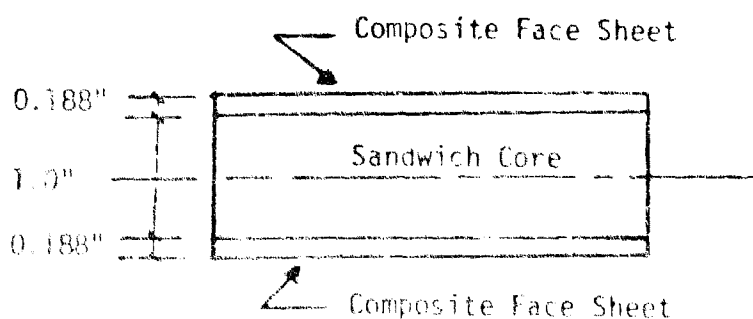


FIGURE 14

A sandwich plate with an isotropic core and composite skins modeled with the symmetric lay-up  $[45, -45, 90, 0, 0, 90, -45, 45]$  is shown in Fig 14. The model contains 35 nodes and 25 elements. The length of the plate is 20, the width is 15, the thickness of each layer is .0236, and the thickness of the core is 1. The material properties of the composite skin are given as  $E_1=18.5 \times 10^6$ ,  $E_2=1.6 \times 10^6$ ,  $\nu=0.25$  and  $G_{12}=.65 \times 10^6$ ,  $G_{12}=G_{22}=4.0 \times 10^5$ . The material properties of the core are given as  $G=3.0 \times 10^4$ . The plate is subjected to a uniform pressure load of 2.

## ELEMENT DEFINITION

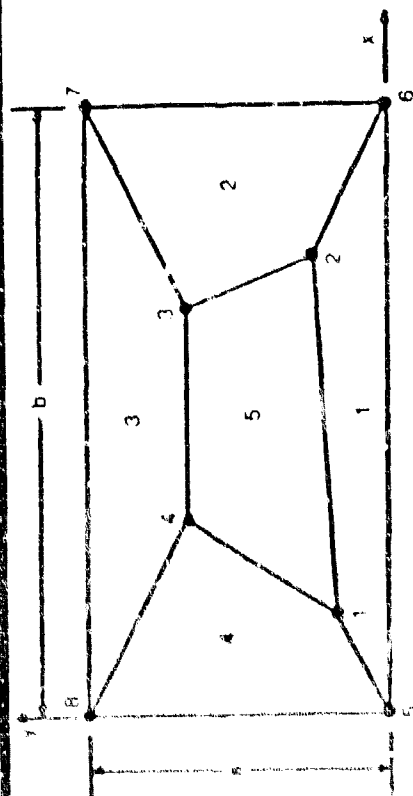


## B.2 RESULTS AND COMPARISONS



# PATCH TEST

## PROBLEM #1



$$a = .12 \quad b = .24 \quad t = .001$$

$$E = 1.0E+6$$

$$\nu = 0.25$$

### MEMBRANE PLATE

$$B.C. \begin{cases} u = 10^{-3}(x + y/2) \\ v = 10^{-3}(y + x/2) \end{cases}$$

### BENDING PLATE

$$B.C. \begin{cases} W = 10^{-3}(x^2 + xy + y^2)/2 \\ \theta_x = 10^{-3}(y + x/2) \\ \theta_y = 10^{-3}(-x - y/2) \end{cases}$$

LOADING

CONSTANT STRESS  
CONSTANT CURVATURE

MAXIMUM ERROR IN  $\sigma_x$

| QUAD2 | COSMIC | QUAD4 | MSC | THEORY $\sigma_x$ |
|-------|--------|-------|-----|-------------------|
| 0.0   | 0.0    | 0.0   | 0.0 | 1333.             |
| 33.1% | 0.0    | 0.0   | 0.0 | $\pm .667$        |

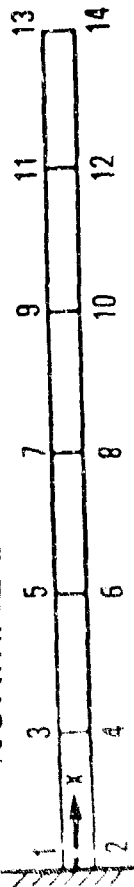




# STRAIGHT CANTILEVER BEAM

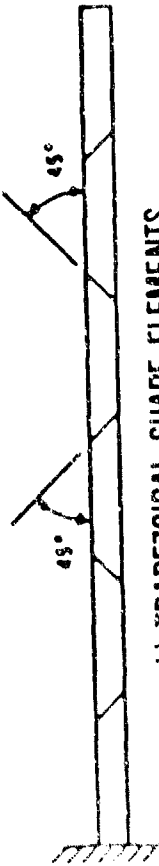
PROBLEM #2

## NORMALIZED TIP DEFLECTION IN THE DIRECTION OF THE LOAD

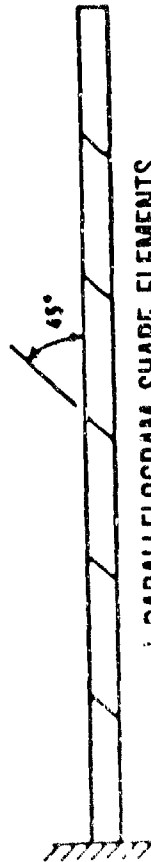


$L=6.0$   $W=0.2$   $D=0.1$   
 $E=1.0E7$   $\nu=.30$   
 UNIT FORCES AT THE TIP

a) RECTANGULAR SHAPE ELEMENTS



b) TRAPEZOIDAL SHAPE ELEMENTS



c) PARALLELOGRAM SHAPE ELEMENTS

EXTENSION  
 IN PLANE SHEAR  
 OUT-OF-PLANE SHEAR  
 TWIST

RECTANGULAR

| QUAD2 | QUAD4<br>COSMIC | MSC  |
|-------|-----------------|------|
| .992  | .995            | .995 |
| .032  | .904            | .904 |
| .971  | .985            | .986 |
| .566  | .959            | .902 |

TRAPEZOIDAL

| QUAD2 | QUAD4<br>COSMIC | MSC  |
|-------|-----------------|------|
| .992  | .996            | .996 |
| .017  | .070            | .071 |
| .965  | .981            | .968 |
| .631  | .916            | .913 |

EXTENSION  
 IN PLANE SHEAR  
 OUT-OF-PLANE SHEAR  
 TWIST

PARALLELOGRAM

| QUAD2 | QUAD4<br>COSMIC | MSC  |
|-------|-----------------|------|
| .992  | .991            | .996 |
| .013  | .078            | .078 |
| .960  | .987            | .976 |
| .598  | .881            | .904 |

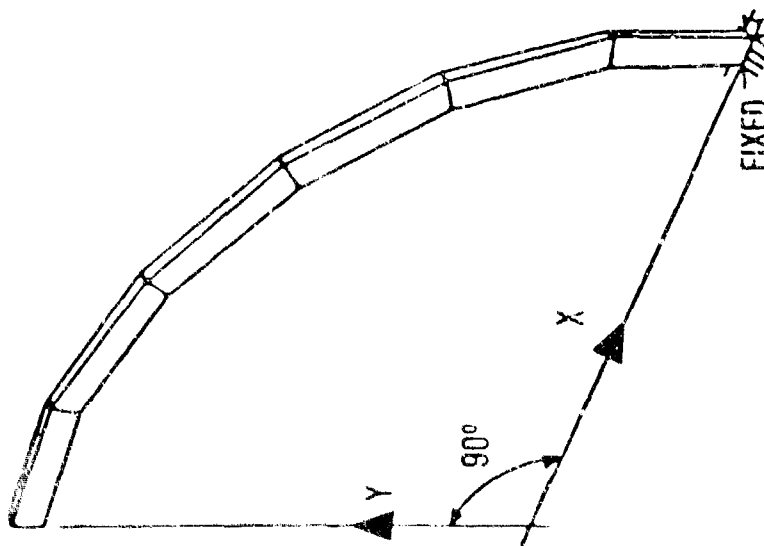
EXTENSION  
 IN-PLANE SHEAR  
 OUT-OF-PLANE SHEAR  
 TWIST



# CURVED BEAM

PROBLEM #3

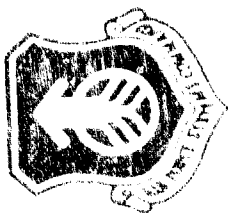
## NORMALIZED TIP DISPLACEMENT IN THE DIRECTION OF THE LOAD



INNER RADIUS = 4.12  
OUTER RADIUS = 4.32  
THICKNESS = .1  
 $E = 1.0E7$   
 $\nu = .25$

| QUAD2 | QUAD4<br>COSMIC | MSC  |
|-------|-----------------|------|
| .625  | .833            | .833 |
| .596  | .950            | .923 |

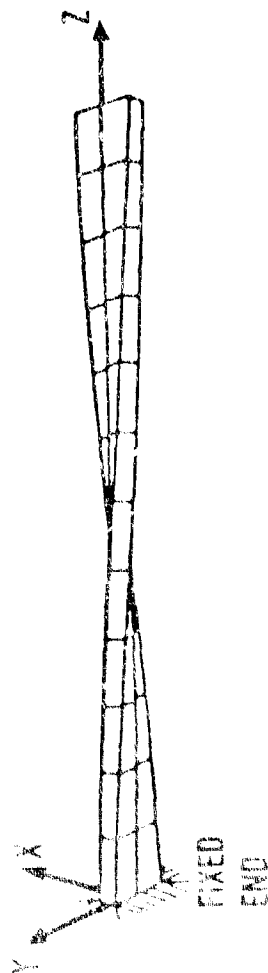
IN-PLANE (VERTICAL)  
OUT-OF-PLANE



# TWISTED BEAM

PROBLEM #A

## NORMALIZED TIP DEFLECTION IN THE DIRECTION OF THE LOAD



$L = 12.0$     $W = 1.1$     $D = .32$   
 $\text{TWIST} = 90^\circ$  (ROOT TO TIP)  
 $E = 29.0E6$   
 $\nu = 0.22$

UNIT FORCES AT THE TIP

| QUAD2 | QUAD4  |      |
|-------|--------|------|
|       | COSMIC | MSC  |
| 98.46 | .995   | .993 |
| 226.4 | .985   | .985 |

IN-PLANE  
 OUT-OF-PLANE

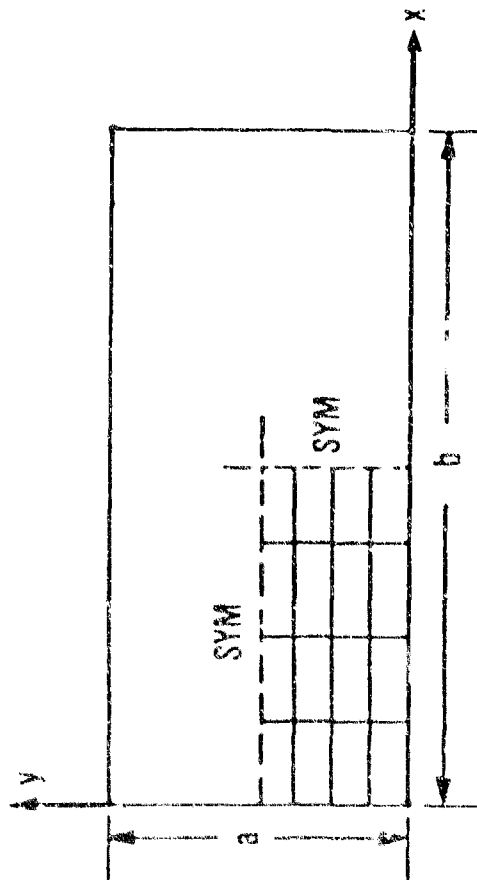


# RECTANGULAR PLATE CLAMPED SUPPORTS

PROBLEM #5

## NORMALIZED LATERAL DEFLECT

THE PLATE



$a = 2.0$     $b = 2.0$  OR  $10.0$   
 THICKNESS =  $0.001$   
 $E = 1.7472E7$   
 $\nu = 0.3$   
 CENTRAL LOAD =  $4.0E4$

ASPECT RATIO = 1.0

| MESH | QUAD2 | QUAD4<br>COSMIC | MSC   |
|------|-------|-----------------|-------|
| 2x2  | .983  | .997            | .938  |
| 4x4  | 1.012 | 1.034           | 1.014 |
| 6x6  | 1.010 | 1.026           | 1.016 |
| 8x8  | 1.009 | 1.019           | 1.014 |

ASPECT RATIO = 5.0

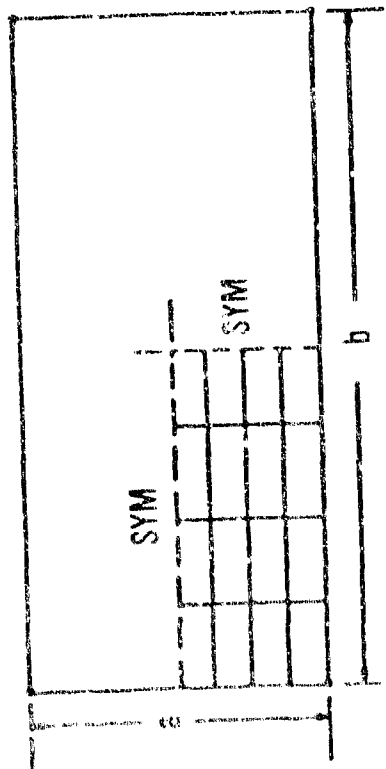
| MESH | QUAD2 | QUAD4<br>COSMIC | MSC  |
|------|-------|-----------------|------|
| 2x2  | .333  | .773            | .519 |
| 4x4  | .513  | 1.028           | .863 |
| 6x6  | .639  | 1.058           | .940 |
| 8x8  | .725  | 1.057           | .972 |



# RECTANGULAR PLATE SIMPLE SUPPORTS

PROBLEM #5

## NORMALIZED LATERAL DEFLECTION AT THE CENTER



$a = 2.0$     $b = 2.0$  OR  $10.0$   
 THICKNESS =  $0.001$   
 $E = 1.7472E7$   
 $\nu = 0.3$   
 UNIFORM PRESSURE  $q = 1.0E-4$

ASPECT RATIO = 1.0

| MESH | QUAD2 | QUAD4<br>COSMIC | MSC   |
|------|-------|-----------------|-------|
| 2x2  | .975  | 1.030           | .981  |
| 4x4  | .998  | 1.013           | 1.004 |
| 6x6  | 1.001 | 1.009           | 1.003 |
| 8x8  | 1.002 | 1.006           | 1.002 |

ASPECT RATIO = 5.0

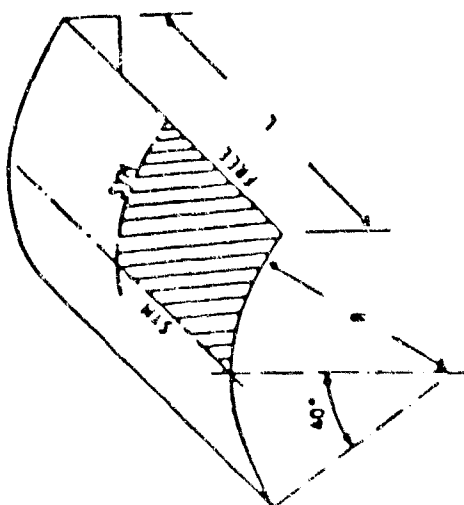
| MESH | QUAD2 | QUAD4<br>COSMIC | MSC   |
|------|-------|-----------------|-------|
| 2x2  | 1.005 | .913            | .999  |
| 4x4  | 1.014 | .991            | .995  |
| 6x6  | 1.007 | .996            | 1.000 |
| 8x8  | 1.005 | .998            | 1.001 |



# SCORDELIS - LO - ROOF

PROBLEM #6

## NORMALIZED VERTICAL DEFLECTION AT THE MIDPOINT OF THE FREE EDGE



RADIUS = 25.0 FT  
LENGTH = 50.0 FT  
THICKNESS = 0.25 FT  
 $E = 4.32E8 \text{ LBS / FT}^2$   
 $\nu = 0.0$

LOADED BY ITS OWN WEIGHT - 90 LBS / FT<sup>2</sup>  
 $U_x = U_z = 0 \text{ ON CURVED EDGES}$

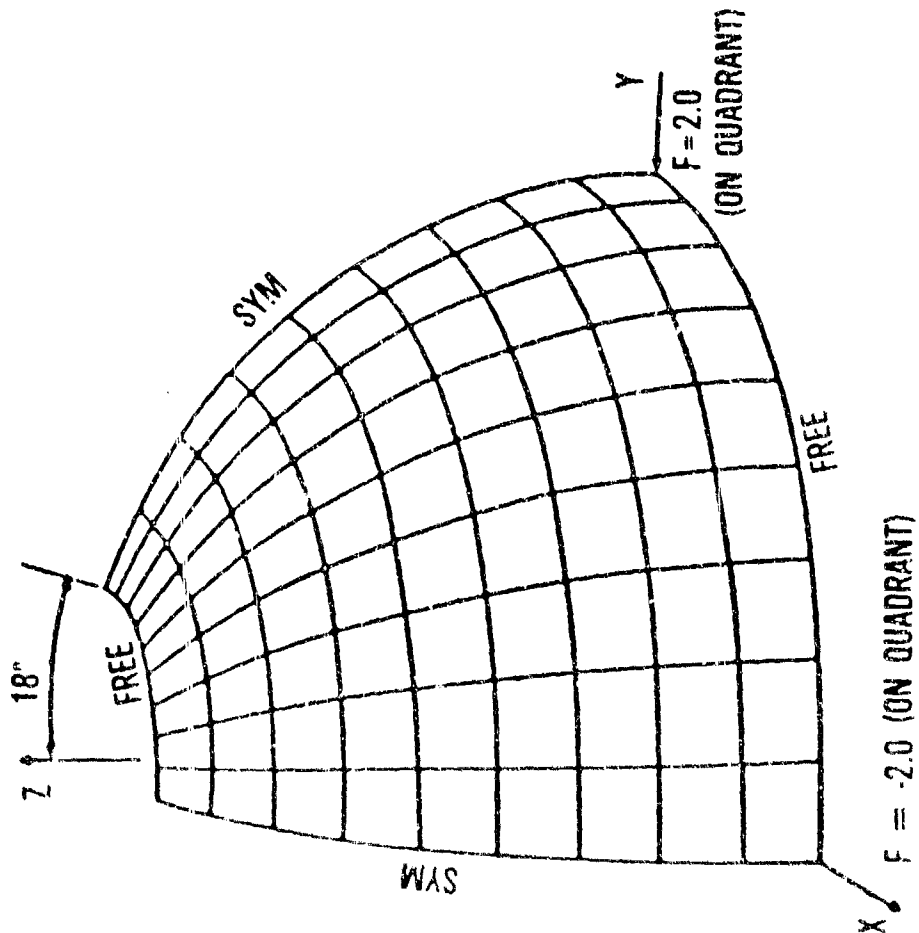
| MESH  | QUAD2 | QUAD4  |       |
|-------|-------|--------|-------|
|       |       | COSMIC | MSC   |
| 2x2   | .900  | 1.419  | 1.376 |
| 4x4   | .691  | 1.047  | 1.050 |
| 6x6   | .791  | .999   | 1.018 |
| 8x8   | .859  | .978   | 1.008 |
| 10x10 | .896  | .966   | 1.004 |



# SPHERICAL SHELL

PROBLEM #7

## NORMALIZED RADIAL DEFLECTION AT THE CENTER



RADIUS = 10.0  
 THICKNESS = .04  
 $E = 6.825E7$   
 $\nu = 0.3$

| MESH  | QUAD2 | QUAD4<br>COSMIC | MSC   |
|-------|-------|-----------------|-------|
| 2x2   | .928  | 1.051           | .972  |
| 4x4   | .990  | 1.054           | 1.024 |
| 6x6   | .990  | 1.030           | 1.013 |
| 8x8   | .986  | 1.015           | 1.005 |
| 10x10 | .984  | 1.007           | 1.001 |
| 12x12 | .982  | 1.003           | .998  |



# SUMMARY OF TEST RESULTS

| TEST DESCRIPTION            | ELEMENT<br>IN-<br>PLANE | LOADING<br>OUT-OF-<br>PLANE | ELEMENT<br>SHAPE | COSMIC<br>QUAD 2 | COSMIC<br>QUAD 4 | MSCI<br>QUAD 4 |
|-----------------------------|-------------------------|-----------------------------|------------------|------------------|------------------|----------------|
| 1. PATCH TEST               | X                       |                             | IRREGULAR        | A                | A                | A              |
| 2. PATCH TEST               |                         | X                           | IRREGULAR        | D                | A                | A              |
| 3. STRAIGHT BEAM, EXTENSION | X                       |                             | ALL              | A                | A                | A              |
| 4. STRAIGHT BEAM, BENDING   | X                       |                             | REGULAR          | F                | B                | B              |
| 5. STRAIGHT BEAM, BENDING   | X                       |                             | IRREGULAR        | F                | F                | F              |
| 6. STRAIGHT BEAM, BENDING   |                         | X                           | REGULAR          | B                | A                | A              |
| 7. STRAIGHT BEAM, BENDING   |                         | X                           | IRREGULAR        | B                | A                | B              |
| 8. STRAIGHT BEAM, TWIST     |                         |                             | ALL              | D                | B                | B              |
| 9. CURVED BEAM              | X                       |                             | REGULAR          | F                | C                | C              |
| 10. CURVED BEAM             |                         | X                           | REGULAR          | D                | B                | B              |
| 11. TWISTED BEAM            | X                       | X                           | REGULAR          | F                | A                | A              |
| 12. RECTANGULAR PLAT (5x5)  |                         | X                           | REGULAR          | A                | A                | A              |
| 13. SCORDELIS-LO ROOF (5x5) | X                       | X                           | REGULAR          | D                | B                | B              |
| 14. SPHERICAL SHELL (9x9)   | X                       | X                           | REGULAR          | A                | A                | A              |

MSCINASTRAM IS A SERVICE AND TRADEMARK OF MACNEAL-SCHWENDER CORP

GRADE SCORES ARE DEFINED BY:

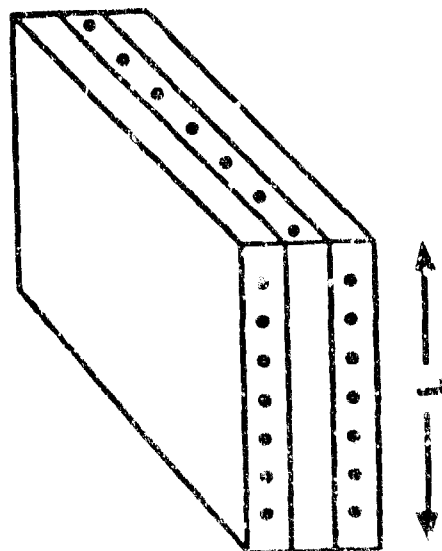
- A. ERROR LESS THAN 2%
- B. ERROR BETWEEN 2% AND 10%
- C. ERROR BETWEEN 10% AND 20%
- D. ERROR BETWEEN 20% AND 50%
- F. ERROR EXCEEDS 50%





# LAMINATED COMPOSITE PLATE — PURE TWIST LOADING

PROBLEM #8



REGULAR SYMMETRIC  
CROSS-PLY LAMINATE

A SQUARE PLATE OF A 4X4 MESH WITH THREE  
CORNERS PINNED AND A TRANSVERSE POINT  
LOAD AT THE FREE CORNER TO SIMULATE A  
PURE TWIST LOADING

$$I_1 = I_2 = I_3 = .02222$$

$$E_1 = 2.0E7$$

$$E_2 = 5.0E5$$

$$\nu_{12} = 0.25$$

$$G_{12} = 2.5E5$$

$$L = 5.0$$

$$\theta_1 = \theta_3 = 0.0$$

$$\theta_2 = 90.0$$

## COMPARISON OF T3 DEFLECTION AT GRID 1

| COSMIC/NASTRAN | MSC/NASTRAN | THEORETICAL |
|----------------|-------------|-------------|
| -3.758E-2      | -3.769E-2   | -3.750E-3   |
| (1.002)*       | (1.005)     |             |

## COMPARISON OF TAU FOR ELEMENT 1, ALL LAYERS

| COSMIC/NASTRAN | MSC/NASTRAN | THEORETICAL |
|----------------|-------------|-------------|
| PLY 1 -5.0E1   | -5.0E1      | -5.0E1      |
| PLY 2 0.0E1    | 0.0         | 0.0         |
| PLY 3 5.0E1    | 5.0E1       | 5.0E1       |

\*NORMALIZED VALUE

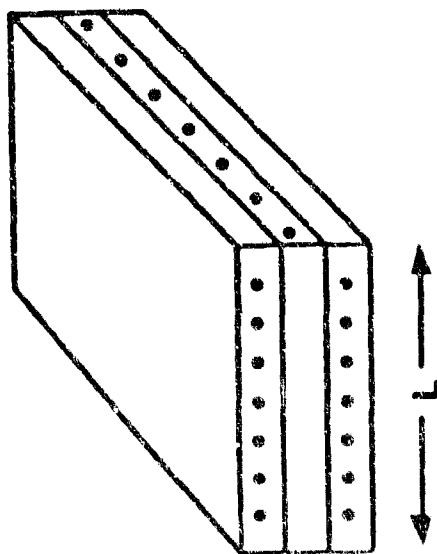


# LAMINATED COMPOSITE PLATE

## PROBLEM #9

$$\begin{aligned}
 I_1 = I_2 = I_3 &= .000666 \\
 E_1 &= 2.0E7 \\
 E_2 &= 5.0E5 \\
 \nu_{12} &= 0.25 \\
 G_{12} &= 2.5E5
 \end{aligned}$$

$$\begin{aligned}
 L &= 1.0 \\
 P &= 1.0 \text{ E-4} \\
 \theta_1 &= \theta_2 = 0.0 \\
 \theta_2 &= 90.0
 \end{aligned}$$



REGULAR SYMMETRIC  
 CROSS-PLY LAMINATE  
 SIMPLY SUPPORTED  
 UNIFORM PRESSURE LOAD

### COMPARISON OF T3 DEFLECTION AT GRID 25

| COSMIC/NASTRAN | MSC/NASTRAN | THEORETICAL |
|----------------|-------------|-------------|
| -1.852E-3      | -1.739E-3   | -1.836E-3   |
| (1.006)*       | (.947)      |             |

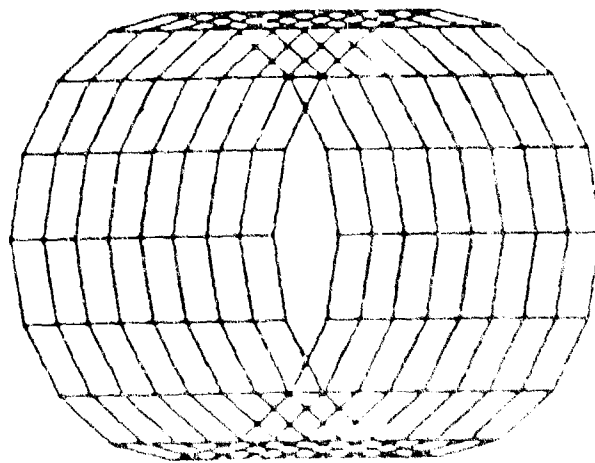
| STRESS RESULTS |           |        |
|----------------|-----------|--------|
| LAYER 1&3      | COSMIC Q4 | THEORY |
| $\sigma_1$     | 53.6      | 56.6   |
| $\sigma_2$     | 1.4       | 1.8    |
| $\tau_{12}$    | -.04      | -.08   |

\*NORMALIZED VALUE



# LAMINATED COMPOSITE OPEN TUBE OF RADIUS 50.0

PROBLEM #10



SYMMETRIC  
8 LAYERS

[45, 45, 0, 90, 90, 0, -45, 45]

CONSTANT PRESSURE

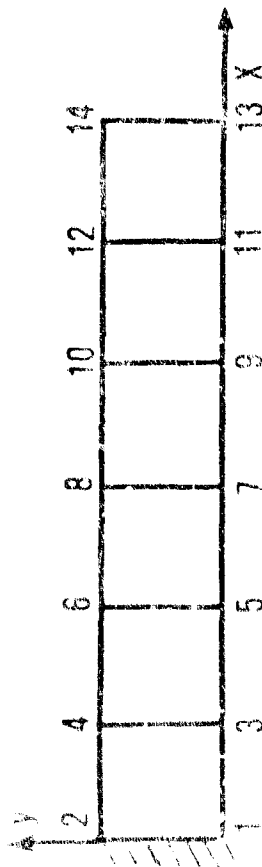
$T_i = .24 \quad i = 1, \dots, 8$   
 $E_1 = 73.8E3 \quad L = 80.0$   
 $E_2 = 3.75E3 \quad P = 10.5$   
 $\nu_{12} = 0.4$   
 $G_{12} = 1.74E3$

| COMPARISON OF $\sigma_1$ LAYER STRESS FOR ELEMENT ID 9 |                |             |
|--|----------------|-------------|
| LAYER  | COSMIC/HASTRAN | MSC/NASTRAN |
| 1  | 2.425E2        | 2.519E2     |
| 2  | 2.543E2        | 2.494E2     |
| 3  | -2.268E2       | -2.257E2    |
| 4  | 7.252E2        | 7.268E2     |
| 5  | 7.252E2        | 7.269E2     |
| 6  | -2.238E2       | -2.255E2    |
| 7  | 2.560E2        | 2.535E2     |
| 8  | 2.472E2        | 2.478E2     |



# STRAIGHT BEAM TEST

PROBLEM #11



$$\begin{aligned} \tau_i &= .25 \\ E_i &= 10.0E6 \\ \nu_i &= .3 \end{aligned}$$

$$i = 1, \dots, 4$$

$$l = 6.0$$

COMPARISON OF T1 DEFLECTION AT GRIDS 13 AND 14  
(EXTENSIONAL)

COSMIC/NASTRAN MSC/NASTRAN THEORETICAL

GRID 13 2.980E-5 2.986E-5 3.0E-5  
GRID 14 2.980E-5 2.986E-5 3.0E-5

(BENDING)

GRID 13 4.224E-1 4.253E-1 4.320E-1  
GRID 14 4.224E-1 4.253E-1 4.320E-1

CANTILEVERED BEAM MODEL UNDER A)  
EXTENSIONAL AND B) BENDING LOADINGS  
SIMULATION OF EQUIVALENT ISOTROPIC  
PROPERTIES. LAMINATE CONFIGURATION

[0/0/0/0]

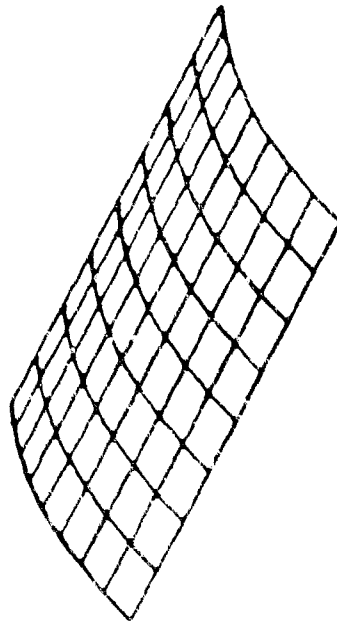
| COMPARISON OF DIRECT LAYER BENDING STRESS<br>ELEMENT 6 (LARGEST BENDING MOMENT) |                |             |
|---|----------------|-------------|
| LAYER   | COSMIC/NASTRAN | MSC/NASTRAN |
| 1   | 1.238E4        | 1.238E4     |
| 2   | 4.125E3        | 4.126E3     |
| 3   | -4.125E3       | -4.126E3    |
| 4   | -1.238E4       | -1.238E4    |



# LAMINATED COMPOSITE SHELL ROOF MODEL

## PROBLEM #12

$T_i = .03125$   $i = 1, \dots, 8$   $RADIUS = 25.0$   
 $E_1 = 2.0E8$   $L = 25.0$   
 $E_2 = 0.5E7$   $P = 90.0$   
 $NU_{12} = 0.25$   $G_{12} = 2.5E6$   
 $G_{12} = 2.5E6$   $G_{22} = 2.5E6$



SYMMETRIC ANGLE PLY

8 LAYERS

[45, -45, 15, -15, -15, 15, -45, 45]

UNIFORM PRESSURE

| COMPARISON OF T1 DEFLECTION AT SELECTED GRIDS |                |             |
|---|----------------|-------------|
| GRID  | COSMIC/NASTRAN | MSC/NASTRAN |
| 34  | -1.0735        | -1.0662     |
| 35  | -1.3545        | -1.3441     |
| 36  | -1.6213        | -1.6074     |
| 43  | -1.3363        | -1.3287     |
| 44  | -1.6876        | -1.6739     |
| 45  | -2.0267        | -2.0079     |

APPENDIX C: CASE CONTROL AND BULK DATA CARDS  
PERTAINING TO THE QUAD4 ELEMENT

Case Control Data Card ELSTRESS - Element Stress Output Request

Description: Requests form and type of element stress output.

Format and Example(s):

ELSTRESS  $\left[ \begin{pmatrix} \text{SORT1} & \text{PRINT} & \text{EXTREME} & \text{REAL} \\ \text{SORT2} & \text{PUNCH} & \text{LAYER} & \text{IMAG} \\ & \text{NOPRINT} & & \text{PHASE} \end{pmatrix} \right] = \left\{ \begin{matrix} \text{ALL} \\ n \\ \text{NONE} \end{matrix} \right\}$

ELSTRESS = 5

ELSTRESS = ALL

ELSTRESS(SORT1, PRINT, PUNCH, PHASE) = 15

| <u>Option</u>    | <u>Meaning</u>   |
|------------------|--|
| SORT1            | Output will be presented as a tabular listing of elements for each load, frequency, eigenvalue, or time, depending on the rigid format. SORT1 is not available in Transient problems (where the default is SORT2). |
| SORT2            | Output will be presented as a tabular listing of load, frequency, or time for each element type. SORT2 is available only in Static Analysis, Transient and Frequency Response problems.                            |
| PRINT            | The printer will be the output media.  |
| PUNCH            | The card punch will be the output media.   |
| EXTREME or LAYER | Requests stresses to be calculated at the extreme (top and bottom) fibers of a plate element or, for composites, the stresses for each layer. (See Remark 8)   |
| REAL or IMAG     | Requests real and imaginary output on Complex Eigenvalue or Frequency Response problems.   |
| PHASE            | Requests magnitude and phase ( $0.0^\circ \leq \text{phase} < 360.0^\circ$ ) on Complex Eigenvalue or Frequency Response problems.   |
| ALL              | Stresses for all elements will be output.  |
| n                | Set identification of a previously appearing SET card (Integer > 0). Only stresses for elements whose identification numbers appear on this SET card will be output.   |
| NONE             | Stresses for no elements will be output.   |

- Remarks:
- Both PRINT and PUNCH may be requested.
  - An output request for ALL in Transient and Frequency response problems generally produces large amounts of printout. An alternative to this would be to define a SET of interest.
  - In Static Analysis or Frequency Response problems, any request for SORT2 output causes all output to be SORT2.
  - ELSTRESS is an alternate form and is entirely equivalent to STRESS.
  - ELSTRESS = NONE allows overriding an overall request.

(Continued)

2.3-18 (8/10/87)

ELSTRESS (Continued)

6. If element stresses in material coordinate system are desired (only for TRIA1, TRIA2, QUAD1 and QUAD2 elements and only in Rigid Format 1), the parameter STRESS (see the description of the PARAM bulk data card in Section 2.4.2) should be set to be a positive integer. If, in addition to element stresses in material coordinate system, stresses at the connected grid points are also desired, the parameter STRESS should be set to 0.
7. When LAYER is selected, individual layer stresses and/or failure indices will be output.
8. The option EXTREME and LAYER is only applicable for the QUAD4 element.



# Case Control Data Card STRESS - Element Stress Output Request

Description: Requests form and type of element stress output.

Format and Example(s):

SYRESS  $\left[ \begin{pmatrix} \text{SORT1} \\ \text{SORT2} \end{pmatrix}, \begin{pmatrix} \text{PRINT} \\ \text{PUNCH} \\ \text{NOPRINT} \end{pmatrix}, \begin{pmatrix} \text{EXTREME} \\ \text{LAYER} \end{pmatrix}, \begin{pmatrix} \text{REAL} \\ \text{IMAG} \\ \text{PHASE} \end{pmatrix} \right] \cdot \left\{ \begin{matrix} \text{ALL} \\ n \\ \text{NONE} \end{matrix} \right\}$

STRESS = 5

STRESS = ALL

STRESS(SORT1, PRINT, PUNCH, PHASE) = 15

| <u>Option</u>    | <u>Meaning</u>   |
|------------------|--|
| SORT1            | Output will be presented as a tabular listing of elements for each load, frequency, eigenvalue, or time, depending on the rigid format. SORT1 is not available in Transient problems (where the default is SORT2). |
| SORT2            | Output will be presented as a tabular listing of load, frequency, or time for each element type. SORT2 is available only in Static Analysis, Transient and Frequency Response problems.                            |
| PRINT            | The printer will be the output media.  |
| PUNCH            | The card punch will be the output media.   |
| EXTREME or LAYER | Requests stresses to be calculated at the extreme (top and bottom) fibers of a plate element or, for composites, the stresses for each layer. (See Remark 8)   |
| REAL or IMAG     | Requests real and imaginary output on Complex Eigenvalue or Frequency Response problems.   |
| PHASE            | Requests magnitude and phase ( $0.0^\circ \leq \text{phase} < 360.0^\circ$ ) on Complex Eigenvalue or Frequency Response problems.   |
| ALL              | Stresses for all elements will be output.  |
| n                | Set identification of a previously appearing SET card (integer > 0). Only stresses for elements whose identification numbers appear on this SET card will be output.   |
| NONE             | Stresses for no elements will be output.   |

- Remarks:
- Both PRINT and PUNCH may be requested.
  - An output request for ALL in Transient and Frequency response problems generally produces large amounts of printout. An alternative to this would be to define a SET of interest.
  - In Static Analysis or Frequency Response problems, any request for SORT2 output causes all output to be SORT2.
  - STRESS is an alternate form and is entirely equivalent to ELSTRESS.
  - STRESS = NONE allows overriding an overall request.

(Continued)

2.3-47 (8/10/87)

STRESS (Continued)

6. If element stresses in material coordinate system are desired (only for TRIA1, TRIA2, QUAD1 and QUAD2 elements and only in Rigid Format 1), the parameter STRESS (see the description of the PARAM bulk data card in Section 2.4.2) should be set to be a positive integer. If, in addition to element stresses in material coordinate system, stresses at the connected grid points are also desired, the parameter STRESS should be set to 0.
7. When LAYER is selected, individual layer stresses and/or failure indices will be output.
8. The option EXTREME and LAYER is only applicable for the QUAD4 element.

Input Data Card CQUAD4

## Quadrilateral Element Connection

Description: Defines a quadrilateral plate element (QUAD4) of the structural model. This is an isoparametric membrane-bending element, with variable element thickness, layered composite material and thermal analysis capabilities.

Format and Example:

| 1      | 2   | 3   | 4    | 5     | 6    | 7    | 8    | 9    | 10  |
|--------|-----|-----|------|-------|------|------|------|------|-----|
| CQUAD4 | EID | PID | G1   | G2    | G3   | G4   | TM   | ZO   | abc |
| CQUAD4 | 101 | 17  | 1001 | 1005  | 1010 | 1024 | 45.0 | 0.01 | ABC |
| +bc    |     |     | T1   | T2    | T3   | T4   |      |      |     |
| +BC    |     |     | 0.03 | 0.125 | 0.05 | 0.04 |      |      |     |

FieldContents

EID Element identification number (Integer > 0)

PID Identification number of a PSHELL entry (Default is EID) (Integer > 0)  
For composites, see Remark 5.

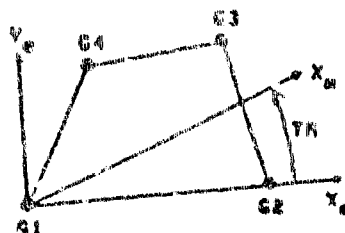
G1 Grid point identification numbers of connection points (Integer > 0)

ZO Offset of the element reference plane from the plane of grid points (Real or blank, see Remark 3 for default)

TM Material property orientation specification (Real or blank; or 0 ≤ Integer < 1,000,000). If Real or blank, specifies the material property orientation angle in degrees. If Integer, the orientation of the material x-axis is along the projection onto the plane of the element of the x-axis of the coordinate system specified by the integer value.

T1 Membrane thickness of element at grid points G1 (Real or blank, see Remark 4 for default).

Remarks: 1. The QUAD4 geometry, coordinate systems and numbering are shown in the figure below:



2. Element identification numbers must be unique with respect to all other element identification numbers.

(Continued)

2.4-E7a (8/10/87)

CQUAD4 (Continued)

3. The material coordinate system (TM) and the offset (ZO) may also be provided on the PSHELL entry. The PSHELL data will be used if the corresponding field on the CQUAD4 entry is blank.
4. The Ti are optional, if not supplied they will be set to the value of T specified on the PSHELL entry. In such cases, the continuation entry is not required.
5. For composites, a PCOMP, PCOMP1, PCOMP2 card can be used instead of a PSHELL card.

# BULK DATA DECK

Input Data Card MAT2 Material Property Definition

Description: Defines the material properties for linear, temperature-independent, anisotropic materials.

Format and Example:

| 1    | 2     | 3     | 4   | 5      | 6     | 7     | 8     | 9     | 10   |
|------|-------|-------|-----|--------|-------|-------|-------|-------|------|
| MAT2 | MID   | G11   | G12 | G13    | G22   | G23   | G33   | RHO   | +abc |
| MAT2 | 13    | 6.2+3 |     |        | 6.2+3 |       | 5.1+3 | 0.056 | ABC  |
| +abc | A1    | A2    | A12 | TO     | GE    | ST    | SC    | SS    | +def |
| +BC  | 0.15  |       |     | -500.0 | 0.002 | 20.+5 |       |       | DEF  |
| +def | MCSID |       |     |        |       |       |       |       |      |
| +EF  | 1008  |       |     |        |       |       |       |       |      |

| Field      | Contents  |
|------------|---|
| MID        | Material identification number (Integer > 0)  |
| Gij        | The material property matrix (Real)   |
| RHO        | Mass density (Real)   |
| Ai         | Thermal expansion coefficient vector (Real)   |
| TO         | Thermal expansion reference temperature (Real)  |
| GE         | Structural element damping coefficient (Real)   |
| ST, SC, SS | Stress limits for tension, compression and shear (Real) (Used only to compute margins of safety in certain elements; they have no effect on the computational procedures) |
| MCSID      | Material coordinate system identification number (Integer ≥ 0 or blank)   |

- Remarks:
1. The material identification numbers must be unique for all MAT1, MAT2 and MAT3 cards.
  2. MAT2 materials may be made temperature dependent by use of the MAT2 card.
  3. The mass density, RHO, will be used to automatically compute mass for all structural elements except the two-dimensional bending only elements TRBSC, TRPLT and QDPLT.
  4. The convention for the  $G_{ij}$  in fields 3 through 8 is represented by the matrix relationship.

$$\begin{pmatrix} \sigma_1 \\ \sigma_2 \\ \tau_{12} \end{pmatrix} = \begin{bmatrix} G_{11} & G_{12} & G_{13} \\ G_{12} & G_{22} & G_{23} \\ G_{13} & G_{23} & G_{33} \end{bmatrix} \begin{pmatrix} \epsilon_1 \\ \epsilon_2 \\ \gamma_{12} \end{pmatrix}$$

5. MCSID (> 0) is required if stresses or strains/curvatures are to be computed in a material coordinate system. This is applicable only for TRIA1, TRIA2, QUAD1, and QUAD2 elements.

Input Data Card MATB

## Orthotropic Plate Material Property Definition

Description: Defines the material property for an orthotropic material for plate elements.

Format and Example:

| 1    | 2     | 3     | 4     | 5    | 6     | 7   | 8   | 9     | 10  |
|------|-------|-------|-------|------|-------|-----|-----|-------|-----|
| MATB | MID   | E1    | E2    | NU12 | G12   | G1Z | G2Z | RHO   | abc |
| MATB | 299   | 32.+6 | 4.2+5 | 0.33 | 2.9+6 |     |     | 0.042 | ABC |
| +bc  | A1    | A2    | TREF  | XT   | XC    | YT  | YC  | S     | def |
| +BC  | 14.-6 | 2.3-6 | 175.  |      |       |     |     |       | DEF |
| +ef  | GE    | F12   |       |      |       |     |     |       |     |
| +EF  | 2.5-4 |       |       |      |       |     |     |       |     |

FieldContents

|       |   |
|-------|---|
| MID   | Material identification number (Integer > 0)  |
| E1,E2 | Modulus of elasticity in the material x and y directions (Real ≠ 0.0)   |
| NU12  | Poisson's Ratio (Real) (See Remark 5)   |
| G12   | Linear in-plane shear modulus (Real > 0.0)  |
| G1Z   | Transverse shear modulus for shear in X-Z plane (Real)  |
| G2Z   | Transverse shear modulus for shear in Y-Z plane (Real)  |
| RHO   | Mass density (Real)   |
| A1,A2 | Thermal expansion coefficients in the material x and y directions (T, Real > 0.0)   |
| TREF  | Thermal expansion reference temperature (XC, Real)  |
| XT,XC | Allowable stresses/strains in tension and compression, respectively, in the material x direction. Required if failure index calculation is desired. (XT, Real > 0.0) (XC, Real) (Default value for XC is XT) (See Remark 3) |
| YT,YC | Allowable stresses/strains in tension and compression, respectively, in the material y direction. Required if failure index calculation is desired. (YT, Real > 0.0) (YC, Real) (Default value for YC is YT) (See Remark 3) |
| S     | Allowable stress/strain for in-plane shear (Real > 0.0) (See Remark 3)  |
| GE    | Structural damping coefficient (Real)   |
| F12   | Tsai-Wu interaction term (Real) (See Remark 4)  |

(Continued)

2.4-173a (8/10/87)

# MATB (Continued)

- Remarks:
1. Material coordinate systems are defined by the plate element connection entries on the CQUAD4 card.
  2. The stress-strain relationship defined by this data is:

$$\begin{Bmatrix} \epsilon_1 \\ \epsilon_2 \\ \gamma_{12} \end{Bmatrix} = \begin{bmatrix} 1/E1 & -\nu_{12}/E1 & 0 \\ -\nu_{12}/E1 & 1/E2 & 0 \\ 0 & 0 & 1/G12 \end{bmatrix} \begin{Bmatrix} \sigma_1 \\ \sigma_2 \\ \tau_{12} \end{Bmatrix} + (T-TREF) \begin{Bmatrix} \alpha_1 \\ \alpha_2 \\ 0 \end{Bmatrix}$$

$$\begin{Bmatrix} \tau_{23} \\ \tau_{31} \end{Bmatrix} = \begin{bmatrix} G12 & 0 \\ 0 & G23 \end{bmatrix} \begin{Bmatrix} \gamma_{23} \\ \gamma_{31} \end{Bmatrix}$$

3. Fields XT, XC, YT, YC and S are used only for composite materials when failure calculations are requested with PCOMP, PCOMP1 or PCOMP2 Bulk Data entries. Allowables represent stresses except when the maximum strain failure theory is used.
4. The F12 field is used only for composite materials when the Tsai-Wu failure theory is used and failure calculations are requested.
5.  $\nu_{12}$  is Poisson's Ratio ( $\epsilon_1/\epsilon_2$  for uniaxial loading in 1-direction). Note that  $\nu_{21} = E_2/\epsilon_1$  uniaxial loading in 2-direction, is related to  $\nu_{12}$ , E1 and E2 by the relationship,  $(\nu_{12}) (E2) = (\nu_{21}) (E1)$ .

Input Data Card PCOMP Layered Composite Element Property

Description: Defines the properties of a n-ply laminated composite material.

Format and Example:

| 1     | 2    | 3    | 4    | 5     | 6         | 7  | 8    | 9      | 10  |
|-------|------|------|------|-------|-----------|----|------|--------|-----|
| PCOMP | PID  | ZO   | NSM  | SBOND | FT        |    |      | LOPT   | abc |
| PCOMP | 100  | -0.5 | 1.5  | 5.+3  | HOFF      |    |      | SYMMEM | ABC |
| +bc   | MID1 | T1   | TH1  | SOUT1 | MID2      | T2 | TH2  | SOUT2  | def |
| +BC   | 150  | 0.05 | 90.  | YES   |           |    | -45. |        | DEF |
| +ef   | MID3 | T3   | TH3  | SOUT3 | ETC. . .  |    |      |        |     |
| +EF   |      |      | 45.0 |       | . . . . . |    |      |        |     |

| <u>Field</u> | <u>Contents</u>  |
|--------------|--|
| PID          | Property identification number (1,000,000 > Integer > 0)   |
| ZO           | Offset of the element reference plane from the plane of grid points (Real or blank, see Remark 2)                                |
| NSM          | Non-structural mass per unit area. (Real)  |
| SBOND        | Allowable shear stress of the bonding material. (Real > 0.0 or blank)<br>Required if failure theory is used.                     |
| FT           | Failure theory, one of the strings "HILL", "HOFF", "TSAI", "STRESS", or "STRAIN". See Remark 4. (BCD or blank)                   |
| LOPT         | Lamination generation option, one of the strings, "ALL", "SYM", "MEM", or "SYMMEM". See Remark 5. (BCD or blank) Default is all. |
| MID1         | Material identification number of the 1 <sup>th</sup> layer. (Integer > 0 or blank)  |
| T1           | Thickness of the 1 <sup>th</sup> layer (Real, > 0.0 or blank)  |
| TH1          | Angle between the longitudinal direction of the fibers of the 1 <sup>th</sup> layer and the material X-axis. (Real or blank)     |
| SOUT1        | Stress output request for 1 <sup>th</sup> later, one of the strings "YES" or "NO". (Default is "NO")                             |

- Remarks:
1. The plies are numbered from 1 to n beginning with the bottom layer.
  2. The offset (ZO) is used only when the corresponding field on the CQIA04 Bulk Data entry referencing this property are blank.

(Continued)

2.4-222a (8/10/87)



PCOMP (Continued)

3. SBOND is required if bonding material failure index calculations are desired.
4. The failure theory is used to determine the element failure on a ply-by-ply basis. The available theories are:

|        |                             |
|--------|-----------------------------|
| HILL   | - Hill Theory               |
| HOFF   | - Hoffman Theory            |
| TSAI   | - Tsai-Wu Theory            |
| STRESS | - For Maximum Stress Theory |
| STRAIN | - For Maximum Strain Theory |

5. To minimize input requirements several lamination options (LOPT) are available. All indicates that every ply is specified, SYM indicates that ply layup is symmetric about the center ply and that the plies on one side of the center line are specified. SYMMEM indicates a symmetric layup of membrane only plies.
6. The material properties, MID1, may reference only MAT1, MAT2 and MAT8 Bulk Data entries.
7. If any of the MID1, T1 or TH1 are blank, then the last non-blank values specified for each will be used to define the values for the ply.

2-4-2276 (8/10/67)

Input Data Card PCOMP1      Layered Composite Element Property

Description: Defines the properties of a n-ply laminated composite material where all plies are composed of the same material and are of equal thickness.

Format and Example:

| 1      | 2     | 3    | 4    | 5     | 6      | 7         | 8    | 9    | 10  |
|--------|-------|------|------|-------|--------|-----------|------|------|-----|
| PCOMP1 | PID   | Z0   | NSM  | SBOND | FT     | MID       | TPLY | LOPT | abc |
| PCOMP1 | 100   | -0.5 | 1.7  | 5.+3  | STRAIN | 200       | 0.25 | SYM  | ABC |
| +bc    | TH1   | TH2  | TH3  | TH4   | TH5    | ETC. . .  |      |      |     |
| +BC    | -45.0 | 45.0 | 90.0 | 90.0  | 45.0   | . . . . . |      |      |     |

| <u>Field</u> | <u>Contents</u>  |
|--------------|--|
| PID          | Property identification number (1,000,000 > Integer > 0)   |
| Z0           | Offset of the element reference plane from the plane of grid points (Real or blank, see Remark 2)                            |
| NSM          | Non-structural mass per unit area. (Real)  |
| SBOND        | Allowable shear stress of the bonding material. (Real > 0.0)   |
| FT           | Failure theory, one of the strings "HILL", "HOFF", "TSAI", "STRESS", or "STRAIN". See Remark 4.                              |
| MID          | Material identification number for all layers. (Integer > 0)   |
| LOPT         | Lamination generation option, one of the strings, "ALL", "SYM", "MEM", or "SYMMEM". See Remark 5.                            |
| TPLY         | Thickness of all layers (Real, > 0.0 or blank)   |
| THi          | Angle between the longitudinal direction of the fibers of the i <sup>th</sup> layer and the material X-axis. (Real or blank) |

- Remarks:
1. The plies are numbered from 1 to n beginning with the bottom layer.
  2. The offset (Z0) is used only when the corresponding field on the CQUAD4 Bulk Data entry referencing this property is blank.
  3. SBOND is required if bonding material failure index calculations are desired.
  4. The failure theory is used to determine the element failure on a ply-by-ply basis. The available theories are:
 

|        |                             |
|--------|-----------------------------|
| HILL   | - Hill Theory               |
| HOFF   | - Hoffman Theory            |
| TSAI   | - Tsai-Wu Theory            |
| STRESS | - For Maximum Stress Theory |
| STRAIN | - For Maximum Strain Theory |

(Continued)

2.4-223c (8/10/87)

PCOMP1 (Continued)

5. To minimize input requirements several lamination options (LOPT) are available. All indicates that every ply is specified, SYM indicates that ply layup is symmetric about the center ply and that the plies on one side of the center line are specified. SYMMEM indicates a symmetric layup of membrane only plies.
6. The material property, MID, may reference only MAT1, MAT2 and MAT8 Bulk Data entries.

2.4.21- (8-10-87)

Input Data Card PCOMP2

## Layered Composite Element Property

Description: Defines the properties of a n-ply laminated composite material where all plies are composed of the same material.

Format and Example:

| 1      | 2    | 3     | 4   | 5     | 6    | 7    | 8         | 9    | 10  |
|--------|------|-------|-----|-------|------|------|-----------|------|-----|
| PCOMP2 | PID  | Z0    | NSM | SBOND | FT   | MID  |           | LOPT | abc |
| PCOMP2 | 100  | -0.5  | 1.7 | 5.+3  | TSAI | 200  |           | SYM  | ABC |
| +bc    | T1   | TH1   | T2  | TH2   | T3   | TH3  | ETC. . .  |      |     |
| +BC    | 0.25 | -45.0 | 0.5 | 90.0  | 0.25 | 45.0 | . . . . . |      |     |

| <u>Field</u> | <u>Contents</u>  |
|--------------|--|
| PID          | Property identification number (1,000,000 > Integer > 0)   |
| Z0           | Offset of the element reference plane from the plane of grid points (Real or blank, see Remark 2)                            |
| NSM          | Non-structural mass per unit area. (Real)  |
| SBOND        | Allowable shear stress of the bonding material. (Real > 0.0)   |
| FT           | Failure theory, one of the strings "HILL", "HOFF", "TSAI", "STRESS", or "STRAIN". See Remark 4.                              |
| MID          | Material identification number for all layers. (Integer > 0 or blank)  |
| LOPT         | Lamination generation option, one of the strings, "ALL", "SYM", "MEM", or "SYMMEM". See Remark 5.                            |
| Ti           | Thickness of the i <sup>th</sup> layer (Real, > 0.0 or blank)  |
| THi          | Angle between the longitudinal direction of the fibers of the i <sup>th</sup> layer and the material X-axis. (Real or blank) |

- Remarks:
1. The plies are numbered from 1 to n beginning with the bottom layer.
  2. The offset (Z0) is used only when the corresponding field on the EQUAD4 Bulk Data entry referencing this property is blank.
  3. SBOND is required if bonding material failure index calculations are desired.
  4. The failure theory is used to determine the element failure on a ply-by-ply basis. The available theories are:
 

|        |                             |
|--------|-----------------------------|
| HILL   | - Hill Theory               |
| HOFF   | - Hoffman Theory            |
| TSAI   | - Tsai-Wu Theory            |
| STRESS | - For Maximum Stress Theory |
| STRAIN | - For Maximum Strain Theory |

(Continued)

2.4-223e (8/10/87)

PCOMP2 (Continued)

5. To minimize input requirements several lamination options (LOPT) are available. All indicates that every ply is specified, SYM indicates that ply layup is symmetric about the center ply and that the plies on one side of the center line are specified. SYMMEM indicates a symmetric layup of membrane only plies.
6. The material property, MID, may reference only MAT1, MAT2 and MAT8 Bulk Data entries.
7. If any of the T1 or TH1 are blank, then the last non-blank values specified for each will be used to define the values for the ply.

# Input Data Card PLOAD4

## Pressure Loads on Face of Structural Elements

Description: Defines a load on a face of a QUAD4 element.

### Format and Example:

| 1      | 2   | 3    | 4   | 5   | 6    | 7  | 8 | 9 | 10  |
|--------|-----|------|-----|-----|------|----|---|---|-----|
| PLOAD4 | SID | EID  | P1  | P2  | P3   | P4 |   |   | abc |
| PLOAD4 | 101 | 2043 | 15. | 18. | 23.6 |    |   |   | ABC |

|     |     |     |    |    |  |  |  |  |  |
|-----|-----|-----|----|----|--|--|--|--|--|
| +bc | CID | N1  | N2 | N3 |  |  |  |  |  |
| +BC | 52  | 1.0 | 0. | 0. |  |  |  |  |  |

### Alternate Form:

|        |      |     |      |    |    |    |        |     |     |
|--------|------|-----|------|----|----|----|--------|-----|-----|
| PLOAD4 | SID  | E1  | P1   | P2 | P3 | P4 | "THRU" | E2  | gh1 |
| PLOAD4 | 1001 | 452 | 105. |    |    |    | THRU   | 568 | GHI |

|     |      |    |    |    |  |  |  |  |  |
|-----|------|----|----|----|--|--|--|--|--|
| +h1 | CID  | N1 | N2 | N3 |  |  |  |  |  |
| +HI | 2375 | 0. | 1. | 1. |  |  |  |  |  |

### Field

### Contents

|           |  |
|-----------|--|
| SID       | Load set identification number (Integer > 0)   |
| EID,E1,E2 | Element identification number (Integer > 0, E1 < E2)                                   |
| P1        | Pressure at the grid point defining the element face (Real or blank)                   |
| CID       | Coordinate system identification number (Integer > 0)                                  |
| N1        | Components of a vector in system CID that defines the direction of the pressure (Real) |

- Remarks:
1. For the plate element QUAD4, if the continuation entry is not given, the direction of the pressure is normal to the element in the element Z direction. If only P1 is given, the pressure is assumed to be uniform over the element surface.
  2. If the loaded surface of an element is curved, and a direction vector is not specified, the direction of the pressure may vary over the surface. The pressure intensity is the load per unit surface area.
  3. Equivalent grid point loads are computed. A uniform pressure need not result in equal grid point loads.

## Bulk Data Entry PSHELL

## Shell Element Property

Description: Defines the membrane, bending, transverse shear, and coupling properties of the QUAD4 shell element.

Format and Example:

| 1      | 2     | 3     | 4    | 5     | 6                  | 7    | 8    | 9    | 10  |
|--------|-------|-------|------|-------|--------------------|------|------|------|-----|
| PSHELL | PID   | MID1  | T    | MID2  | 12I/T <sup>3</sup> | MID3 | TS/T | NSM  | abc |
| PSHELL | 203   | 204   | 1.90 | 205   | 1.2                | 206  | 0.8  | 6.32 | ABC |
| +bc    | Z1    | Z2    | MID4 | MCSID | SCSID              | Z0   |      |      |     |
| +BC    | + .95 | - .95 |      | 0     | 0                  | 0.01 |      |      |     |

FieldContents

|                    |  |
|--------------------|--|
| PID                | Property identification number (Integer > 0)   |
| MID1               | Material identification number for membrane (Integer > 0 or blank)   |
| T                  | Default value for membrane thickness (Real > 0.0)  |
| MID2               | Material identification number for bending (Integer > 0 or blank)  |
| 12I/T <sup>3</sup> | Bending stiffness parameter (Real or blank, default = 1.0)   |
| MID3               | Material identification number for transverse shear (Integer > 0 or blank, must be blank unless MID2 > 0)  |
| TS/T               | Transverse shear thickness divided by membrane thickness (Real or blank, default = .833333)  |
| NSM                | Nonstructural mass per unit area (Real)  |
| Z1,Z2              | Fiber distances for stress computation. The positive direction is determined by the righthand rule and the order in which the grid points are listed on the connection entry. (Real or blank, defaults are -1/2 for Z1 and +1/2 for Z2). |
| MID4               | Material identification number for membrane-bending coupling (Integer > 0 or blank, must be blank unless MID1 > 0 and MID2 > 0, may not equal MID1 or MID2)  |
| MCSID              | Identification number of material coordinate system (Real or blank, or Integer ≥ 0) (See Remark 11)  |
| SCSID              | Identification number of stress coordinate system (Real or blank, or Integer ≥ 0) (See Remark 11)  |
| Z0                 | Offset of the element reference plane from the plane of grid points. (Real or blank, default = 0.0) (See Remark 12)  |

(Continued)

2.4 255 (8/10/87)

PSHELL (Continued)

- Remarks:
1. All PSHELL property entries must have unique identification numbers.
  2. The structural mass is computed from the density using the membrane thickness and membrane material properties.
  3. The results of leaving any MID field blank are:

|      |   |
|------|---|
| MID1 | No membrane or coupling stiffness                   |
| MID2 | No bending, coupling, or transverse shear stiffness |
| MID3 | No transverse shear flexibility                     |
| MID4 | No membrane-bending coupling                        |
  4. The continuation entry is not required.
  5. Structural damping, when needed, is obtained from the MID1 material.
  6. The MID4 field should be left blank if the material properties are symmetric with the middle surface of the shell.
  7. For structural problems, PSHELL entries may reference MAT1, MAT2 or MAT8 material property data.
  8. If the transverse shear material, MID3, references MAT2 data, then G33 must be zero. If MID3 references MAT8 data, then G1,Z and G2,Z must not be zero.
  9. For heat transfer problems PSHELL entries may reference MAT4 or MAT5 material property data.
  10. If MCSID/SCSID is left blank (0.0) or is real, it is considered to be the angle of rotation of the X axis of the material/stress coordinate system with respect to the X axis of the element coordinate system in the XY plane of the latter. If Integer, the orientation of the material/stress x-axis is along the projection of the x-axis of the specified coordinate system onto the x-y plane of the element system. The value of MCSID is the default value for the 1M field on the CQUAD4 Bulk Data entries.
  11. The value of ZO is the default value for the corresponding field on the CQUAD4 Bulk Data entries.



APPENDIX D: NASTRAN INPUT DATA FOR SAMPLE  
PROBLEMS 1-6, 8, 9, 11, 13c, 14

[illegible]

1964-1965 RELEASE UNDER E.O. 14176

**C O N T R O L C O U R S E**

**THE LUTHERAN**

1934

1141 12, 1963 22, 1963 1967 1968 1969

#11. INFORMATION MESSAGE 227, BUREAU NOT SORTED, REPORT WILL BE OPEN BOX, REFERENCE PLATE 06709 TEST

[illegible]



IP MASTROM, TISCHLER  
 APP DISP  
 SOL 1.0  
 TIME 5  
 CEND

PROBLEM #2

1 STRAIGHT CANTILEVER BEAM - REGULAR SHAPE ELEMENTS APRIL 11, 1983 RELEASE 1987 CDC PAGE 2

CASE CONTROL DECK ECHO

CARD  
COUNT

1 TITLE - STRAIGHT CANTILEVER BEAM - REGULAR SHAPE ELEMENTS

2 DISP - ALL

3 STRESS - ALL

4 ELFORCES - ALL

5 STRAIN - ALL

6 SPC - 1

7 SPCFORCES - ALL

8 OLOAD - ALL

9 BLOAD IN THE X DIRECTION

10 SURCASE 1

11 LOAD - 1

12 BLOAD IN THE Z DIRECTION

13 SURCASE 2

14 LOAD - 2

15 BLOAD IN THE Y DIRECTION

16 SURCASE 3

17 LOAD - 3

18 STUDEST LOAB

19 SURCASE 4

20 LOAD - 4

21 BEGIN BULK

6 0000 USER INFORMATION MESSAGE 007 BULK DATA NOT SORTED, XSORT WILL RE-ORDER DECK.  
 1 STRAIGHT CANTILEVER BEAM - REGULAR SHAPE ELEMENTS APRIL 11, 1986 RELEASE 1987 CDC PAGE 3

SORTED BULK DATA ECHO

CARD  
COUNT

1 COLUMD4

2 COLUMD4

3 COLUMD4

4 COLUMD4

5 COLUMD4

6 COLUMD4

7 FORCE

8 FORCE

9 FORCE

10 FORCE

11 FORCE

12 FORCE

13 FORCE

14 FORCE

15 FORCE

16 FORCE

17 FORCE

18 FORCE

19 FORCE

20 FORCE

21 FORCE

| CARD | COUNT | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 |
|------|-------|---|---|---|---|---|---|---|---|---|----|
| 1    | 1     | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1  |
| 2    | 1     | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1  |
| 3    | 1     | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1  |
| 4    | 1     | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1  |
| 5    | 1     | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1  |
| 6    | 1     | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1  |
| 7    | 1     | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1  |
| 8    | 1     | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1  |
| 9    | 1     | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1  |
| 10   | 1     | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1  |
| 11   | 1     | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1  |
| 12   | 1     | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1  |
| 13   | 1     | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1  |
| 14   | 1     | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1  |
| 15   | 1     | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1  |
| 16   | 1     | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1  |
| 17   | 1     | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1  |
| 18   | 1     | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1  |
| 19   | 1     | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1  |
| 20   | 1     | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1  |
| 21   | 1     | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1  |







18 WESTERN PISCHELE

APR 21 1988

TIME 5:04

DATE

PROBLEM #2

1 STRAIN CONTROL BEAR - TRIANGULAR SHAPE ELEMENTS

APRIL 21, 1988 RELEASE 1987 CDC

PAGE 2

CASE CONTROL SECK ECHO

CASE  
COUNT

1 TITLE - STRAIN CONTROL BEAR - TRIANGULAR SHAPE ELEMENTS

2 DIM - ALL

3 CLOAD - ALL

4 STRESS - ALL

5 STRAIN - ALL

6 SDC - 1

7 SURFACES - ALL

8 STCAD IN THE X DIRECTION

9 SUBCASE 1

10 LOAD - 1

11 LOAD IN THE Z DIRECTION

12 SUBCASE 2

13 LOAD - 2

14 LOAD IN THE Y DIRECTION

15 SUBCASE 3

16 LOAD - 3

17 STYLISH LOAD

18 SUBCASE 4

19 LOAD - 4

20 BEGIN RUN

2 BULK DATA NOT SORTED, XOUNT WILL RE-ORDER DECK.

3 STRAIN CONTROL BEAR - TRIANGULAR SHAPE ELEMENTS

APRIL 21, 1988

RELEASE 1987 CDC

PAGE 3

CASE  
COUNT

1 CLOAD

2 CLOAD

3 CLOAD

4 CLOAD

5 CLOAD

6 CLOAD

7 FORCE

8 FORCE

9 FORCE

10 FORCE

11 FORCE

12 FORCE

13 FORCE

14 FORCE

15 CLOAD

16 CLOAD

17 CLOAD

18 CLOAD

19 CLOAD

20 CLOAD

21 CLOAD

22 CLOAD

23 CLOAD

24 CLOAD

SORTED BULK DATA ECHO

1 2 3 4 5 6 7 8 9 10

1 1 1 1 1 1 1 1 1 1

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3 1 1 1 1 1 1 1 1 1

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5 1 1 1 1 1 1 1 1 1

6 1 1 1 1 1 1 1 1 1

7 1 1 1 1 1 1 1 1 1

8 1 1 1 1 1 1 1 1 1

9 1 1 1 1 1 1 1 1 1

10 1 1 1 1 1 1 1 1 1

11 1 1 1 1 1 1 1 1 1

12 1 1 1 1 1 1 1 1 1

13 1 1 1 1 1 1 1 1 1

14 1 1 1 1 1 1 1 1 1

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16 1 1 1 1 1 1 1 1 1

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18 1 1 1 1 1 1 1 1 1

19 1 1 1 1 1 1 1 1 1

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12. WESTMAN, FISCHLER

APP DISC

SEC 1.0

TIME 5.00

COMP

CONTROL DECK

APRIL 12, 1968 RELEASE 1987 CUC PAGE 2

CASE CONTROL DECK ECHO

CASE  
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TITLE - CURVED BEAM  
DISP - ALL  
SPFORCES - ALL  
STRESS - ALL  
LOAD - ALL  
SPEC - 1  
BLOAD IN THE Y DIRECTION  
SUBCASE 1  
BLOAD IN THE Z DIRECTION  
SUBCASE 2  
LOAD - 2  
BECOM BULK

4. BYE USER INFORMATION MESSAGE 207. BULK DATA NOT SORTED. SORT WILL BE ORDER RECD.

APRIL 12, 1968 RELEASE 1987 CUC PAGE 3

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ID MASTRAN, TISCHLER

APP DISP

SOL 1.0

TIME 500

CEND

TWISTED BEAM

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APRIL 12, 1988 RELEASE 1987 CDC PAGE 2

CASE CONTROL DECK ECHO

CARD  
COUNT  
1  
2  
3  
4  
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6  
7  
8  
9  
10  
11  
TITLE - TWISTED BEAM  
SPEC - 5  
SPCFORCES - ALL  
OLOAD - ALL  
DISP - ALL  
STRESS - ALL  
SUNCASE 1  
LOAD - 1  
SUBCASE 2  
LOAD - 2  
BEGIN BULK

0000 USER INFORMATION MESSAGE 207, BULK DATA NOT SORTED, XGONT WILL RE-ORDER DECK.  
1 TWISTED BEAM

APRIL 12, 1988 RELEASE 1987 CDC PAGE 3

| SORTED BULK DATA ECHO |    |    |    |    |    |    |    |    |    |    |
|-----------------------|----|----|----|----|----|----|----|----|----|----|
| CARD<br>COUNT         | 1  | 2  | 3  | 4  | 5  | 6  | 7  | 8  | 9  | 10 |
| 1- COUADA 1           | 20 | 20 | 20 | 30 | 38 | 35 | 35 | 35 | 35 | 35 |
| 2- COUADA 2           | 20 | 20 | 20 | 38 | 37 | 34 | 35 | 35 | 35 | 35 |
| 3- COUADA 3           | 20 | 20 | 20 | 35 | 35 | 32 | 32 | 32 | 32 | 32 |
| 4- COUADA 4           | 20 | 20 | 20 | 35 | 34 | 31 | 29 | 29 | 29 | 29 |
| 5- COUADA 5           | 20 | 20 | 20 | 32 | 32 | 28 | 27 | 27 | 27 | 27 |
| 6- COUADA 6           | 20 | 20 | 20 | 30 | 28 | 25 | 24 | 24 | 24 | 24 |
| 7- COUADA 7           | 20 | 20 | 20 | 27 | 26 | 23 | 23 | 23 | 23 | 23 |
| 8- COUADA 8           | 20 | 20 | 20 | 24 | 22 | 20 | 20 | 20 | 20 | 20 |
| 9- COUADA 9           | 20 | 20 | 20 | 24 | 22 | 19 | 18 | 18 | 18 | 18 |
| 10- COUADA 10         | 20 | 20 | 20 | 21 | 20 | 17 | 17 | 17 | 17 | 17 |
| 11- COUADA 11         | 20 | 20 | 20 | 20 | 19 | 16 | 15 | 15 | 15 | 15 |
| 12- COUADA 12         | 20 | 20 | 20 | 18 | 17 | 14 | 14 | 14 | 14 | 14 |
| 13- COUADA 13         | 20 | 20 | 20 | 17 | 16 | 13 | 12 | 12 | 12 | 12 |
| 14- COUADA 14         | 20 | 20 | 20 | 15 | 14 | 11 | 11 | 11 | 11 | 11 |
| 15- COUADA 15         | 20 | 20 | 20 | 14 | 13 | 10 | 9  | 9  | 9  | 9  |
| 16- COUADA 16         | 20 | 20 | 20 | 12 | 11 | 8  | 8  | 8  | 8  | 8  |
| 17- COUADA 17         | 20 | 20 | 20 | 11 | 10 | 7  | 6  | 6  | 6  | 6  |
| 18- COUADA 18         | 20 | 20 | 20 | 9  | 8  | 5  | 4  | 4  | 4  | 4  |
| 19- COUADA 19         | 20 | 20 | 20 | 8  | 7  | 4  | 3  | 3  | 3  | 3  |
| 20- COUADA 20         | 20 | 20 | 20 | 6  | 5  | 2  | 2  | 2  | 2  | 2  |
| 21- COUADA 21         | 20 | 20 | 20 | 5  | 4  | 1  | 1  | 1  | 1  | 1  |
| 22- COUADA 22         | 20 | 20 | 20 | 5  | 4  | 1  | 1  | 1  | 1  | 1  |
| 23- COUADA 23         | 20 | 20 | 20 | 5  | 4  | 1  | 1  | 1  | 1  | 1  |
| 24- COUADA 24         | 20 | 20 | 20 | 5  | 4  | 1  | 1  | 1  | 1  | 1  |
| 25- FORCE 1           | 1  | 1  | 1  | 5  | 4  | 1  | 1  | 1  | 1  | 1  |
| 26- FORCE 2           | 1  | 1  | 1  | 5  | 4  | 1  | 1  | 1  | 1  | 1  |
| 27- FORCE 3           | 1  | 1  | 1  | 5  | 4  | 1  | 1  | 1  | 1  | 1  |
| 28- GRID 1            | 1  | 1  | 1  | 5  | 4  | 1  | 1  | 1  | 1  | 1  |
| 29- GRID 2            | 1  | 1  | 1  | 5  | 4  | 1  | 1  | 1  | 1  | 1  |
| 30- GRID 3            | 1  | 1  | 1  | 5  | 4  | 1  | 1  | 1  | 1  | 1  |
| 31- GRID 4            | 1  | 1  | 1  | 5  | 4  | 1  | 1  | 1  | 1  | 1  |
| 32- GRID 5            | 1  | 1  | 1  | 5  | 4  | 1  | 1  | 1  | 1  | 1  |
| 33- GRID 6            | 1  | 1  | 1  | 5  | 4  | 1  | 1  | 1  | 1  | 1  |



APPLICATION OF SINGLE POINT CONSTRAINTS. REFER TO PRINTOUT OF AUTOMATICALLY GENERATED SPC1 CARDS FOR DETAILS.  
 TWISTED BEAM

APRIL 12, 1988 RELEASE 1987 CDC

1

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| AUTOMATICALLY GENERATED SPC1 CARDS |      |    |    |    |    |    |    |    |    |    |
|------------------------------------|------|----|----|----|----|----|----|----|----|----|
| CARD                               | 1    | 2  | 3  | 4  | 5  | 6  | 7  | 8  | 9  | 10 |
| COUNT                              | ..   | .. | .. | .. | .. | .. | .. | .. | .. | .. |
| 1-                                 | SPC1 | 5  | 4  | 23 | 26 | 29 | 32 | 35 |    |    |
| 2-                                 | SPC1 | 5  | 5  | 2  | 5  | 8  | 11 | 14 |    |    |
| 3-                                 | SPC1 | 5  | 5  | 20 |    |    |    |    | 17 |    |

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ID NASTRAN, TISCHLER  
APP DISP  
SOL 1, 0  
TIME 5.40  
CEND

1 RECTANGULAR PLATE - CLAMPED AR-1 N-6 APRIL 13, 1988 RELEASE 1987 CDC PAGE 2

CASE CONTROL DECK ECHO

CARD  
COUNT  
1 TITLE - RECTANGULAR PLATE - CLAMPED AR-1 N-6  
2 DISP - ALL  
3 STRESS - ALL  
4 LOAD - ALL  
5 SPCFORCES - ALL  
6 SPC - 5  
7 SUBCASE 1  
8 UNIFORM PRESSURE  
9 LOAD - 1  
10 SUBCASE 2  
11 8 CENTRAL LOAD  
12 LOAD - 2  
13 BEGIN BULK

0 USER INFORMATION MESSAGE 207, BULK DATA NOT SORTED, XSORT WILL RE-ORDER DECK.  
1 RECTANGULAR PLATE - CLAMPED AR-1 N-6 APRIL 13, 1988 RELEASE 1987 CDC PAGE 3

| SORTED BULK DATA ECHO |   |   |    |    |    |    |    |    |    |    |
|-----------------------|---|---|----|----|----|----|----|----|----|----|
| CARD<br>COUNT         | 1 | 2 | 3  | 4  | 5  | 6  | 7  | 8  | 9  | 10 |
| 1- COUADA 1           | 1 | 1 | 3  | 1  | 2  | 9  | 8  | 7  | 8  | 10 |
| 2- COUADA 2           | 1 | 1 | 1  | 3  | 3  | 10 | 9  | 11 | 11 | 10 |
| 3- COUADA 3           | 1 | 1 | 2  | 4  | 4  | 11 | 10 | 12 | 11 | 11 |
| 4- COUADA 4           | 1 | 1 | 4  | 5  | 5  | 12 | 11 | 13 | 12 | 11 |
| 5- COUADA 5           | 1 | 1 | 5  | 6  | 6  | 13 | 12 | 14 | 13 | 12 |
| 6- COUADA 6           | 1 | 1 | 6  | 7  | 7  | 14 | 13 | 15 | 14 | 13 |
| 7- COUADA 7           | 1 | 1 | 8  | 8  | 8  | 15 | 14 | 16 | 15 | 14 |
| 8- COUADA 8           | 1 | 1 | 9  | 9  | 9  | 16 | 15 | 17 | 16 | 15 |
| 9- COUADA 9           | 1 | 1 | 10 | 10 | 10 | 17 | 16 | 18 | 17 | 16 |
| 10- COUADA 10         | 1 | 1 | 11 | 11 | 11 | 18 | 17 | 19 | 18 | 17 |
| 11- COUADA 11         | 1 | 1 | 12 | 12 | 12 | 19 | 18 | 20 | 19 | 18 |
| 12- COUADA 12         | 1 | 1 | 13 | 13 | 13 | 20 | 19 | 21 | 20 | 19 |
| 13- COUADA 13         | 1 | 1 | 14 | 14 | 14 | 21 | 20 | 22 | 21 | 20 |
| 14- COUADA 14         | 1 | 1 | 15 | 15 | 15 | 22 | 21 | 23 | 22 | 21 |
| 15- COUADA 15         | 1 | 1 | 16 | 16 | 16 | 23 | 22 | 24 | 23 | 22 |
| 16- COUADA 16         | 1 | 1 | 17 | 17 | 17 | 24 | 23 | 25 | 24 | 23 |
| 17- COUADA 17         | 1 | 1 | 18 | 18 | 18 | 25 | 24 | 26 | 25 | 24 |
| 18- COUADA 18         | 1 | 1 | 19 | 19 | 19 | 26 | 25 | 27 | 26 | 25 |
| 19- COUADA 19         | 1 | 1 | 20 | 20 | 20 | 27 | 26 | 28 | 27 | 26 |
| 20- COUADA 20         | 1 | 1 | 21 | 21 | 21 | 28 | 27 | 29 | 28 | 27 |
| 21- COUADA 21         | 1 | 1 | 22 | 22 | 22 | 29 | 28 | 30 | 29 | 28 |
| 22- COUADA 22         | 1 | 1 | 23 | 23 | 23 | 30 | 29 | 31 | 30 | 29 |
| 23- COUADA 23         | 1 | 1 | 24 | 24 | 24 | 31 | 30 | 32 | 31 | 30 |
| 24- COUADA 24         | 1 | 1 | 25 | 25 | 25 | 32 | 31 | 33 | 32 | 31 |
| 25- COUADA 25         | 1 | 1 | 26 | 26 | 26 | 33 | 32 | 34 | 33 | 32 |
| 26- COUADA 26         | 1 | 1 | 27 | 27 | 27 | 34 | 33 | 35 | 34 | 33 |
| 27- COUADA 27         | 1 | 1 | 28 | 28 | 28 | 35 | 34 | 36 | 35 | 34 |
| 28- COUADA 28         | 1 | 1 | 29 | 29 | 29 | 36 | 35 | 37 | 36 | 35 |
| 29- COUADA 29         | 1 | 1 | 30 | 30 | 30 | 37 | 36 | 38 | 37 | 36 |
| 30- COUADA 30         | 1 | 1 | 31 | 31 | 31 | 38 | 37 | 39 | 38 | 37 |
| 31- COUADA 31         | 1 | 1 | 32 | 32 | 32 | 39 | 38 | 40 | 39 | 38 |
|                       |   |   | 33 | 33 | 33 | 40 | 39 | 41 | 40 | 39 |
|                       |   |   | 34 | 34 | 34 | 41 | 40 | 42 | 41 | 40 |
|                       |   |   | 35 | 35 | 35 | 42 | 41 | 43 | 42 | 41 |
|                       |   |   | 36 | 36 | 36 | 43 | 42 |    |    |    |

|      | 1      | 37  | 45 | 44  |  |  |
|------|--------|-----|----|-----|--|--|
| 32-  | COUMD4 | 32  | 45 | 44  |  |  |
| 33-  | COUMD4 | 33  | 46 | 45  |  |  |
| 34-  | COUMD4 | 34  | 47 | 46  |  |  |
| 35-  | COUMD4 | 35  |    | 46  |  |  |
| 36-  | COUMD4 | 36  |    | 47  |  |  |
| 37-  | COUMD4 | 37  |    | 48  |  |  |
| 38-  | COUMD4 | 38  |    | 49  |  |  |
| 39-  | COUMD4 | 39  |    | 50  |  |  |
| 40-  | COUMD4 | 40  |    | 51  |  |  |
| 41-  | COUMD4 | 41  |    | 52  |  |  |
| 42-  | COUMD4 | 42  |    | 53  |  |  |
| 43-  | COUMD4 | 43  |    | 54  |  |  |
| 44-  | COUMD4 | 44  |    | 55  |  |  |
| 45-  | COUMD4 | 45  |    | 56  |  |  |
| 46-  | COUMD4 | 46  |    | 57  |  |  |
| 47-  | COUMD4 | 47  |    | 58  |  |  |
| 48-  | COUMD4 | 48  |    | 59  |  |  |
| 49-  | COUMD4 | 49  |    | 60  |  |  |
| 50-  | COUMD4 | 50  |    | 61  |  |  |
| 51-  | COUMD4 | 51  |    | 62  |  |  |
| 52-  | COUMD4 | 52  |    | 63  |  |  |
| 53-  | COUMD4 | 53  |    | 64  |  |  |
| 54-  | COUMD4 | 54  |    | 65  |  |  |
| 55-  | COUMD4 | 55  |    | 66  |  |  |
| 56-  | COUMD4 | 56  |    | 67  |  |  |
| 57-  | COUMD4 | 57  |    | 68  |  |  |
| 58-  | COUMD4 | 58  |    | 69  |  |  |
| 59-  | COUMD4 | 59  |    | 70  |  |  |
| 60-  | COUMD4 | 60  |    | 71  |  |  |
| 61-  | COUMD4 | 61  |    | 72  |  |  |
| 62-  | COUMD4 | 62  |    | 73  |  |  |
| 63-  | COUMD4 | 63  |    | 74  |  |  |
| 64-  | COUMD4 | 64  |    | 75  |  |  |
| 65-  | COUMD4 | 65  |    | 76  |  |  |
| 66-  | COUMD4 | 66  |    | 77  |  |  |
| 67-  | COUMD4 | 67  |    | 78  |  |  |
| 68-  | COUMD4 | 68  |    | 79  |  |  |
| 69-  | COUMD4 | 69  |    | 80  |  |  |
| 70-  | COUMD4 | 70  |    | 81  |  |  |
| 71-  | COUMD4 | 71  |    | 82  |  |  |
| 72-  | COUMD4 | 72  |    | 83  |  |  |
| 73-  | COUMD4 | 73  |    | 84  |  |  |
| 74-  | COUMD4 | 74  |    | 85  |  |  |
| 75-  | COUMD4 | 75  |    | 86  |  |  |
| 76-  | COUMD4 | 76  |    | 87  |  |  |
| 77-  | COUMD4 | 77  |    | 88  |  |  |
| 78-  | COUMD4 | 78  |    | 89  |  |  |
| 79-  | COUMD4 | 79  |    | 90  |  |  |
| 80-  | COUMD4 | 80  |    | 91  |  |  |
| 81-  | COUMD4 | 81  |    | 92  |  |  |
| 82-  | COUMD4 | 82  |    | 93  |  |  |
| 83-  | COUMD4 | 83  |    | 94  |  |  |
| 84-  | COUMD4 | 84  |    | 95  |  |  |
| 85-  | COUMD4 | 85  |    | 96  |  |  |
| 86-  | COUMD4 | 86  |    | 97  |  |  |
| 87-  | COUMD4 | 87  |    | 98  |  |  |
| 88-  | COUMD4 | 88  |    | 99  |  |  |
| 89-  | COUMD4 | 89  |    | 100 |  |  |
| 90-  | COUMD4 | 90  |    | 101 |  |  |
| 91-  | COUMD4 | 91  |    | 102 |  |  |
| 92-  | COUMD4 | 92  |    | 103 |  |  |
| 93-  | COUMD4 | 93  |    | 104 |  |  |
| 94-  | COUMD4 | 94  |    | 105 |  |  |
| 95-  | COUMD4 | 95  |    | 106 |  |  |
| 96-  | COUMD4 | 96  |    | 107 |  |  |
| 97-  | COUMD4 | 97  |    | 108 |  |  |
| 98-  | COUMD4 | 98  |    | 109 |  |  |
| 99-  | COUMD4 | 99  |    | 110 |  |  |
| 100- | COUMD4 | 100 |    | 111 |  |  |
| 101- | COUMD4 | 101 |    | 112 |  |  |
| 102- | COUMD4 | 102 |    | 113 |  |  |
| 103- | COUMD4 |     |    |     |  |  |

| CARD | COUNT | 1  | 2  | 3     | 4          | 5     | 6   | 7 | 8 | 9 | 10 |
|------|-------|----|----|-------|------------|-------|-----|---|---|---|----|
| 35-  | 35    | 1  | 1  | 40    | 41         | 48    | 47  |   |   |   |    |
| 36-  | 36    | 1  | 1  | 41    | 42         | 49    | 48  |   |   |   |    |
| 37-  | 37    | 2  | 40 |       | -1.0E-04.0 |       | 1.0 |   |   |   |    |
| 38-  | 38    | 1  |    | .0000 | .0000      | .0000 |     |   |   |   |    |
| 39-  | 39    | 2  |    | .0000 | .1657      | .0000 |     |   |   |   |    |
| 40-  | 40    | 3  |    | .0000 | .3333      | .0000 |     |   |   |   |    |
| 41-  | 41    | 4  |    | .0000 | .5000      | .0000 |     |   |   |   |    |
| 42-  | 42    | 5  |    | .0000 | .6667      | .0000 |     |   |   |   |    |
| 43-  | 43    | 6  |    | .0000 | .8333      | .0000 |     |   |   |   |    |
| 44-  | 44    | 7  |    | .0000 | 1.0000     | .0000 |     |   |   |   |    |
| 45-  | 45    | 8  |    | .1667 | .0000      | .0000 |     |   |   |   |    |
| 46-  | 46    | 9  |    | .1667 | .1667      | .0000 |     |   |   |   |    |
| 47-  | 47    | 10 |    | .1667 | .3333      | .0000 |     |   |   |   |    |
| 48-  | 48    | 11 |    | .1667 | .5000      | .0000 |     |   |   |   |    |
| 49-  | 49    | 12 |    | .1667 | .6667      | .0000 |     |   |   |   |    |
| 50-  | 50    | 13 |    | .1667 | .8333      | .0000 |     |   |   |   |    |
| 51-  | 51    | 14 |    | .1667 | 1.0000     | .0000 |     |   |   |   |    |
| 52-  | 52    | 15 |    | .3333 | .0000      | .0000 |     |   |   |   |    |
| 53-  | 53    | 16 |    | .3333 | .1667      | .0000 |     |   |   |   |    |
| 54-  | 54    | 17 |    | .3333 | .3333      | .0000 |     |   |   |   |    |
| 55-  | 55    | 18 |    | .3333 | .5000      | .0000 |     |   |   |   |    |
| 56-  | 56    | 19 |    | .3333 | .6667      | .0000 |     |   |   |   |    |
| 57-  | 57    | 20 |    | .3333 | .8333      | .0000 |     |   |   |   |    |
| 58-  | 58    | 21 |    | .3333 | 1.0000     | .0000 |     |   |   |   |    |
| 59-  | 59    | 22 |    | .5000 | .0000      | .0000 |     |   |   |   |    |
| 60-  | 60    | 23 |    | .5000 | .1667      | .0000 |     |   |   |   |    |
| 61-  | 61    | 24 |    | .5000 | .3333      | .0000 |     |   |   |   |    |
| 62-  | 62    | 25 |    | .5000 | .5000      | .0000 |     |   |   |   |    |
| 63-  | 63    | 26 |    | .5000 | .6667      | .0000 |     |   |   |   |    |
| 64-  | 64    | 27 |    | .5000 | .8333      | .0000 |     |   |   |   |    |
| 65-  | 65    | 28 |    | .5000 | 1.0000     | .0000 |     |   |   |   |    |
| 66-  | 66    | 29 |    | .6667 | .0000      | .0000 |     |   |   |   |    |
| 67-  | 67    | 30 |    | .6667 | .1667      | .0000 |     |   |   |   |    |
| 68-  | 68    | 31 |    | .6667 | .3333      | .0000 |     |   |   |   |    |

1 RECTANGULAR PLATE - CLAMPED AIR-1 N-6

| COUNT | 1    | 2  | 3 | 4      | 5      | 6 | 7 | 8 | 9 | 10 |
|-------|------|----|---|--------|--------|---|---|---|---|----|
| 69-   | CR10 | 38 |   | .0057  | .0000  |   |   |   |   |    |
| 70-   | CR10 | 39 |   | .0057  | .0057  |   |   |   |   |    |
| 71-   | CR10 | 34 |   | .0057  | .0000  |   |   |   |   |    |
| 72-   | CR10 | 35 |   | .0057  | 1.0000 |   |   |   |   |    |
| 73-   | CR10 | 36 |   | .0000  | .0000  |   |   |   |   |    |
| 74-   | CR10 | 37 |   | .0000  | .0000  |   |   |   |   |    |
| 75-   | CR10 | 38 |   | .0000  | .0000  |   |   |   |   |    |
| 76-   | CR10 | 39 |   | .0000  | .0000  |   |   |   |   |    |
| 77-   | CR10 | 40 |   | .0000  | .0000  |   |   |   |   |    |
| 78-   | CR10 | 41 |   | .0000  | .0000  |   |   |   |   |    |
| 79-   | CR10 | 42 |   | .0000  | .0000  |   |   |   |   |    |
| 80-   | CR10 | 43 |   | 1.0000 | .0000  |   |   |   |   |    |
| 81-   | CR10 | 44 |   | .0000  | .0000  |   |   |   |   |    |
| 82-   | CR10 | 45 |   | 1.0000 | .0000  |   |   |   |   |    |

— 403 —

[illegible]



18 INSTRUM, TIE-CLER  
ADD DISP  
SOL 1.0  
TIME 5.00  
CEND

PAGE 2

RELEASE 1987 CDC

APRIL 13, 1988

CASE CONTROL DECK ECHO

CASE COUNT  
1  
2  
3  
4  
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13

TITLE - RECTANGULAR PLATE - CLAMPED AR-S N=6  
DISP - ALL  
STRESS - ALL  
LOAD - ALL  
SUPPORTS - ALL  
SAC - 5  
SUBCASE 1  
8 UNIFORM PRESSURE  
LOAD - 1  
SUBCASE 2  
8 CENTRAL LOAD  
LOAD - 2  
BEGIN BULK

5 #12 USER INFORMATION MESSAGE 287, BULK DATA NOT SORTED. XSORT WILL RE-ORDER DECK.  
1 RECTANGULAR PLATE - CLAMPED AR-S N=6  
3

APRIL 13, 1988

RELEASE 1987 CDC

PAGE

| SORTED BULK DATA ECHO |       |    |    |    |    |    |    |    |    |    |    |    |
|-----------------------|-------|----|----|----|----|----|----|----|----|----|----|----|
| CASE                  | COUNT | 1  | 2  | 3  | 4  | 5  | 6  | 7  | 8  | 9  | 10 | 11 |
| 1                     | 1     | 1  | 1  | 1  | 1  | 1  | 1  | 1  | 1  | 1  | 1  | 1  |
| 2                     | 2     | 2  | 2  | 2  | 2  | 2  | 2  | 2  | 2  | 2  | 2  | 2  |
| 3                     | 3     | 3  | 3  | 3  | 3  | 3  | 3  | 3  | 3  | 3  | 3  | 3  |
| 4                     | 4     | 4  | 4  | 4  | 4  | 4  | 4  | 4  | 4  | 4  | 4  | 4  |
| 5                     | 5     | 5  | 5  | 5  | 5  | 5  | 5  | 5  | 5  | 5  | 5  | 5  |
| 6                     | 6     | 6  | 6  | 6  | 6  | 6  | 6  | 6  | 6  | 6  | 6  | 6  |
| 7                     | 7     | 7  | 7  | 7  | 7  | 7  | 7  | 7  | 7  | 7  | 7  | 7  |
| 8                     | 8     | 8  | 8  | 8  | 8  | 8  | 8  | 8  | 8  | 8  | 8  | 8  |
| 9                     | 9     | 9  | 9  | 9  | 9  | 9  | 9  | 9  | 9  | 9  | 9  | 9  |
| 10                    | 10    | 10 | 10 | 10 | 10 | 10 | 10 | 10 | 10 | 10 | 10 | 10 |
| 11                    | 11    | 11 | 11 | 11 | 11 | 11 | 11 | 11 | 11 | 11 | 11 | 11 |
| 12                    | 12    | 12 | 12 | 12 | 12 | 12 | 12 | 12 | 12 | 12 | 12 | 12 |
| 13                    | 13    | 13 | 13 | 13 | 13 | 13 | 13 | 13 | 13 | 13 | 13 | 13 |
| 14                    | 14    | 14 | 14 | 14 | 14 | 14 | 14 | 14 | 14 | 14 | 14 | 14 |
| 15                    | 15    | 15 | 15 | 15 | 15 | 15 | 15 | 15 | 15 | 15 | 15 | 15 |
| 16                    | 16    | 16 | 16 | 16 | 16 | 16 | 16 | 16 | 16 | 16 | 16 | 16 |
| 17                    | 17    | 17 | 17 | 17 | 17 | 17 | 17 | 17 | 17 | 17 | 17 | 17 |
| 18                    | 18    | 18 | 18 | 18 | 18 | 18 | 18 | 18 | 18 | 18 | 18 | 18 |
| 19                    | 19    | 19 | 19 | 19 | 19 | 19 | 19 | 19 | 19 | 19 | 19 | 19 |
| 20                    | 20    | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 | 20 |
| 21                    | 21    | 21 | 21 | 21 | 21 | 21 | 21 | 21 | 21 | 21 | 21 | 21 |
| 22                    | 22    | 22 | 22 | 22 | 22 | 22 | 22 | 22 | 22 | 22 | 22 | 22 |
| 23                    | 23    | 23 | 23 | 23 | 23 | 23 | 23 | 23 | 23 | 23 | 23 | 23 |
| 24                    | 24    | 24 | 24 | 24 | 24 | 24 | 24 | 24 | 24 | 24 | 24 | 24 |
| 25                    | 25    | 25 | 25 | 25 | 25 | 25 | 25 | 25 | 25 | 25 | 25 | 25 |
| 26                    | 26    | 26 | 26 | 26 | 26 | 26 | 26 | 26 | 26 | 26 | 26 | 26 |
| 27                    | 27    | 27 | 27 | 27 | 27 | 27 | 27 | 27 | 27 | 27 | 27 | 27 |
| 28                    | 28    | 28 | 28 | 28 | 28 | 28 | 28 | 28 | 28 | 28 | 28 | 28 |
| 29                    | 29    | 29 | 29 | 29 | 29 | 29 | 29 | 29 | 29 | 29 | 29 | 29 |
| 30                    | 30    | 30 | 30 | 30 | 30 | 30 | 30 | 30 | 30 | 30 | 30 | 30 |
| 31                    | 31    | 31 | 31 | 31 | 31 | 31 | 31 | 31 | 31 | 31 | 31 | 31 |
| 32                    | 32    | 32 | 32 | 32 | 32 | 32 | 32 | 32 | 32 | 32 | 32 | 32 |
| 33                    | 33    | 33 | 33 | 33 | 33 | 33 | 33 | 33 | 33 | 33 | 33 | 33 |
| 34                    | 34    | 34 | 34 | 34 | 34 | 34 | 34 | 34 | 34 | 34 | 34 | 34 |
| 35                    | 35    | 35 | 35 | 35 | 35 | 35 | 35 | 35 | 35 | 35 | 35 | 35 |
| 36                    | 36    | 36 | 36 | 36 | 36 | 36 | 36 | 36 | 36 | 36 | 36 | 36 |
| 37                    | 37    | 37 | 37 | 37 | 37 | 37 | 37 | 37 | 37 | 37 | 37 | 37 |
| 38                    | 38    | 38 | 38 | 38 | 38 | 38 | 38 | 38 | 38 | 38 | 38 | 38 |
| 39                    | 39    | 39 | 39 | 39 | 39 | 39 | 39 | 39 | 39 | 39 | 39 | 39 |
| 40                    | 40    | 40 | 40 | 40 | 40 | 40 | 40 | 40 | 40 | 40 | 40 | 40 |
| 41                    | 41    | 41 | 41 | 41 | 41 | 41 | 41 | 41 | 41 | 41 | 41 | 41 |
| 42                    | 42    | 42 | 42 | 42 | 42 | 42 | 42 | 42 | 42 | 42 | 42 | 42 |
| 43                    | 43    | 43 | 43 | 43 | 43 | 43 | 43 | 43 | 43 | 43 | 43 | 43 |
| 44                    | 44    | 44 | 44 | 44 | 44 | 44 | 44 | 44 | 44 | 44 | 44 | 44 |







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# PROBLEM #5

|      |        |    |          |        |       |    |     |    |
|------|--------|----|----------|--------|-------|----|-----|----|
| 83-  | GRID   | 46 | 1.0000   | .5000  | .0000 |    |     |    |
| 84-  | GRID   | 47 | 1.0000   | .5557  | .0000 |    |     |    |
| 85-  | GRID   | 48 | 1.0000   | .8333  | .0000 |    |     |    |
| 86-  | GRID   | 49 | 1.0000   | 1.0000 | .0000 |    |     |    |
| 87-  | PLATE  | 1  | 1.7472E7 | .3     |       |    |     |    |
| 88-  | PSHELL | 1  | 1.0E-04  | 1      | 1.0   | 1  | 1.0 | .0 |
| 89-  | SPC1   | 5  | 6        | 1      | 1.0   |    |     |    |
| 90-  | SPC1   | 5  | 6        | 1      | 1.0   |    |     |    |
| 91-  | SPC1   | 5  | 6        | 1      | 1.0   |    |     |    |
| 92-  | SPC1   | 5  | 6        | 1      | 1.0   |    |     |    |
| 93-  | SPC1   | 5  | 6        | 1      | 1.0   |    |     |    |
| 94-  | SPC1   | 5  | 6        | 1      | 1.0   |    |     |    |
| 95-  | SPC1   | 5  | 6        | 1      | 1.0   |    |     |    |
| 96-  | SPC1   | 5  | 6        | 1      | 1.0   |    |     |    |
| 97-  | SPC1   | 5  | 6        | 1      | 1.0   |    |     |    |
| 98-  | SPC1   | 5  | 6        | 1      | 1.0   |    |     |    |
| 99-  | SPC1   | 5  | 6        | 1      | 1.0   |    |     |    |
| 100- | ENDATA | 5  | 1236     | 1236   | 43    | 47 | 48  | 49 |



|   |          |   |    |    |    |
|---|----------|---|----|----|----|
| 32-   | COUAD 32 | 1 | 37 | 45 | 44 |
| 33-   | COUAD 33 | 1 | 38 | 46 | 45 |
| 34-   | COUAD 34 | 1 | 39 | 47 | 46 |
| RECTANGULAR PLATE - SIMPLY SUPPORTED AR-5 N-6 |          |   |    |    |    |
| APRIL 13, 1968                                |          |   |    |    |    |
| RELEASE 1987 CDC                              |          |   |    |    |    |
| PAGE 4  |          |   |    |    |    |

| CARD | COUNT  | 1  | 2  | 3      | 4          | 5     | 6     | 7   | 8 | 9 | 10 |
|------|--------|----|----|--------|------------|-------|-------|-----|---|---|----|
| 35-  | COUNA4 | 35 | 1  | 40     | 41         | 42    | 48    | 47  |   |   |    |
| 36-  | COUNA4 | 36 | 1  | 41     | 42         | 48    | 49    | 48  |   |   |    |
| 37-  | FORCE  | 2  | 49 |        | -1.0E-04.0 |       |       | 1.0 |   |   |    |
| 38-  | GRID   | 1  |    | .0000  | .0000      | .0000 | .0000 |     |   |   |    |
| 39-  | GRID   | 2  |    | .0000  | .1667      | .0000 | .0000 |     |   |   |    |
| 40-  | GRID   | 3  |    | .0000  | .3333      | .0000 | .0000 |     |   |   |    |
| 41-  | GRID   | 4  |    | .0000  | .5000      | .0000 | .0000 |     |   |   |    |
| 42-  | GRID   | 5  |    | .0000  | .6667      | .0000 | .0000 |     |   |   |    |
| 43-  | GRID   | 6  |    | .0000  | .8333      | .0000 | .0000 |     |   |   |    |
| 44-  | GRID   | 7  |    | .0000  | 1.0000     | .0000 | .0000 |     |   |   |    |
| 45-  | GRID   | 8  |    | .8333  | .0000      | .0000 | .0000 |     |   |   |    |
| 46-  | GRID   | 9  |    | .8333  | .1667      | .0000 | .0000 |     |   |   |    |
| 47-  | GRID   | 10 |    | .8333  | .3333      | .0000 | .0000 |     |   |   |    |
| 48-  | GRID   | 11 |    | .8333  | .5000      | .0000 | .0000 |     |   |   |    |
| 49-  | GRID   | 12 |    | .8333  | .6667      | .0000 | .0000 |     |   |   |    |
| 50-  | GRID   | 13 |    | .8333  | .8333      | .0000 | .0000 |     |   |   |    |
| 51-  | GRID   | 14 |    | .8333  | 1.0000     | .0000 | .0000 |     |   |   |    |
| 52-  | GRID   | 15 |    | 1.6667 | .0000      | .0000 | .0000 |     |   |   |    |
| 53-  | GRID   | 16 |    | 1.6667 | .1667      | .0000 | .0000 |     |   |   |    |
| 54-  | GRID   | 17 |    | 1.6667 | .3333      | .0000 | .0000 |     |   |   |    |
| 55-  | GRID   | 18 |    | 1.6667 | .5000      | .0000 | .0000 |     |   |   |    |
| 56-  | GRID   | 19 |    | 1.6667 | .6667      | .0000 | .0000 |     |   |   |    |
| 57-  | GRID   | 20 |    | 1.6667 | .8333      | .0000 | .0000 |     |   |   |    |
| 58-  | GRID   | 21 |    | 1.6667 | 1.0000     | .0000 | .0000 |     |   |   |    |
| 59-  | GRID   | 22 |    | 2.5000 | .0000      | .0000 | .0000 |     |   |   |    |
| 60-  | GRID   | 23 |    | 2.5000 | .1667      | .0000 | .0000 |     |   |   |    |
| 61-  | GRID   | 24 |    | 2.5000 | .3333      | .0000 | .0000 |     |   |   |    |
| 62-  | GRID   | 25 |    | 2.5000 | .5000      | .0000 | .0000 |     |   |   |    |
| 63-  | GRID   | 26 |    | 2.5000 | .6667      | .0000 | .0000 |     |   |   |    |
| 64-  | GRID   | 27 |    | 2.5000 | .8333      | .0000 | .0000 |     |   |   |    |
| 65-  | GRID   | 28 |    | 2.5000 | 1.0000     | .0000 | .0000 |     |   |   |    |
| 66-  | GRID   | 29 |    | 3.3333 | .0000      | .0000 | .0000 |     |   |   |    |
| 67-  | GRID   | 30 |    | 3.3333 | .1667      | .0000 | .0000 |     |   |   |    |
| 68-  | GRID   | 31 |    | 3.3333 | .3333      | .0000 | .0000 |     |   |   |    |

| CARD<br>COUNT | 1    | 2  | 3 | 4      | 5      | 6 | 7 | 8 | 9 | 10 |
|---------------|------|----|---|--------|--------|---|---|---|---|----|
| 69-           | CARD | 32 |   | 3.3333 | .0000  |   |   |   |   |    |
| 70-           | CARD | 33 |   | 3.3333 | .0000  |   |   |   |   |    |
| 71-           | CARD | 34 |   | 3.3333 | .0000  |   |   |   |   |    |
| 72-           | CARD | 35 |   | 3.3333 | 1.0000 |   |   |   |   |    |
| 73-           | CARD | 36 |   | 4.1667 | .0000  |   |   |   |   |    |
| 74-           | CARD | 37 |   | 4.1667 | .1667  |   |   |   |   |    |
| 75-           | CARD | 38 |   | 4.1667 | .3333  |   |   |   |   |    |
| 76-           | CARD | 39 |   | 4.1667 | .5000  |   |   |   |   |    |
| 77-           | CARD | 40 |   | 4.1667 | .6667  |   |   |   |   |    |
| 78-           | CARD | 41 |   | 4.1667 | .8333  |   |   |   |   |    |
| 79-           | CARD | 42 |   | 4.1667 | 1.0000 |   |   |   |   |    |
| 80-           | CARD | 43 |   | 5.0000 | .0000  |   |   |   |   |    |
| 81-           | CARD | 44 |   | 5.0000 | .1667  |   |   |   |   |    |
| 82-           | CARD | 45 |   | 5.0000 | .3333  |   |   |   |   |    |





PROBLEM #6

18 MAY 1968, TISONER

APR 01 54  
CPL 1.6  
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CPL 1.6

SCORES - 10 ROOF N - 8

MAY 12, 1968 RELEASE 1967 CDC PAGE 2

CASE CONTROL BACK ECHO

CARD  
COUNT

TITLE - SCORES - 10 ROOF N - 8  
DISP - ALL  
SYMBOLS - ALL  
LOAD - ALL  
SPACED - ALL  
SP - 8  
SUBCASE 1  
CARD 1  
BEGIN BUILD

8 MAY 12, 1968 RELEASE 1967 CDC PAGE 3

8 MAY 12, 1968 RELEASE 1967 CDC PAGE 3

8 MAY 12, 1968 RELEASE 1967 CDC PAGE 3

SORTED 3 1K DATA ECHO

CARD  
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| 85-  | CARD | 20 | 14.3394 | 6.2500  | 20.4788 |
| 86-  | CARD | 21 | 12.5000 | 6.2500  | 21.0506 |
| 87-  | CARD | 22 | 10.5655 | 6.2500  | 22.6577 |
| 88-  | CARD | 23 | 8.5565  | 6.2500  | 23.4923 |
| 89-  | CARD | 24 | 6.4705  | 6.2500  | 24.1481 |
| 90-  | CARD | 25 | 4.3412  | 6.2500  | 24.6202 |
| 91-  | CARD | 26 | 2.1789  | 6.2500  | 24.9049 |
| 92-  | CARD | 27 | -.0000  | 6.2500  | 25.0000 |
| 93-  | CARD | 28 | 16.0697 | 9.3750  | 19.1511 |
| 94-  | CARD | 29 | 14.3394 | 9.3750  | 20.4788 |
| 95-  | CARD | 30 | 12.5000 | 9.3750  | 21.0506 |
| 96-  | CARD | 31 | 10.5655 | 9.3750  | 22.6577 |
| 97-  | CARD | 32 | 8.5565  | 9.3750  | 23.4923 |
| 98-  | CARD | 33 | 6.4705  | 9.3750  | 24.1481 |
| 99-  | CARD | 34 | 4.3412  | 9.3750  | 24.6202 |
| 100- | CARD | 35 | 2.1789  | 9.3750  | 24.9049 |
| 101- | CARD | 36 | -.0000  | 9.3750  | 25.0000 |
| 102- | CARD | 37 | 16.0697 | 12.5000 | 19.1511 |

1 SCORDELIS - LO ROOF M - 8

MAY 12, 1968 RELEASE 1987 CDC PAGE 6

|      |       |    |         |         |         |   |   |   |   |   |    |
|------|-------|----|---------|---------|---------|---|---|---|---|---|----|
| CARD | COUNT | 1  | 2       | 3       | 4       | 5 | 6 | 7 | 8 | 9 | 10 |
| 103- | CARD  | 38 | 14.3394 | 12.5000 | 20.4788 |   |   |   |   |   |    |
| 104- | CARD  | 39 | 12.5000 | 12.5000 | 21.0506 |   |   |   |   |   |    |
| 105- | CARD  | 40 | 10.5655 | 12.5000 | 22.6577 |   |   |   |   |   |    |
| 106- | CARD  | 41 | 8.5565  | 12.5000 | 23.4923 |   |   |   |   |   |    |
| 107- | CARD  | 42 | 6.4705  | 12.5000 | 24.1481 |   |   |   |   |   |    |
| 108- | CARD  | 43 | 4.3412  | 12.5000 | 24.6202 |   |   |   |   |   |    |
| 109- | CARD  | 44 | 2.1789  | 12.5000 | 24.9049 |   |   |   |   |   |    |
| 110- | CARD  | 45 | -.0000  | 12.5000 | 25.0000 |   |   |   |   |   |    |
| 111- | CARD  | 46 | 16.0697 | 15.6250 | 19.1511 |   |   |   |   |   |    |
| 112- | CARD  | 47 | 14.3394 | 15.6250 | 20.4788 |   |   |   |   |   |    |
| 113- | CARD  | 48 | 12.5000 | 15.6250 | 21.0506 |   |   |   |   |   |    |
| 114- | CARD  | 49 | 10.5655 | 15.6250 | 22.6577 |   |   |   |   |   |    |
| 115- | CARD  | 50 | 8.5565  | 15.6250 | 23.4923 |   |   |   |   |   |    |
| 116- | CARD  | 51 | 6.4705  | 15.6250 | 24.1481 |   |   |   |   |   |    |
| 117- | CARD  | 52 | 4.3412  | 15.6250 | 24.6202 |   |   |   |   |   |    |
| 118- | CARD  | 53 | 2.1789  | 15.6250 | 24.9049 |   |   |   |   |   |    |
| 119- | CARD  | 54 | -.0000  | 15.6250 | 25.0000 |   |   |   |   |   |    |
| 120- | CARD  | 55 | 16.0697 | 18.7500 | 19.1511 |   |   |   |   |   |    |
| 121- | CARD  | 56 | 14.3394 | 18.7500 | 20.4788 |   |   |   |   |   |    |
| 122- | CARD  | 57 | 12.5000 | 18.7500 | 21.0506 |   |   |   |   |   |    |
| 123- | CARD  | 58 | 10.5655 | 18.7500 | 22.6577 |   |   |   |   |   |    |
| 124- | CARD  | 59 | 8.5565  | 18.7500 | 23.4923 |   |   |   |   |   |    |
| 125- | CARD  | 60 | 6.4705  | 18.7500 | 24.1481 |   |   |   |   |   |    |
| 126- | CARD  | 61 | 4.3412  | 18.7500 | 24.6202 |   |   |   |   |   |    |
| 127- | CARD  | 62 | 2.1789  | 18.7500 | 24.9049 |   |   |   |   |   |    |
| 128- | CARD  | 63 | -.0000  | 18.7500 | 25.0000 |   |   |   |   |   |    |
| 129- | CARD  | 64 | 16.0697 | 21.8750 | 19.1511 |   |   |   |   |   |    |
| 130- | CARD  | 65 | 14.3394 | 21.8750 | 20.4788 |   |   |   |   |   |    |
| 131- | CARD  | 66 | 12.5000 | 21.8750 | 21.0506 |   |   |   |   |   |    |
| 132- | CARD  | 67 | 10.5655 | 21.8750 | 22.6577 |   |   |   |   |   |    |
| 133- | CARD  | 68 | 8.5565  | 21.8750 | 23.4923 |   |   |   |   |   |    |
| 134- | CARD  | 69 | 6.4705  | 21.8750 | 24.1481 |   |   |   |   |   |    |
| 135- | CARD  | 70 | 4.3412  | 21.8750 | 24.6202 |   |   |   |   |   |    |
| 136- | CARD  | 71 | 2.1789  | 21.8750 | 24.9049 |   |   |   |   |   |    |

1 SCORDELIS - LO ROOF M - 8

MAY 12, 1968 RELEASE 1987 CDC PAGE 7

SORTED BULK DATA ECHO

|         |        |         |
|---------|--------|---------|
| 14-3724 | 6.2500 | 20.4788 |
| 15-3608 | 6.2500 | 21.0586 |
| 16-3555 | 6.2500 | 22.5577 |
| 17-3505 | 6.2500 | 23.4323 |
| 18-3455 | 6.2500 | 24.1481 |
| 19-3412 | 6.2500 | 24.6282 |
| 20-3369 | 6.2500 | 24.9749 |
| 21-3326 | 6.2500 | 25.0000 |
| 22-3283 | 6.2500 | 19.1511 |
| 23-3240 | 6.2500 | 19.4788 |
| 24-3197 | 6.2500 | 21.0586 |
| 25-3154 | 6.2500 | 21.6577 |
| 26-3111 | 6.2500 | 23.4323 |
| 27-3068 | 6.2500 | 24.1481 |
| 28-3025 | 6.2500 | 24.6282 |
| 29-2982 | 6.2500 | 24.9749 |
| 30-2939 | 6.2500 | 25.0000 |
| 31-2896 | 6.2500 | 19.1511 |

SCHEMATIC - 10 6000 N. E

MAY 12, 1955 RELEASE 1987 CDC PAGE 6

|         |        |         |
|---------|--------|---------|
| 14-3724 | 6.2500 | 20.4788 |
| 15-3608 | 6.2500 | 21.0586 |
| 16-3555 | 6.2500 | 22.5577 |
| 17-3505 | 6.2500 | 23.4323 |
| 18-3455 | 6.2500 | 24.1481 |
| 19-3412 | 6.2500 | 24.6282 |
| 20-3369 | 6.2500 | 24.9749 |
| 21-3326 | 6.2500 | 25.0000 |
| 22-3283 | 6.2500 | 19.1511 |
| 23-3240 | 6.2500 | 19.4788 |
| 24-3197 | 6.2500 | 21.0586 |
| 25-3154 | 6.2500 | 21.6577 |
| 26-3111 | 6.2500 | 23.4323 |
| 27-3068 | 6.2500 | 24.1481 |
| 28-3025 | 6.2500 | 24.6282 |
| 29-2982 | 6.2500 | 24.9749 |
| 30-2939 | 6.2500 | 25.0000 |
| 31-2896 | 6.2500 | 19.1511 |

SORTED BULK DATA

COUNT

|         |        |         |
|---------|--------|---------|
| 14-3724 | 6.2500 | 20.4788 |
| 15-3608 | 6.2500 | 21.0586 |
| 16-3555 | 6.2500 | 22.5577 |
| 17-3505 | 6.2500 | 23.4323 |
| 18-3455 | 6.2500 | 24.1481 |
| 19-3412 | 6.2500 | 24.6282 |
| 20-3369 | 6.2500 | 24.9749 |
| 21-3326 | 6.2500 | 25.0000 |
| 22-3283 | 6.2500 | 19.1511 |
| 23-3240 | 6.2500 | 19.4788 |
| 24-3197 | 6.2500 | 21.0586 |
| 25-3154 | 6.2500 | 21.6577 |
| 26-3111 | 6.2500 | 23.4323 |
| 27-3068 | 6.2500 | 24.1481 |
| 28-3025 | 6.2500 | 24.6282 |
| 29-2982 | 6.2500 | 24.9749 |
| 30-2939 | 6.2500 | 25.0000 |
| 31-2896 | 6.2500 | 19.1511 |

SCHEMATIC - 10 6000 N. E

MAY 12, 1955 RELEASE 1987 CDC PAGE 7

SORTED BULK DATA

PROBLEM #6

| CARD<br>COUNT | 1    | 2  | 3       | 4       | 5        | 6        | 7   | 8  | 9    | 10 |
|---------------|------|----|---------|---------|----------|----------|-----|----|------|----|
| 137-          | GRID | 72 | 16.0697 | 21.8750 | 25.0000  | 25.0000  |     |    |      |    |
| 138-          | GRID | 73 | 14.3394 | 25.0000 | 20.4788  | 19.1511  |     |    |      |    |
| 139-          | GRID | 74 | 12.5000 | 25.0000 | 21.6506  | 20.4788  |     |    |      |    |
| 140-          | GRID | 75 | 10.5658 | 25.0000 | 22.6577  | 21.6506  |     |    |      |    |
| 141-          | GRID | 76 | 8.5506  | 25.0000 | 23.4823  | 22.6577  |     |    |      |    |
| 142-          | GRID | 77 | 6.4705  | 25.0000 | 24.1481  | 23.4823  |     |    |      |    |
| 143-          | GRID | 78 | 4.3412  | 25.0000 | 24.6302  | 24.1481  |     |    |      |    |
| 144-          | GRID | 79 | 2.1789  | 25.0000 | 24.9849  | 24.6302  |     |    |      |    |
| 145-          | GRID | 80 | 0.0000  | 25.0000 | 25.0000  | 24.9849  |     |    |      |    |
| 146-          | GRID | 81 | 4.38    | 25.0000 | 11.18012 | 11.18012 | 1.0 | .9 |      |    |
| 147-          | GRID | 1  | 1       | 25      | 1        | 1        | 5   | 6  | +BCD |    |
| 148-          | GRID | 2  | 13      | 1       | 3        | 4        | 45  | 54 | +CDE |    |
| 149-          | GRID | 3  | 8       | 9       | 18       | 36       | 77  | 78 | +ABC |    |
| 150-          | GRID | 4  | 15      | 9       | 74       | 75       |     |    |      |    |
| 151-          | GRID | 5  | 72      | 81      |          |          |     |    |      |    |
| 152-          | GRID | 63 | 24      | 81      |          |          |     |    |      |    |
| 153-          | GRID | 5  | 80      |         |          |          |     |    |      |    |
| 154-          | GRID | 79 |         |         |          |          |     |    |      |    |
| 155-          | GRID |    |         |         |          |          |     |    |      |    |

ENDATA

10 UNIFORM PROBLEM 1

SOL 1.0  
MPC DISP  
TIME 30  
END

LAMINATED COMPOSITE PLATE  
PURE TUIST LOADING

MARCH 22, 1989 RELEASE 1988 CDC PAGE 2

CASE CONTROL DECK ECHO

CARD  
COUNT

TITLE - LAMINATED COMPOSITE PLATE  
SUBTITLE - PURE TUIST LOADING

MODEL - A SQUARE PLATE OF A 4X4 MESH WITH THREE CORNERS  
PINNED AND A TRANSVERSE POINT LOAD AT THE FREE  
CORNER TO SIMULATE A PURE TUIST LOADING. THE  
LAMINATE LAYUP IS OF A CROSS-PLY CONFIGURATION  
[0/90/0].

1 2 COMPARISON OF T3 DEFLECTION AT GRID 1 2 2

| UNIT/NASTRAN | MSC/NASTRAN | THEORETICAL |
|--------------|-------------|-------------|
| -3.756E-2    | -3.763E-2   | -3.750E-2   |

1 2 COMPARISON OF TAU FOR ELEMENT 1, ALL LAYERS 2 2

| UNIT/NASTRAN | MSC/NASTRAN | THEORETICAL |
|--------------|-------------|-------------|
| -5.0E1       | -5.0E1      | -5.0E1      |
| 0.0          | 0.0         | 0.0         |
| 5.0E1        | 5.0E1       | 5.0E1       |

REFERENCES: JONES R. M., MECHANICS OF COMPOSITE MATERIALS.  
M. GRAU-HILL BOOK COMPANY. (PAGE 181)

SFC - 1

SURFACE 1  
LABEL - LAYER STRESS REQUEST

LAMINATED COMPOSITE PLATE  
PURE TUIST LOADING

MARCH 22, 1989 RELEASE 1988 CDC PAGE 3

CASE CONTROL DECK ECHO

CARD  
COUNT

DISP - ALL  
STRESS(LAYER) - ALL  
FORCE - ALL  
LOAD - 1  
BEGIN BULK  
LAYER BULK

LAMINATED COMPOSITE PLATE  
PURE TUIST LOADING

MARCH 22, 1989 RELEASE 1988 CDC PAGE 4

USER INFORMATION MESSAGE 207, BULK DATA NOT SORTED, XSCRT WILL RE-ORDER DECK.  
LAMINATED COMPOSITE PLATE

MARCH 22, 1989 RELEASE 1988 CDC PAGE 5

 Springer

COPIES

[illegible]





13- 13- COUADA 12 1 13 18 19 21 14 19 1  
 14- 14- COUADA 13 1 18 23 21 10 5 19 1  
 15- 15- COUADA 14 1 4 9 14 10 5 15 1  
 16- 16- COUADA 15 1 9 14 10 5 15 1  
 17- 17- COUADA 16 1 1 14 19 20 15 1  
 18- 18- COUADA 17 1 19 24 25 20 1  
 19- 19- GRID 1 0.000 0.000 0.000 0.000  
 20- 20- GRID 2 .250 0.000 .250 0.000  
 21- 21- GRID 3 0.000 0.000 .500 0.000  
 22- 22- GRID 4 0.000 0.000 .750 0.000  
 23- 23- GRID 5 0.000 0.000 1.000 0.000  
 24- 24- GRID 6 .250 0.000 0.000 0.000  
 25- 25- GRID 7 .250 0.000 .250 0.000  
 26- 26- GRID 8 .250 0.000 .250 0.000  
 27- 27- GRID 9 .250 0.000 .250 0.000  
 28- 28- GRID 10 .250 0.000 .250 0.000  
 29- 29- GRID 11 .500 0.000 .500 0.000  
 30- 30- GRID 12 .500 0.000 .500 0.000  
 31- 31- GRID 13 .500 0.000 .500 0.000  
 32- 32- GRID 14 .500 0.000 .500 0.000  
 33- 33- GRID 15 .500 0.000 .500 0.000  
 34- 34- GRID 16 .750 0.000 .750 0.000

1 COMP01 \*\*\* UAI \*\*\* QUADA 4 FLAT PLATE TEST  
 0 MESH 4X4 , ASPECT RATIO 1.0 SYMM CROSS-PLY [0/00/0]  
 0

MARCH 22, 1989 RELEASE 1988 CDC PAGE 5

SORTED BULK DATA ECHO  
 ---1--- ++2++ ---3--- ++4++ ---5--- ++6++ ---7--- ++8++ ---9--- ++10++  
 GRID 17 .750 .250 0.000  
 GRID 18 .750 .500 0.000  
 GRID 19 .750 .750 0.000  
 GRID 20 .750 1.000 0.000  
 GRID 21 1.000 0.000 0.000  
 GRID 22 1.000 .250 0.000  
 GRID 23 1.000 .500 0.000  
 GRID 24 1.000 .750 0.000  
 GRID 25 1.000 1.000 0.000  
 NAT8 1 20.0E+06.50 E+6.25 .250 E+6  
 PCOMP 1 -.001  
 +PC1 1 .000666 0.0 YES +PC1  
 +PC2 1 .000666 0.0 YES +PC2  
 PLOAD4 1 2 -1.0E-04 THRU 17  
 SPC1 1 6 1 THRU 25  
 SPC1 1 15 22 23 24  
 SPC1 1 24 10 15 20  
 SPC1 1 1234 2 3 4 5  
 SPC1 1 1235 6 11 16 21  
 SPC1 1 1245 25  
 SPC1 1 12345  
 ENDDATA

ID UAI, NASTRAN PROBLEM 4

SOL 1.0  
APP DISP  
TIME 30  
CEND

COMP04 111 UAI 111 QUAD4 4-NODE STRAIGHT BEAM TEST

MARCH 22, 1989 RELEASE 1988 CDC PAGE 2

REGULAR SHAPE ELEMENTS ( ISOTROPIC PROPERTIES )

CASE CONTROL DECK ECHO

CARD  
COUNT

TITLE = COMP04 111 UAI 111 QUAD4 4-NODE STRAIGHT BEAM TEST  
LABEL = REGULAR SHAPE ELEMENTS ( ISOTROPIC PROPERTIES )

MODEL: CANTILEVERED BEAM MODEL UNDER A) EXTENSIONAL AND  
B) BENDING LOADINGS. SIMULATION OF EQUIVALENT  
ISOTROPIC PROPERTIES. LAMINATE CONFIGURATION  
[0/0/0/0]

1 1 COMPARISON OF T1 DEFLECTION AT GRIDS 13 AND 14 1 1

UAI/NASTRAN MSC/NASTRAN THEORETICAL

SUBCASE 1 (EXTENSIONAL)

GRID 13 2.986E-5 2.986E-5 3.0E-5  
GRID 14 2.986E-5 2.986E-5 3.0E-5

1 1 COMPARISON OF T3 DEFLECTION AT GRIDS 13 AND 14 1 1

UAI/NASTRAN MSC/NASTRAN THEORETICAL

SUBCASE 2 (BENDING)

GRID 13 4.253E-1 4.253E-1 4.320E-1  
GRID 14 4.253E-1 4.253E-1 4.320E-1

1 1 COMPARISON OF BENDING MOMENT DISTRIBUTION FROM 1 1  
1 1 THE FREE END TO THE CANTILEVERED END 1 1  
NOTE: THE BENDING MOMENTS ARE AT THE ELEMENT CENTER

UAI/NASTRAN MSC/NASTRAN THEORETICAL

COMP04 111 UAI 111 QUAD4 4-NODE STRAIGHT BEAM TEST

MARCH 22, 1989 RELEASE 1988 CDC PAGE 3

REGULAR SHAPE ELEMENTS ( ISOTROPIC PROPERTIES )

CASE CONTROL DECK ECHO

CARD  
COUNT

7.500E0 7.500E0 7.500E0  
1.250E1 1.250E1 1.250E1  
1.750E1 1.750E1 1.750E1  
2.250E1 2.250E1 2.250E1  
2.750E1 2.750E1 2.750E1

1 1 COMPARISON OF DIRECT LAYER BENDING STRESS 1 1  
1 1 ELEMENT 6 (LARGEST BENDING MOMENT) 1 1

UAI/NASTRAN MSC/NASTRAN

```

46 8
47 8
48 8 LAYER 1 1.238E4 1.238E4
49 8 LAYER 2 4.125E3 4.125E3
50 8 LAYER 3 -4.125E3 -4.125E3
51 8 LAYER 4 -1.238E4 -1.238E4
52 8
53 8
54 8 STRESS(LAYER) = ALL
55 8 DISP = ALL
56 8 FORCE = ALL
57 8 SPC = 1
58 8 SUBCASE 1
59 8 SUBTITLE = EXTENSION
60 8 LOAD = 1
61 8 SUBCASE 2
62 8 SUBTITLE = OUT-OF-PLANE SHEAR
63 8 LOAD = 2
64 8 BEGIN BULK
65 8 COMP04 111 QUAD4 4-NODE STRAIGHT BEAM TEST

```

MARCH 22, 1989 RELEASE 1988 CDC PAGE 4

```

0
0
0 111 USER INFORMATION MESSAGE 207, BULK DATA NOT SORTED, XSORT WILL RE-ORDER DECK.
0 111 COMP04 111 QUAD4 4-NODE STRAIGHT BEAM TEST

```

MARCH 22, 1989 RELEASE 1988 CDC PAGE 5

REGULAR SHAPE ELEMENTS ( ISOTROPIC PROPERTIES )

| CARD | COUNT   | --- | 1---    | ++2++ | --- | 3--- | 0.0 | ++4++ | --- | 5--- | 0.0 | ++6++ | --- | 7--- | 0.0 | ++8++ | --- | 9--- | ++10++ |
|------|---------|-----|---------|-------|-----|------|-----|-------|-----|------|-----|-------|-----|------|-----|-------|-----|------|--------|
| 1-   | CORNER  | 1   | 0.0     | 1.0   | 0.0 | 0.0  | 1.0 | 0.0   | 0.0 | 0.0  | 0.0 | 0.0   | 0.0 | 0.0  | 0.0 | 0.0   | 0.0 | 0.0  | 0.0    |
| 2-   | ABC     | 1   | 0.0     | 1.0   | 0.0 | 0.0  | 1.0 | 0.0   | 0.0 | 0.0  | 0.0 | 0.0   | 0.0 | 0.0  | 0.0 | 0.0   | 0.0 | 0.0  | 0.0    |
| 3-   | QUAD4   | 1   | 1       | 3     | 5   | 7    | 9   | 11    | 13  | 14   | 4   | 6     | 8   | 10   | 12  | 1     | 1   | 1    | 1      |
| 4-   | QUAD4   | 2   | 1       | 3     | 5   | 7    | 9   | 11    | 13  | 14   | 4   | 6     | 8   | 10   | 12  | 1     | 1   | 1    | 1      |
| 5-   | QUAD4   | 3   | 1       | 3     | 5   | 7    | 9   | 11    | 13  | 14   | 4   | 6     | 8   | 10   | 12  | 1     | 1   | 1    | 1      |
| 6-   | QUAD4   | 4   | 1       | 3     | 5   | 7    | 9   | 11    | 13  | 14   | 4   | 6     | 8   | 10   | 12  | 1     | 1   | 1    | 1      |
| 7-   | QUAD4   | 5   | 1       | 3     | 5   | 7    | 9   | 11    | 13  | 14   | 4   | 6     | 8   | 10   | 12  | 1     | 1   | 1    | 1      |
| 8-   | QUAD4   | 6   | 1       | 3     | 5   | 7    | 9   | 11    | 13  | 14   | 4   | 6     | 8   | 10   | 12  | 1     | 1   | 1    | 1      |
| 9-   | FORCE   | 1   | 13      | 13    | 13  | 13   | 13  | 13    | 13  | 13   | 1.0 | 0.0   | 0.0 | 0.0  | 0.0 | 0.0   | 0.0 | 0.0  | 0.0    |
| 10-  | FORCE   | 1   | 13      | 13    | 13  | 13   | 13  | 13    | 13  | 13   | 1.0 | 0.0   | 0.0 | 0.0  | 0.0 | 0.0   | 0.0 | 0.0  | 0.0    |
| 11-  | FORCE   | 1   | 13      | 13    | 13  | 13   | 13  | 13    | 13  | 13   | 1.0 | 0.0   | 0.0 | 0.0  | 0.0 | 0.0   | 0.0 | 0.0  | 0.0    |
| 12-  | FORCE   | 1   | 13      | 13    | 13  | 13   | 13  | 13    | 13  | 13   | 1.0 | 0.0   | 0.0 | 0.0  | 0.0 | 0.0   | 0.0 | 0.0  | 0.0    |
| 13-  | GRID    | 1   | 2       | 2     | 2   | 2    | 2   | 2     | 2   | 2    | 0.0 | 0.0   | 0.0 | 0.0  | 0.0 | 0.0   | 0.0 | 0.0  | 0.0    |
| 14-  | GRID    | 2   | 2       | 2     | 2   | 2    | 2   | 2     | 2   | 2    | 0.0 | 0.0   | 0.0 | 0.0  | 0.0 | 0.0   | 0.0 | 0.0  | 0.0    |
| 15-  | GRID    | 3   | 2       | 2     | 2   | 2    | 2   | 2     | 2   | 2    | 0.0 | 0.0   | 0.0 | 0.0  | 0.0 | 0.0   | 0.0 | 0.0  | 0.0    |
| 16-  | GRID    | 4   | 2       | 2     | 2   | 2    | 2   | 2     | 2   | 2    | 0.0 | 0.0   | 0.0 | 0.0  | 0.0 | 0.0   | 0.0 | 0.0  | 0.0    |
| 17-  | GRID    | 5   | 2       | 2     | 2   | 2    | 2   | 2     | 2   | 2    | 0.0 | 0.0   | 0.0 | 0.0  | 0.0 | 0.0   | 0.0 | 0.0  | 0.0    |
| 18-  | GRID    | 6   | 2       | 2     | 2   | 2    | 2   | 2     | 2   | 2    | 0.0 | 0.0   | 0.0 | 0.0  | 0.0 | 0.0   | 0.0 | 0.0  | 0.0    |
| 19-  | GRID    | 7   | 2       | 2     | 2   | 2    | 2   | 2     | 2   | 2    | 0.0 | 0.0   | 0.0 | 0.0  | 0.0 | 0.0   | 0.0 | 0.0  | 0.0    |
| 20-  | GRID    | 8   | 2       | 2     | 2   | 2    | 2   | 2     | 2   | 2    | 0.0 | 0.0   | 0.0 | 0.0  | 0.0 | 0.0   | 0.0 | 0.0  | 0.0    |
| 21-  | GRID    | 9   | 2       | 2     | 2   | 2    | 2   | 2     | 2   | 2    | 0.0 | 0.0   | 0.0 | 0.0  | 0.0 | 0.0   | 0.0 | 0.0  | 0.0    |
| 22-  | GRID    | 10  | 2       | 2     | 2   | 2    | 2   | 2     | 2   | 2    | 0.0 | 0.0   | 0.0 | 0.0  | 0.0 | 0.0   | 0.0 | 0.0  | 0.0    |
| 23-  | GRID    | 11  | 2       | 2     | 2   | 2    | 2   | 2     | 2   | 2    | 0.0 | 0.0   | 0.0 | 0.0  | 0.0 | 0.0   | 0.0 | 0.0  | 0.0    |
| 24-  | GRID    | 12  | 2       | 2     | 2   | 2    | 2   | 2     | 2   | 2    | 0.0 | 0.0   | 0.0 | 0.0  | 0.0 | 0.0   | 0.0 | 0.0  | 0.0    |
| 25-  | GRID    | 13  | 2       | 2     | 2   | 2    | 2   | 2     | 2   | 2    | 0.0 | 0.0   | 0.0 | 0.0  | 0.0 | 0.0   | 0.0 | 0.0  | 0.0    |
| 26-  | GRID    | 14  | 2       | 2     | 2   | 2    | 2   | 2     | 2   | 2    | 0.0 | 0.0   | 0.0 | 0.0  | 0.0 | 0.0   | 0.0 | 0.0  | 0.0    |
| 27-  | MAT1    | 1   | 100E+08 | 6.0   | 6.0 | 6.0  | 6.0 | 6.0   | 6.0 | 6.0  | 0.0 | 0.0   | 0.0 | 0.0  | 0.0 | 0.0   | 0.0 | 0.0  | 0.0    |
| 28-  | PCOMP2  | 1   | 0.0     | 0.025 | 0.0 | 0.0  | 0.0 | 0.0   | 0.0 | 0.0  | 0.0 | 0.0   | 0.0 | 0.0  | 0.0 | 0.0   | 0.0 | 0.0  | 0.0    |
| 29-  | SPC1    | 1   | 0.0     | 0.025 | 0.0 | 0.0  | 0.0 | 0.0   | 0.0 | 0.0  | 0.0 | 0.0   | 0.0 | 0.0  | 0.0 | 0.0   | 0.0 | 0.0  | 0.0    |
| 30-  | SPC1    | 1   | 6       | 1     | 1   | 1    | 1   | 1     | 1   | 1    | 0.0 | 0.0   | 0.0 | 0.0  | 0.0 | 0.0   | 0.0 | 0.0  | 0.0    |
| 31-  | ENDDATA | 1   | 123456  | 1     | 2   | 2    | 2   | 2     | 2   | 2    | 0.0 | 0.0   | 0.0 | 0.0  | 0.0 | 0.0   | 0.0 | 0.0  | 0.0    |

SYM +PC1

NASTRAN EXECUTIVE CONTROL DECK ECHO

10 COMPOSITE PLATE 15 BY 20 NASTRAN USERS COLLOQUIUM 1990  
 APP DISPLACEMENT  
 SOL 1.0  
 TIME 10  
 GENO

CASE CONTROL DECK ECHO

CARD  
 COUNT

TITLE = PROBLEM 13 CASE 3 COMBINED LOADING  
 LOAD = 1  
 SPC = 1  
 OUTPUT  
 QUAD=ALL  
 STRESS=ALL  
 DISPLACEMENTS=ALL  
 STRAIN=ALL  
 BEGIN BULK

| CARD | COUNT | --- | 1--- | --- | 2--- | --- | 3--- | --- | 4--- | --- | 5--- | --- | 6--- | --- | 7--- | --- | 8--- | --- | 9--- | --- | 10--- |
|------|-------|-----|------|-----|------|-----|------|-----|------|-----|------|-----|------|-----|------|-----|------|-----|------|-----|-------|
| 1    | 1     | --- | 1    | --- | 1    | --- | 1    | --- | 1    | --- | 1    | --- | 1    | --- | 1    | --- | 1    | --- | 1    | --- | 1     |
| 2    | 2     | --- | 2    | --- | 2    | --- | 2    | --- | 2    | --- | 2    | --- | 2    | --- | 2    | --- | 2    | --- | 2    | --- | 2     |
| 3    | 3     | --- | 3    | --- | 3    | --- | 3    | --- | 3    | --- | 3    | --- | 3    | --- | 3    | --- | 3    | --- | 3    | --- | 3     |
| 4    | 4     | --- | 4    | --- | 4    | --- | 4    | --- | 4    | --- | 4    | --- | 4    | --- | 4    | --- | 4    | --- | 4    | --- | 4     |
| 5    | 5     | --- | 5    | --- | 5    | --- | 5    | --- | 5    | --- | 5    | --- | 5    | --- | 5    | --- | 5    | --- | 5    | --- | 5     |
| 6    | 6     | --- | 6    | --- | 6    | --- | 6    | --- | 6    | --- | 6    | --- | 6    | --- | 6    | --- | 6    | --- | 6    | --- | 6     |
| 7    | 7     | --- | 7    | --- | 7    | --- | 7    | --- | 7    | --- | 7    | --- | 7    | --- | 7    | --- | 7    | --- | 7    | --- | 7     |
| 8    | 8     | --- | 8    | --- | 8    | --- | 8    | --- | 8    | --- | 8    | --- | 8    | --- | 8    | --- | 8    | --- | 8    | --- | 8     |
| 9    | 9     | --- | 9    | --- | 9    | --- | 9    | --- | 9    | --- | 9    | --- | 9    | --- | 9    | --- | 9    | --- | 9    | --- | 9     |
| 10   | 10    | --- | 10   | --- | 10   | --- | 10   | --- | 10   | --- | 10   | --- | 10   | --- | 10   | --- | 10   | --- | 10   | --- | 10    |
| 11   | 11    | --- | 11   | --- | 11   | --- | 11   | --- | 11   | --- | 11   | --- | 11   | --- | 11   | --- | 11   | --- | 11   | --- | 11    |
| 12   | 12    | --- | 12   | --- | 12   | --- | 12   | --- | 12   | --- | 12   | --- | 12   | --- | 12   | --- | 12   | --- | 12   | --- | 12    |
| 13   | 13    | --- | 13   | --- | 13   | --- | 13   | --- | 13   | --- | 13   | --- | 13   | --- | 13   | --- | 13   | --- | 13   | --- | 13    |
| 14   | 14    | --- | 14   | --- | 14   | --- | 14   | --- | 14   | --- | 14   | --- | 14   | --- | 14   | --- | 14   | --- | 14   | --- | 14    |
| 15   | 15    | --- | 15   | --- | 15   | --- | 15   | --- | 15   | --- | 15   | --- | 15   | --- | 15   | --- | 15   | --- | 15   | --- | 15    |
| 16   | 16    | --- | 16   | --- | 16   | --- | 16   | --- | 16   | --- | 16   | --- | 16   | --- | 16   | --- | 16   | --- | 16   | --- | 16    |
| 17   | 17    | --- | 17   | --- | 17   | --- | 17   | --- | 17   | --- | 17   | --- | 17   | --- | 17   | --- | 17   | --- | 17   | --- | 17    |
| 18   | 18    | --- | 18   | --- | 18   | --- | 18   | --- | 18   | --- | 18   | --- | 18   | --- | 18   | --- | 18   | --- | 18   | --- | 18    |
| 19   | 19    | --- | 19   | --- | 19   | --- | 19   | --- | 19   | --- | 19   | --- | 19   | --- | 19   | --- | 19   | --- | 19   | --- | 19    |
| 20   | 20    | --- | 20   | --- | 20   | --- | 20   | --- | 20   | --- | 20   | --- | 20   | --- | 20   | --- | 20   | --- | 20   | --- | 20    |
| 21   | 21    | --- | 21   | --- | 21   | --- | 21   | --- | 21   | --- | 21   | --- | 21   | --- | 21   | --- | 21   | --- | 21   | --- | 21    |
| 22   | 22    | --- | 22   | --- | 22   | --- | 22   | --- | 22   | --- | 22   | --- | 22   | --- | 22   | --- | 22   | --- | 22   | --- | 22    |
| 23   | 23    | --- | 23   | --- | 23   | --- | 23   | --- | 23   | --- | 23   | --- | 23   | --- | 23   | --- | 23   | --- | 23   | --- | 23    |
| 24   | 24    | --- | 24   | --- | 24   | --- | 24   | --- | 24   | --- | 24   | --- | 24   | --- | 24   | --- | 24   | --- | 24   | --- | 24    |
| 25   | 25    | --- | 25   | --- | 25   | --- | 25   | --- | 25   | --- | 25   | --- | 25   | --- | 25   | --- | 25   | --- | 25   | --- | 25    |
| 26   | 26    | --- | 26   | --- | 26   | --- | 26   | --- | 26   | --- | 26   | --- | 26   | --- | 26   | --- | 26   | --- | 26   | --- | 26    |
| 27   | 27    | --- | 27   | --- | 27   | --- | 27   | --- | 27   | --- | 27   | --- | 27   | --- | 27   | --- | 27   | --- | 27   | --- | 27    |
| 28   | 28    | --- | 28   | --- | 28   | --- | 28   | --- | 28   | --- | 28   | --- | 28   | --- | 28   | --- | 28   | --- | 28   | --- | 28    |
| 29   | 29    | --- | 29   | --- | 29   | --- | 29   | --- | 29   | --- | 29   | --- | 29   | --- | 29   | --- | 29   | --- | 29   | --- | 29    |
| 30   | 30    | --- | 30   | --- | 30   | --- | 30   | --- | 30   | --- | 30   | --- | 30   | --- | 30   | --- | 30   | --- | 30   | --- | 30    |
| 31   | 31    | --- | 31   | --- | 31   | --- | 31   | --- | 31   | --- | 31   | --- | 31   | --- | 31   | --- | 31   | --- | 31   | --- | 31    |
| 32   | 32    | --- | 32   | --- | 32   | --- | 32   | --- | 32   | --- | 32   | --- | 32   | --- | 32   | --- | 32   | --- | 32   | --- | 32    |
| 33   | 33    | --- | 33   | --- | 33   | --- | 33   | --- | 33   | --- | 33   | --- | 33   | --- | 33   | --- | 33   | --- | 33   | --- | 33    |
| 34   | 34    | --- | 34   | --- | 34   | --- | 34   | --- | 34   | --- | 34   | --- | 34   | --- | 34   | --- | 34   | --- | 34   | --- | 34    |
| 35   | 35    | --- | 35   | --- | 35   | --- | 35   | --- | 35   | --- | 35   | --- | 35   | --- | 35   | --- | 35   | --- | 35   | --- | 35    |
| 36   | 36    | --- | 36   | --- | 36   | --- | 36   | --- | 36   | --- | 36   | --- | 36   | --- | 36   | --- | 36   | --- | 36   | --- | 36    |

[illegible]

APPENDIX E: NASTRAN INPUT AND OUTPUT FOR SAMPLE  
PROBLEMS 7, 10, 12, 13a, 13b, 13d, 14, and 9 and 12 with TRIA3

1 NASTRAM EXECUTIVE CONTROL DECK ECHO

PROBLEM 47

13 NASTRAM, FISCHLER  
APP DIS  
SOL 1  
TIME 0  
END

CASE CONTROL DECK E C M G

CARD  
COUNT

TITLE - SPHERICAL SHELL - N=8  
 DISP - ALL  
 STRESS - ALL  
 DLOAD - ALL  
 SUPPORTS - ALL  
 SRC - 5  
 SUBCASE 1  
 LOAD 1  
 BEGIN BULK





| CARD | COUNT | 1 | 2   | 3   | 4   | 5   | 6   | 7   | 8 | 9 | 10 |
|------|-------|---|-----|-----|-----|-----|-----|-----|---|---|----|
| 35-  | 35-   | 1 | 33  | 37  | 38  | 47  | 46  | 46  |   |   |    |
| 36-  | 36-   | 1 | 34  | 38  | 39  | 48  | 47  | 47  |   |   |    |
| 37-  | 37-   | 1 | 35  | 39  | 40  | 49  | 48  | 48  |   |   |    |
| 38-  | 38-   | 1 | 36  | 40  | 41  | 50  | 49  | 49  |   |   |    |
| 39-  | 39-   | 1 | 37  | 41  | 42  | 51  | 50  | 50  |   |   |    |
| 40-  | 40-   | 1 | 38  | 42  | 43  | 52  | 51  | 51  |   |   |    |
| 41-  | 41-   | 1 | 39  | 43  | 44  | 53  | 52  | 52  |   |   |    |
| 42-  | 42-   | 1 | 40  | 44  | 45  | 54  | 53  | 53  |   |   |    |
| 43-  | 43-   | 1 | 41  | 45  | 46  | 55  | 54  | 54  |   |   |    |
| 44-  | 44-   | 1 | 42  | 46  | 47  | 56  | 55  | 55  |   |   |    |
| 45-  | 45-   | 1 | 43  | 47  | 48  | 57  | 56  | 56  |   |   |    |
| 46-  | 46-   | 1 | 44  | 48  | 49  | 58  | 57  | 57  |   |   |    |
| 47-  | 47-   | 1 | 45  | 49  | 50  | 59  | 58  | 58  |   |   |    |
| 48-  | 48-   | 1 | 46  | 50  | 51  | 60  | 59  | 59  |   |   |    |
| 49-  | 49-   | 1 | 47  | 51  | 52  | 61  | 60  | 60  |   |   |    |
| 50-  | 50-   | 1 | 48  | 52  | 53  | 62  | 61  | 61  |   |   |    |
| 51-  | 51-   | 1 | 49  | 53  | 54  | 63  | 62  | 62  |   |   |    |
| 52-  | 52-   | 1 | 50  | 54  | 55  | 64  | 63  | 63  |   |   |    |
| 53-  | 53-   | 1 | 51  | 55  | 56  | 65  | 64  | 64  |   |   |    |
| 54-  | 54-   | 1 | 52  | 56  | 57  | 66  | 65  | 65  |   |   |    |
| 55-  | 55-   | 1 | 53  | 57  | 58  | 67  | 66  | 66  |   |   |    |
| 56-  | 56-   | 1 | 54  | 58  | 59  | 68  | 67  | 67  |   |   |    |
| 57-  | 57-   | 1 | 55  | 59  | 60  | 69  | 68  | 68  |   |   |    |
| 58-  | 58-   | 1 | 56  | 60  | 61  | 70  | 69  | 69  |   |   |    |
| 59-  | 59-   | 1 | 57  | 61  | 62  | 71  | 70  | 70  |   |   |    |
| 60-  | 60-   | 1 | 58  | 62  | 63  | 72  | 71  | 71  |   |   |    |
| 61-  | 61-   | 1 | 59  | 63  | 64  | 73  | 72  | 72  |   |   |    |
| 62-  | 62-   | 1 | 60  | 64  | 65  | 74  | 73  | 73  |   |   |    |
| 63-  | 63-   | 1 | 61  | 65  | 66  | 75  | 74  | 74  |   |   |    |
| 64-  | 64-   | 1 | 62  | 66  | 67  | 76  | 75  | 75  |   |   |    |
| 65-  | 65-   | 1 | 63  | 67  | 68  | 77  | 76  | 76  |   |   |    |
| 66-  | 66-   | 1 | 64  | 68  | 69  | 78  | 77  | 77  |   |   |    |
| 67-  | 67-   | 1 | 65  | 69  | 70  | 79  | 78  | 78  |   |   |    |
| 68-  | 68-   | 1 | 66  | 70  | 71  | 80  | 79  | 79  |   |   |    |
|      |       | 1 | 67  | 71  | 72  | 81  | 80  | 80  |   |   |    |
|      |       | 1 | 68  | 72  | 73  | 82  | 81  | 81  |   |   |    |
|      |       | 1 | 69  | 73  | 74  | 83  | 82  | 82  |   |   |    |
|      |       | 1 | 70  | 74  | 75  | 84  | 83  | 83  |   |   |    |
|      |       | 1 | 71  | 75  | 76  | 85  | 84  | 84  |   |   |    |
|      |       | 1 | 72  | 76  | 77  | 86  | 85  | 85  |   |   |    |
|      |       | 1 | 73  | 77  | 78  | 87  | 86  | 86  |   |   |    |
|      |       | 1 | 74  | 78  | 79  | 88  | 87  | 87  |   |   |    |
|      |       | 1 | 75  | 79  | 80  | 89  | 88  | 88  |   |   |    |
|      |       | 1 | 76  | 80  | 81  | 90  | 89  | 89  |   |   |    |
|      |       | 1 | 77  | 81  | 82  | 91  | 90  | 90  |   |   |    |
|      |       | 1 | 78  | 82  | 83  | 92  | 91  | 91  |   |   |    |
|      |       | 1 | 79  | 83  | 84  | 93  | 92  | 92  |   |   |    |
|      |       | 1 | 80  | 84  | 85  | 94  | 93  | 93  |   |   |    |
|      |       | 1 | 81  | 85  | 86  | 95  | 94  | 94  |   |   |    |
|      |       | 1 | 82  | 86  | 87  | 96  | 95  | 95  |   |   |    |
|      |       | 1 | 83  | 87  | 88  | 97  | 96  | 96  |   |   |    |
|      |       | 1 | 84  | 88  | 89  | 98  | 97  | 97  |   |   |    |
|      |       | 1 | 85  | 89  | 90  | 99  | 98  | 98  |   |   |    |
|      |       | 1 | 86  | 90  | 91  | 100 | 99  | 99  |   |   |    |
|      |       | 1 | 87  | 91  | 92  | 101 | 100 | 100 |   |   |    |
|      |       | 1 | 88  | 92  | 93  | 102 | 101 | 101 |   |   |    |
|      |       | 1 | 89  | 93  | 94  | 103 | 102 | 102 |   |   |    |
|      |       | 1 | 90  | 94  | 95  | 104 | 103 | 103 |   |   |    |
|      |       | 1 | 91  | 95  | 96  | 105 | 104 | 104 |   |   |    |
|      |       | 1 | 92  | 96  | 97  | 106 | 105 | 105 |   |   |    |
|      |       | 1 | 93  | 97  | 98  | 107 | 106 | 106 |   |   |    |
|      |       | 1 | 94  | 98  | 99  | 108 | 107 | 107 |   |   |    |
|      |       | 1 | 95  | 99  | 100 | 109 | 108 | 108 |   |   |    |
|      |       | 1 | 96  | 100 | 101 | 110 | 109 | 109 |   |   |    |
|      |       | 1 | 97  | 101 | 102 | 111 | 110 | 110 |   |   |    |
|      |       | 1 | 98  | 102 | 103 | 112 | 111 | 111 |   |   |    |
|      |       | 1 | 99  | 103 | 104 | 113 | 112 | 112 |   |   |    |
|      |       | 1 | 100 | 104 | 105 | 114 | 113 | 113 |   |   |    |
|      |       | 1 | 101 | 105 | 106 | 115 | 114 | 114 |   |   |    |
|      |       | 1 | 102 | 106 | 107 | 116 | 115 | 115 |   |   |    |
|      |       | 1 | 103 | 107 | 108 | 117 | 116 | 116 |   |   |    |
|      |       | 1 | 104 | 108 | 109 | 118 | 117 | 117 |   |   |    |
|      |       | 1 | 105 | 109 | 110 | 119 | 118 | 118 |   |   |    |
|      |       | 1 | 106 | 110 | 111 | 120 | 119 | 119 |   |   |    |
|      |       | 1 | 107 | 111 | 112 | 121 | 120 | 120 |   |   |    |
|      |       | 1 | 108 | 112 | 113 | 122 | 121 | 121 |   |   |    |
|      |       | 1 | 109 | 113 | 114 | 123 | 122 | 122 |   |   |    |
|      |       | 1 | 110 | 114 | 115 | 124 | 123 | 123 |   |   |    |
|      |       | 1 | 111 | 115 | 116 | 125 | 124 | 124 |   |   |    |
|      |       | 1 | 112 | 116 | 117 | 126 | 125 | 125 |   |   |    |
|      |       | 1 | 113 | 117 | 118 | 127 | 126 | 126 |   |   |    |
|      |       | 1 | 114 | 118 | 119 | 128 | 127 | 127 |   |   |    |
|      |       | 1 | 115 | 119 | 120 | 129 | 128 | 128 |   |   |    |
|      |       | 1 | 116 | 120 | 121 | 130 | 129 | 129 |   |   |    |
|      |       | 1 | 117 | 121 | 122 | 131 | 130 | 130 |   |   |    |
|      |       | 1 | 118 | 122 | 123 | 132 | 131 | 131 |   |   |    |
|      |       | 1 | 119 | 123 | 124 | 133 | 132 | 132 |   |   |    |
|      |       | 1 | 120 | 124 | 125 | 134 | 133 | 133 |   |   |    |
|      |       | 1 | 121 | 125 | 126 | 135 | 134 | 134 |   |   |    |
|      |       | 1 | 122 | 126 | 127 | 136 | 135 | 135 |   |   |    |
|      |       | 1 | 123 | 127 | 128 | 137 | 136 | 136 |   |   |    |
|      |       | 1 | 124 | 128 | 129 | 138 | 137 | 137 |   |   |    |
|      |       | 1 | 125 | 129 | 130 | 139 | 138 | 138 |   |   |    |
|      |       | 1 | 126 | 130 | 131 | 140 | 139 | 139 |   |   |    |
|      |       | 1 | 127 | 131 | 132 | 141 | 140 | 140 |   |   |    |
|      |       | 1 | 128 | 132 | 133 | 142 | 141 | 141 |   |   |    |
|      |       | 1 | 129 | 133 | 134 | 143 | 142 | 142 |   |   |    |
|      |       | 1 | 130 | 134 | 135 | 144 | 143 | 143 |   |   |    |
|      |       | 1 | 131 | 135 | 136 | 145 | 144 | 144 |   |   |    |
|      |       | 1 | 132 | 136 | 137 | 146 | 145 | 145 |   |   |    |
|      |       | 1 | 133 | 137 | 138 | 147 | 146 | 146 |   |   |    |
|      |       | 1 | 134 | 138 | 139 | 148 | 147 | 147 |   |   |    |
|      |       | 1 | 135 | 139 | 140 | 149 | 148 | 148 |   |   |    |
|      |       | 1 | 136 | 140 | 141 | 150 | 149 | 149 |   |   |    |
|      |       | 1 | 137 | 141 | 142 | 151 | 150 | 150 |   |   |    |
|      |       | 1 | 138 | 142 | 143 | 152 | 151 | 151 |   |   |    |
|      |       | 1 | 139 | 143 | 144 | 153 | 152 | 152 |   |   |    |
|      |       | 1 | 140 | 144 | 145 | 154 | 153 | 153 |   |   |    |
|      |       | 1 | 141 | 145 | 146 | 155 | 154 | 154 |   |   |    |
|      |       | 1 | 142 | 146 | 147 | 156 | 155 | 155 |   |   |    |
|      |       | 1 | 143 | 147 | 148 | 157 | 156 | 156 |   |   |    |
|      |       | 1 | 144 | 148 | 149 | 158 | 157 | 157 |   |   |    |
|      |       | 1 | 145 | 149 | 150 | 159 | 158 | 158 |   |   |    |
|      |       | 1 | 146 | 150 | 151 | 160 | 159 | 159 |   |   |    |
|      |       | 1 | 147 | 151 | 152 | 161 | 160 | 160 |   |   |    |
|      |       | 1 | 148 | 152 | 153 | 162 | 161 | 161 |   |   |    |
|      |       | 1 | 149 | 153 | 154 | 163 | 162 | 162 |   |   |    |
|      |       | 1 | 150 | 154 | 155 | 164 | 163 | 163 |   |   |    |
|      |       | 1 | 151 | 155 | 156 | 165 | 164 | 164 |   |   |    |
|      |       | 1 | 152 | 156 | 157 | 166 | 165 | 165 |   |   |    |
|      |       | 1 | 153 | 157 | 158 | 167 | 166 | 166 |   |   |    |
|      |       | 1 | 154 | 158 | 159 | 168 | 167 | 167 |   |   |    |
|      |       | 1 | 155 | 159 | 160 | 169 | 168 | 168 |   |   |    |
|      |       | 1 | 156 | 160 | 161 | 170 | 169 | 169 |   |   |    |
|      |       | 1 | 157 | 161 | 162 | 171 | 170 | 170 |   |   |    |
|      |       | 1 | 158 | 162 | 163 | 172 | 171 | 171 |   |   |    |
|      |       | 1 | 159 | 163 | 164 | 173 | 172 | 172 |   |   |    |
|      |       | 1 | 160 | 164 | 165 | 174 | 173 | 173 |   |   |    |
|      |       | 1 | 161 | 165 | 166 | 175 | 174 | 174 |   |   |    |
|      |       | 1 | 162 | 166 | 167 | 176 | 175 | 175 |   |   |    |
|      |       | 1 | 163 | 167 | 168 | 177 | 176 | 176 |   |   |    |
|      |       | 1 | 164 | 168 | 169 | 178 | 177 | 177 |   |   |    |
|      |       | 1 | 165 | 169 | 170 | 179 | 178 | 178 |   |   |    |
|      |       | 1 | 166 | 170 | 171 | 180 | 179 | 179 |   |   |    |
|      |       | 1 | 167 | 171 | 172 | 181 | 180 | 180 |   |   |    |
|      |       | 1 | 168 | 172 | 173 | 182 | 181 | 181 |   |   |    |
|      |       | 1 | 169 | 173 | 174 | 183 | 182 | 182 |   |   |    |
|      |       | 1 | 170 | 174 | 175 | 184 | 183 | 183 |   |   |    |
|      |       | 1 | 171 | 175 | 176 | 185 | 184 | 184 |   |   |    |
|      |       | 1 | 172 | 176 | 177 | 186 | 185 | 185 |   |   |    |
|      |       | 1 | 173 | 177 | 178 | 187 | 186 | 186 |   |   |    |
|      |       | 1 | 174 | 178 | 179 | 188 | 187 | 187 |   |   |    |
|      |       | 1 | 175 | 179 | 180 | 189 | 188 | 188 |   |   |    |
|      |       | 1 | 176 | 180 | 181 | 190 | 189 | 189 |   |   |    |
|      |       | 1 | 177 | 181 | 182 | 191 | 190 | 190 |   |   |    |
|      |       | 1 | 178 | 182 | 183 | 192 | 191 | 191 |   |   |    |
|      |       | 1 | 179 | 183 | 184 | 193 | 192 | 192 |   |   |    |
|      |       | 1 | 180 | 184 | 185 | 194 | 193 | 193 |   |   |    |
|      |       | 1 | 181 | 185 | 186 | 195 | 194 | 194 |   |   |    |
|      |       | 1 | 182 | 186 | 187 | 196 | 195 | 195 |   |   |    |
|      |       | 1 | 183 | 187 | 188 | 197 | 196 | 196 |   |   |    |
|      |       | 1 | 184 | 188 | 189 | 198 | 197 | 197 |   |   |    |
|      |       | 1 | 185 | 189 | 190 | 199 | 198 | 198 |   |   |    |
|      |       | 1 | 186 | 190 | 191 | 200 | 199 | 199 |   |   |    |

| CARD<br>COUNT | 1    | 2  | 3     | 4     | 5     | 6     | 7     | 8     | 9     | 10    |
|---------------|------|----|-------|-------|-------|-------|-------|-------|-------|-------|
| 69-           | CARD | 1  | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 |
| 70-           | CARD | 2  | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 |
| 71-           | CARD | 3  | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 |
| 72-           | CARD | 4  | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 |
| 73-           | CARD | 5  | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 |
| 74-           | CARD | 6  | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 |
| 75-           | CARD | 7  | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 |
| 76-           | CARD | 8  | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 |
| 77-           | CARD | 9  | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 |
| 78-           | CARD | 10 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 |
| 79-           | CARD | 11 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 |
| 80-           | CARD | 12 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 |
| 81-           | CARD | 13 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 |
| 82-           | CARD | 14 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 |
| 83-           | CARD | 15 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 |
| 84-           | CARD | 16 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 |
| 85-           | CARD | 17 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 |
| 86-           | CARD | 18 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 |
| 87-           | CARD | 19 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 |
| 88-           | CARD | 20 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 |
| 89-           | CARD | 21 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 |
| 90-           | CARD | 22 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 |
| 91-           | CARD | 23 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 |
| 92-           | CARD | 24 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 |
| 93-           | CARD | 25 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 |
| 94-           | CARD | 26 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 |
| 95-           | CARD | 27 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 |
| 96-           | CARD | 28 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 |
| 97-           | CARD | 29 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 |
| 98-           | CARD | 30 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 |
| 99-           | CARD | 31 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 |
| 100-          | CARD | 32 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 |
| 101-          | CARD | 33 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 |
| 102-          | CARD | 34 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 | 10.00 |



1 SPHERICAL SHELL - N=8

| CARD<br>COUNT | 1       | 2  | 3   | 4     | 5     | 6     | 7  | 8   | 9   | 10   |
|---------------|---------|----|-----|-------|-------|-------|----|-----|-----|------|
| 137-          | GRID    | 69 | 1   | 10.00 | 46.00 | 78.75 | 1  | 1.0 | 0.0 |      |
| 138-          | GRID    | 70 | 1   | 10.00 | 36.00 | 78.75 | 1  | 1.0 | 0.0 |      |
| 139-          | GRID    | 71 | 1   | 10.00 | 27.00 | 78.75 | 1  | 1.0 | 0.0 |      |
| 140-          | GRID    | 72 | 1   | 10.00 | 18.00 | 78.75 | 1  | 1.0 | 0.0 |      |
| 141-          | GRID    | 73 | 1   | 10.00 | 9.00  | 78.75 | 1  | 1.0 | 0.0 |      |
| 142-          | GRID    | 74 | 1   | 10.00 | 0.00  | 78.75 | 1  | 1.0 | 0.0 |      |
| 143-          | GRID    | 75 | 1   | 10.00 | 81.00 | 78.75 | 1  | 1.0 | 0.0 |      |
| 144-          | GRID    | 76 | 1   | 10.00 | 72.00 | 78.75 | 1  | 1.0 | 0.0 |      |
| 145-          | GRID    | 77 | 1   | 10.00 | 63.00 | 78.75 | 1  | 1.0 | 0.0 |      |
| 146-          | GRID    | 78 | 1   | 10.00 | 54.00 | 78.75 | 1  | 1.0 | 0.0 |      |
| 147-          | GRID    | 79 | 1   | 10.00 | 45.00 | 78.75 | 1  | 1.0 | 0.0 |      |
| 148-          | GRID    | 80 | 1   | 10.00 | 36.00 | 78.75 | 1  | 1.0 | 0.0 |      |
| 149-          | GRID    | 81 | 1   | 10.00 | 27.00 | 78.75 | 1  | 1.0 | 0.0 |      |
| 150-          | GRID    | 81 | 1   | 10.00 | 18.00 | 78.75 | 1  | 1.0 | 0.0 |      |
| 151-          | PSHELL  | 1  | 1   | 0.04  | 1     | 1.0   | 1  | 1.0 | 0.0 |      |
| 152-          | SPC1    | 5  | 3   | 9     | 74    | 75    | 76 | 77  | 78  | +XYZ |
| 153-          | SPC1    | 5  | 156 | 73    | 8     | 3     | 4  | 5   | 6   | +BCD |
| 154-          | +XYZ    | 79 | 89  | 81    | 2     | 3     | 4  | 5   | 6   | +BCD |
| 155-          | SPC1    | 5  | 246 | 1     | 8     | 3     | 4  | 5   | 6   | +BCD |
| 156-          | +BCD    | 7  | 8   | 9     | 8     | 3     | 4  | 5   | 6   | +BCD |
|               | ENDDATA |    |     |       |       |       |    |     |     |      |

MAY 26, 1968

RELEASE 1987 CDC

PAGE 9

SUBCASE 1

## DISPLACEMENT VECTOR

| POINT ID. | TYPE | T1           | T2            | T3            | R1            | R2            | R3            |
|-----------|------|--------------|---------------|---------------|---------------|---------------|---------------|
| 1         | G    | 9.544737E-02 | .0            | 4.349715E-02  | .0            | -1.905434E-02 | .0            |
| 2         | G    | 6.788819E-02 | .0            | 4.13084E-02   | .0            | -1.563841E-02 | .0            |
| 3         | G    | 4.605500E-02 | .0            | 3.630303E-02  | .0            | -1.284450E-02 | .0            |
| 4         | G    | 3.121333E-02 | .0            | 2.979180E-02  | .0            | -9.686670E-03 | .0            |
| 5         | G    | 1.960743E-02 | .0            | 2.264792E-02  | .0            | -7.789540E-03 | .0            |
| 6         | G    | 1.137281E-02 | .0            | 1.643621E-02  | .0            | -6.100912E-03 | .0            |
| 7         | G    | 5.802211E-03 | .0            | 8.218661E-03  | .0            | -4.663406E-03 | .0            |
| 8         | G    | 2.618190E-03 | .0            | 3.875984E-03  | .0            | -3.311706E-03 | .0            |
| 9         | G    | 1.012640E-03 | .0            | .0            | .0            | -2.302208E-03 | .0            |
| 10        | G    | 8.770363E-02 | -7.725300E-04 | 3.949185E-02  | 1.878509E-05  | -1.662371E-02 | 7.478230E-03  |
| 11        | G    | 6.319187E-02 | -4.562560E-04 | 3.760400E-02  | -1.690150E-03 | -1.448311E-02 | 4.415313E-03  |
| 12        | G    | 4.398692E-02 | -8.955313E-04 | 3.340055E-02  | -1.553613E-03 | -1.144960E-02 | 2.913430E-03  |
| 13        | G    | 2.835179E-02 | -1.843323E-04 | 2.715280E-02  | -1.868340E-03 | -9.284242E-03 | 2.036600E-03  |
| 14        | G    | 1.845969E-02 | -1.136186E-04 | 2.001461E-02  | -1.216400E-03 | -7.483871E-03 | 1.328190E-03  |
| 15        | G    | 1.071653E-02 | -6.453034E-05 | 1.413663E-02  | -9.106040E-04 | -6.989461E-03 | 9.338001E-04  |
| 16        | G    | 5.550755E-03 | -3.358406E-05 | 8.216670E-03  | -8.663933E-04 | -4.543630E-03 | 5.441530E-04  |
| 17        | G    | 2.468774E-03 | -1.413908E-05 | 3.283310E-03  | -6.839354E-04 | -3.248961E-03 | 3.800113E-04  |
| 18        | G    | 9.551497E-04 | -6.387433E-06 | -2.968631E-04 | -8.317373E-04 | -8.336243E-03 | -5.332016E-04 |
| 19        | G    | 7.068362E-02 | -5.943434E-03 | 2.879180E-02  | -1.672673E-03 | -1.278260E-02 | 1.029005E-02  |
| 20        | G    | 5.169261E-02 | -3.937679E-03 | 2.747165E-02  | -2.072313E-03 | -1.161336E-02 | 7.573821E-03  |
| 21        | G    | 3.631221E-02 | -8.626021E-03 | 2.410071E-02  | -2.594324E-03 | -9.836334E-03 | 5.334102E-03  |
| 22        | G    | 2.435236E-02 | -1.700279E-03 | 1.976111E-02  | -2.317133E-03 | -9.16242E-03  | 3.846202E-03  |
| 23        | G    | 1.536196E-02 | -1.053460E-03 | 1.481305E-02  | -2.302801E-03 | -6.787670E-03 | 2.553011E-03  |
| 24        | G    | 8.932310E-03 | -6.057056E-04 | 9.88145E-03   | -1.947311E-03 | -5.443095E-03 | 1.753932E-03  |
| 25        | G    | 4.634611E-03 | -3.131132E-04 | 5.368509E-03  | -1.692643E-03 | -4.264298E-03 | 1.084766E-03  |
| 26        | G    | 2.060113E-03 | -1.376020E-04 | 1.596790E-03  | -1.297075E-03 | -3.067222E-03 | 7.199090E-04  |
| 27        | G    | 7.977308E-04 | -5.477713E-05 | -1.142203E-03 | -1.541375E-03 | -2.401565E-03 | -9.709962E-04 |
| 28        | G    | 5.104025E-02 | -1.644782E-02 | 1.360612E-02  | -3.706443E-03 | -9.454210E-03 | 1.818583E-02  |
| 29        | G    | 3.735272E-02 | -1.159913E-02 | 1.252242E-02  | -4.684040E-03 | -8.947507E-03 | 8.900656E-03  |
| 30        | G    | 2.629264E-02 | -7.982790E-03 | 1.119666E-02  | -4.194769E-03 | -7.969223E-03 | 6.595640E-03  |
| 31        | G    | 1.765761E-02 | -5.278470E-03 | 8.840945E-03  | -3.713324E-03 | -6.766233E-03 | 4.855640E-03  |
| 32        | G    | 1.115223E-02 | -3.303756E-03 | 6.202541E-03  | -3.485825E-03 | -5.800227E-03 | 3.305100E-03  |
| 33        | G    | 6.488834E-03 | -1.911786E-03 | 3.561300E-03  | -2.984866E-03 | -4.730257E-03 | 2.268400E-03  |
| 34        | G    | 3.368570E-03 | -9.900306E-04 | 1.113423E-03  | -2.472434E-03 | -3.792770E-03 | 1.343077E-03  |

## SUBCASE 1

## DISPLACEMENT VECTOR

| POINT ID. | TYPE | T1           | T2            | T3            | R1            | R2            | R3            |
|-----------|------|--------------|---------------|---------------|---------------|---------------|---------------|
| 36        | G    | 1.497048E-03 | -4.382182E-04 | -9.254670E-04 | -1.874700E-03 | -2.771883E-03 | 9.367254E-04  |
| 37        | G    | 5.866340E-04 | -1.713210E-04 | -2.407270E-03 | -2.407270E-03 | -2.439504E-03 | -1.258584E-03 |
| 38        | G    | 3.204137E-03 | -3.204137E-02 | -3.899490E-03 | -6.398147E-03 | -6.398147E-03 | 1.257742E-02  |
| 39        | G    | 2.389766E-02 | -2.329766E-02 | -3.899490E-03 | -6.197170E-03 | -6.197170E-03 | 9.628137E-03  |
| 40        | G    | 1.623622E-02 | -1.623622E-02 | -3.899490E-03 | -6.068810E-03 | -6.068810E-03 | 7.041978E-03  |
| 41        | G    | 1.086837E-02 | -1.086837E-02 | -3.899490E-03 | -5.285423E-03 | -5.285423E-03 | 5.188747E-03  |
| 42        | G    | 6.841495E-03 | -6.841495E-03 | -3.899490E-03 | -4.851408E-03 | -4.851408E-03 | 3.568677E-03  |
| 43        | G    | 3.974918E-03 | -3.974918E-03 | -3.899490E-03 | -3.887494E-03 | -3.887494E-03 | 2.446870E-03  |
| 44        | G    | 2.068214E-03 | -2.068214E-03 | -3.899490E-03 | -3.186160E-03 | -3.186160E-03 | 1.469618E-03  |
| 45        | G    | 9.154102E-04 | -9.154102E-04 | -3.899490E-03 | -2.370913E-03 | -2.370913E-03 | 1.014390E-03  |
| 46        | G    | 3.558834E-04 | -3.558834E-04 | -3.899490E-03 | -8.336308E-03 | -8.336308E-03 | -1.347534E-03 |
| 47        | G    | 1.159913E-04 | -1.159913E-04 | -3.899490E-03 | -9.464210E-03 | -9.464210E-03 | 1.818083E-03  |
| 48        | G    | 7.982709E-05 | -7.982709E-05 | -3.899490E-03 | -8.947607E-03 | -8.947607E-03 | 8.990655E-03  |
| 49        | G    | 5.278479E-05 | -5.278479E-05 | -3.899490E-03 | -7.958883E-03 | -7.958883E-03 | 8.595440E-03  |
| 50        | G    | 3.303766E-05 | -3.303766E-05 | -3.899490E-03 | -6.764233E-03 | -6.764233E-03 | 4.855542E-03  |
| 51        | G    | 1.911706E-05 | -1.911706E-05 | -3.899490E-03 | -5.860827E-03 | -5.860827E-03 | 3.395100E-03  |
| 52        | G    | 9.900366E-06 | -9.900366E-06 | -3.899490E-03 | -4.730257E-03 | -4.730257E-03 | 2.268499E-03  |
| 53        | G    | 4.382122E-06 | -4.382122E-06 | -3.899490E-03 | -3.792770E-03 | -3.792770E-03 | 1.343873E-03  |
| 54        | G    | 1.713210E-06 | -1.713210E-06 | -3.899490E-03 | -2.771883E-03 | -2.771883E-03 | 9.367254E-04  |
| 55        | G    | 5.943434E-07 | -5.943434E-07 | -3.899490E-03 | -2.439504E-03 | -2.439504E-03 | -1.258584E-03 |
| 56        | G    | 3.937670E-07 | -3.937670E-07 | -3.899490E-03 | -1.874700E-03 | -1.874700E-03 | 1.029806E-02  |
| 57        | G    | 2.626021E-07 | -2.626021E-07 | -3.899490E-03 | -1.613364E-02 | -1.613364E-02 | 7.573821E-03  |
| 58        | G    | 1.700270E-07 | -1.700270E-07 | -3.899490E-03 | -9.835334E-03 | -9.835334E-03 | 5.334102E-03  |
| 59        | G    | 1.053468E-07 | -1.053468E-07 | -3.899490E-03 | -8.162462E-03 | -8.162462E-03 | 3.845292E-03  |
| 60        | G    | 6.057056E-08 | -6.057056E-08 | -3.899490E-03 | -6.787676E-03 | -6.787676E-03 | 2.553011E-03  |
| 61        | G    | 3.131132E-08 | -3.131132E-08 | -3.899490E-03 | -5.443095E-03 | -5.443095E-03 | 1.753932E-03  |
| 62        | G    | 1.376809E-08 | -1.376809E-08 | -3.899490E-03 | -4.264295E-03 | -4.264295E-03 | 1.024766E-03  |
| 63        | G    | 5.477712E-09 | -5.477712E-09 | -3.899490E-03 | -3.067222E-03 | -3.067222E-03 | 7.199090E-04  |
| 64        | G    | 7.726308E-09 | -7.726308E-09 | -3.899490E-03 | -2.401555E-03 | -2.401555E-03 | -9.769962E-04 |
| 65        | G    | 4.552560E-09 | -4.552560E-09 | -3.899490E-03 | -1.662371E-02 | -1.662371E-02 | 7.472394E-03  |
| 66        | G    | 2.955313E-09 | -2.955313E-09 | -3.899490E-03 | -1.448311E-02 | -1.448311E-02 | 4.415313E-03  |
| 67        | G    | 1.843323E-09 | -1.843323E-09 | -3.899490E-03 | -1.144960E-02 | -1.144960E-02 | 2.913439E-03  |
| 68        | G    | 1.135186E-09 | -1.135186E-09 | -3.899490E-03 | -9.284242E-03 | -9.284242E-03 | 2.638098E-03  |
| 69        | G    | 1.135186E-09 | -1.135186E-09 | -3.899490E-03 | -7.483871E-03 | -7.483871E-03 | 1.338100E-03  |

SUBCASE 1

| POINT ID. | TYPE | DISPLACEMENT |               |               |               |               |               | VECTOR |  |  |
|-----------|------|--------------|---------------|---------------|---------------|---------------|---------------|--------|--|--|
|           |      | T1           | T2            | T3            | R1            | R2            | R3            |        |  |  |
| 69        | Q    | 6.453034E-05 | -1.071653E-02 | -2.193663E-02 | -5.920461E-03 | -9.990048E-04 | 9.338091E-04  |        |  |  |
| 70        | Q    | 3.358406E-06 | -5.555075E-03 | -1.601587E-02 | -4.563686E-03 | -8.648938E-04 | 5.441538E-04  |        |  |  |
| 71        | Q    | 1.413006E-06 | -8.468774E-03 | -1.108231E-02 | -3.848951E-03 | -8.630364E-04 | 3.890113E-04  |        |  |  |
| 72        | Q    | 6.387433E-06 | -9.561407E-04 | -7.502144E-03 | -8.336243E-03 | -8.317373E-04 | -5.338016E-04 |        |  |  |
| 73        | Q    | .0           | -9.544737E-02 | -5.120615E-02 | -1.065434E-02 | .0            | .0            |        |  |  |
| 74        | Q    | .0           | -4.788819E-02 | -4.918783E-02 | -1.563841E-02 | .0            | .0            |        |  |  |
| 75        | Q    | .0           | -4.695506E-02 | -4.410093E-02 | -1.894450E-02 | .0            | .0            |        |  |  |
| 76        | Q    | .0           | -3.183338E-02 | -3.750028E-02 | -9.806870E-03 | .0            | .0            |        |  |  |
| 77        | Q    | .0           | -1.960743E-02 | -3.046639E-02 | -7.780540E-03 | .0            | .0            |        |  |  |
| 78        | Q    | .0           | -1.137281E-02 | -2.343421E-02 | -6.100012E-03 | .0            | .0            |        |  |  |
| 79        | Q    | .0           | -5.892211E-03 | -2.343421E-02 | -4.643480E-03 | .0            | .0            |        |  |  |
| 80        | Q    | .0           | -2.618190E-03 | -1.167406E-02 | -3.311705E-03 | .0            | .0            |        |  |  |
| 81        | Q    | .0           | -1.012620E-03 | -7.790007E-03 | -2.308200E-03 | .0            | .0            |        |  |  |

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SUBJECT :

LOAD VECTORS

R3  
.0  
.0

R2  
.0  
.0

R1  
.0  
.0

T3  
.0  
.0

T2  
.0  
-1.000000000

T1  
.0  
-1.000000000

T4  
.0  
-1.000000000

T5  
.0  
-1.000000000

蘇聯文學

**A**

[illegible]







SUBCASE 1

STRESSSES IN GENERAL QUADRILATERAL ELEMENTS (IN STRESS COORDINATE SYSTEM)

| ELEMENT | I | J | K | L | STRESSSES     |               |               |              | IN STRESS COORD. SYSTEM |              |               |              | PRINCIPAL STRESSSES (ZERO SHEAR) |              |               |              | MAX           |              |               |              |
|---------|---|---|---|---|---------------|---------------|---------------|--------------|-------------------------|--------------|---------------|--------------|----------------------------------|--------------|---------------|--------------|---------------|--------------|---------------|--------------|
|         |   |   |   |   | MINOR         | MAJOR         | MINOR         | MAJOR        | MINOR                   | MAJOR        | MINOR         | MAJOR        | MINOR                            | MAJOR        | MINOR         | MAJOR        | MINOR         | MAJOR        | MINOR         | MAJOR        |
| 1       | 1 | 2 | 3 | 4 | 1.123152E+02  | -2.255471E+02 | -1.184533E+03 | 1.184533E+03 | -41.12312               | 1.700953E+03 | -1.712314E+03 | 1.700953E+03 | -1.712314E+03                    | 1.700953E+03 | -1.712314E+03 | 1.700953E+03 | -1.712314E+03 | 1.700953E+03 | -1.712314E+03 | 1.700953E+03 |
| 2       | 2 | 3 | 4 | 1 | -2.255471E+02 | 1.184533E+03  | -1.184533E+03 | 1.184533E+03 | 45.4464                 | 1.712314E+03 | -1.700953E+03 | 1.712314E+03 | -1.700953E+03                    | 1.712314E+03 | -1.700953E+03 | 1.712314E+03 | -1.700953E+03 | 1.712314E+03 | -1.700953E+03 | 1.712314E+03 |
| 3       | 3 | 4 | 1 | 2 | 1.123152E+02  | -2.255471E+02 | -1.184533E+03 | 1.184533E+03 | -43.1148                | 1.457335E+03 | -1.414987E+03 | 1.457335E+03 | -1.414987E+03                    | 1.457335E+03 | -1.414987E+03 | 1.457335E+03 | -1.414987E+03 | 1.457335E+03 | -1.414987E+03 | 1.457335E+03 |
| 4       | 4 | 1 | 2 | 3 | -2.255471E+02 | 1.184533E+03  | -1.184533E+03 | 1.184533E+03 | 47.5385                 | 1.426377E+03 | -1.414987E+03 | 1.426377E+03 | -1.414987E+03                    | 1.426377E+03 | -1.414987E+03 | 1.426377E+03 | -1.414987E+03 | 1.426377E+03 | -1.414987E+03 | 1.426377E+03 |
| 5       | 1 | 2 | 3 | 4 | 1.123152E+02  | -2.255471E+02 | -1.184533E+03 | 1.184533E+03 | -42.1332                | 1.313047E+03 | -1.313047E+03 | 1.313047E+03 | -1.313047E+03                    | 1.313047E+03 | -1.313047E+03 | 1.313047E+03 | -1.313047E+03 | 1.313047E+03 | -1.313047E+03 | 1.313047E+03 |
| 6       | 2 | 3 | 4 | 1 | -2.255471E+02 | 1.184533E+03  | -1.184533E+03 | 1.184533E+03 | 48.8618                 | 1.811802E+03 | -1.811802E+03 | 1.811802E+03 | -1.811802E+03                    | 1.811802E+03 | -1.811802E+03 | 1.811802E+03 | -1.811802E+03 | 1.811802E+03 | -1.811802E+03 | 1.811802E+03 |
| 7       | 3 | 4 | 1 | 2 | 1.123152E+02  | -2.255471E+02 | -1.184533E+03 | 1.184533E+03 | -43.6095                | 9.183883E+02 | -9.183883E+02 | 9.183883E+02 | -9.183883E+02                    | 9.183883E+02 | -9.183883E+02 | 9.183883E+02 | -9.183883E+02 | 9.183883E+02 | -9.183883E+02 | 9.183883E+02 |
| 8       | 4 | 1 | 2 | 3 | -2.255471E+02 | 1.184533E+03  | -1.184533E+03 | 1.184533E+03 | 48.4535                 | 9.110483E+02 | -9.110483E+02 | 9.110483E+02 | -9.110483E+02                    | 9.110483E+02 | -9.110483E+02 | 9.110483E+02 | -9.110483E+02 | 9.110483E+02 | -9.110483E+02 | 9.110483E+02 |
| 9       | 1 | 2 | 3 | 4 | 1.123152E+02  | -2.255471E+02 | -1.184533E+03 | 1.184533E+03 | -43.5258                | 6.116212E+02 | -6.116212E+02 | 6.116212E+02 | -6.116212E+02                    | 6.116212E+02 | -6.116212E+02 | 6.116212E+02 | -6.116212E+02 | 6.116212E+02 | -6.116212E+02 | 6.116212E+02 |
| 10      | 2 | 3 | 4 | 1 | -2.255471E+02 | 1.184533E+03  | -1.184533E+03 | 1.184533E+03 | 48.4878                 | 8.117757E+02 | -8.117757E+02 | 8.117757E+02 | -8.117757E+02                    | 8.117757E+02 | -8.117757E+02 | 8.117757E+02 | -8.117757E+02 | 8.117757E+02 | -8.117757E+02 | 8.117757E+02 |
| 11      | 3 | 4 | 1 | 2 | 1.123152E+02  | -2.255471E+02 | -1.184533E+03 | 1.184533E+03 | -46.7264                | 7.938863E+02 | -7.938863E+02 | 7.938863E+02 | -7.938863E+02                    | 7.938863E+02 | -7.938863E+02 | 7.938863E+02 | -7.938863E+02 | 7.938863E+02 | -7.938863E+02 | 7.938863E+02 |
| 12      | 4 | 1 | 2 | 3 | -2.255471E+02 | 1.184533E+03  | -1.184533E+03 | 1.184533E+03 | -35.6532                | 1.824946E+03 | -1.824946E+03 | 1.824946E+03 | -1.824946E+03                    | 1.824946E+03 | -1.824946E+03 | 1.824946E+03 | -1.824946E+03 | 1.824946E+03 | -1.824946E+03 | 1.824946E+03 |
| 13      | 1 | 2 | 3 | 4 | 1.123152E+02  | -2.255471E+02 | -1.184533E+03 | 1.184533E+03 | -31.7839                | 2.322152E+03 | -2.322152E+03 | 2.322152E+03 | -2.322152E+03                    | 2.322152E+03 | -2.322152E+03 | 2.322152E+03 | -2.322152E+03 | 2.322152E+03 | -2.322152E+03 | 2.322152E+03 |
| 14      | 2 | 3 | 4 | 1 | -2.255471E+02 | 1.184533E+03  | -1.184533E+03 | 1.184533E+03 | 55.4364                 | 1.835035E+03 | -1.835035E+03 | 1.835035E+03 | -1.835035E+03                    | 1.835035E+03 | -1.835035E+03 | 1.835035E+03 | -1.835035E+03 | 1.835035E+03 | -1.835035E+03 | 1.835035E+03 |
| 15      | 3 | 4 | 1 | 2 | 1.123152E+02  | -2.255471E+02 | -1.184533E+03 | 1.184533E+03 | -32.1716                | 1.856041E+03 | -1.856041E+03 | 1.856041E+03 | -1.856041E+03                    | 1.856041E+03 | -1.856041E+03 | 1.856041E+03 | -1.856041E+03 | 1.856041E+03 | -1.856041E+03 | 1.856041E+03 |
| 16      | 4 | 1 | 2 | 3 | -2.255471E+02 | 1.184533E+03  | -1.184533E+03 | 1.184533E+03 | 57.6047                 | 1.457335E+03 | -1.457335E+03 | 1.457335E+03 | -1.457335E+03                    | 1.457335E+03 | -1.457335E+03 | 1.457335E+03 | -1.457335E+03 | 1.457335E+03 | -1.457335E+03 | 1.457335E+03 |
| 17      | 1 | 2 | 3 | 4 | 1.123152E+02  | -2.255471E+02 | -1.184533E+03 | 1.184533E+03 | -32.1716                | 1.457335E+03 | -1.457335E+03 | 1.457335E+03 | -1.457335E+03                    | 1.457335E+03 | -1.457335E+03 | 1.457335E+03 | -1.457335E+03 | 1.457335E+03 | -1.457335E+03 | 1.457335E+03 |
| 18      | 2 | 3 | 4 | 1 | -2.255471E+02 | 1.184533E+03  | -1.184533E+03 | 1.184533E+03 | 53.8278                 | 1.457335E+03 | -1.457335E+03 | 1.457335E+03 | -1.457335E+03                    | 1.457335E+03 | -1.457335E+03 | 1.457335E+03 | -1.457335E+03 | 1.457335E+03 | -1.457335E+03 | 1.457335E+03 |
| 19      | 3 | 4 | 1 | 2 | 1.123152E+02  | -2.255471E+02 | -1.184533E+03 | 1.184533E+03 | -31.8278                | 1.313047E+03 | -1.313047E+03 | 1.313047E+03 | -1.313047E+03                    | 1.313047E+03 | -1.313047E+03 | 1.313047E+03 | -1.313047E+03 | 1.313047E+03 | -1.313047E+03 | 1.313047E+03 |
| 20      | 4 | 1 | 2 | 3 | -2.255471E+02 | 1.184533E+03  | -1.184533E+03 | 1.184533E+03 | 57.19931                | 1.313047E+03 | -1.313047E+03 | 1.313047E+03 | -1.313047E+03                    | 1.313047E+03 | -1.313047E+03 | 1.313047E+03 | -1.313047E+03 | 1.313047E+03 | -1.313047E+03 | 1.313047E+03 |

SUBCASE 1

## STRESSES IN GENERAL QUADRILATERAL ELEMENTS (COUAD 4)

| ELEMENT ID. | FIBRE DISTANCE | STRESSES IN STRESS COORD. SYSTEM |               |               |          | PRINCIPAL STRESSES (ZERO SHEAR) |               |              | MAX SHEAR    |
|-------------|----------------|----------------------------------|---------------|---------------|----------|---------------------------------|---------------|--------------|--------------|
|             |                | NORMAL-X                         | NORMAL-Y      | SHEAR-XY      | ANGLE    | MAJOR                           | MINOR         |              |              |
| 46          | -2.00000E-02   | 4.451425E+02                     | -4.766131E+02 | -9.51430E+02  | -38.0771 | 1.041447E+03                    | -1.072017E+03 | 1.067183E+03 | 1.067183E+03 |
| 46          | -2.00000E-02   | -4.562843E+02                    | -4.803330E+02 | 9.546343E+02  | 57.9810  | 1.077021E+03                    | -1.047878E+03 | 1.062897E+03 | 1.062897E+03 |
| 46          | -2.00000E-02   | 3.823891E+02                     | -4.183047E+02 | -8.421855E+02 | -38.2264 | 9.145178E+02                    | -9.566149E+02 | 9.325163E+02 | 9.325163E+02 |
| 47          | -2.00000E-02   | -3.867201E+02                    | 4.272070E+02  | 8.455401E+02  | 57.8522  | 9.588018E+02                    | -8.191828E+02 | 8.450883E+02 | 8.450883E+02 |
| 47          | -2.00000E-02   | 3.700993E+02                     | -4.608043E+02 | -7.829104E+02 | -31.4318 | 8.300907E+02                    | -8.000549E+02 | 8.149081E+02 | 8.149081E+02 |
| 48          | -2.00000E-02   | -3.801053E+02                    | 3.890894E+02  | 7.477719E+02  | 58.0080  | 8.483047E+02                    | -8.384037E+02 | 8.400852E+02 | 8.400852E+02 |
| 48          | -2.00000E-02   | 1.974444E+02                     | -4.830641E+02 | -6.895400E+02 | -59.0831 | 5.376524E+02                    | -8.241818E+02 | 6.800878E+02 | 6.800878E+02 |
| 49          | -2.00000E-02   | 1.097107E+02                     | 4.935783E+02  | 8.111145E+02  | 59.6215  | 8.510043E+02                    | -5.409400E+02 | 7.003730E+02 | 7.003730E+02 |
| 49          | -2.00000E-02   | -1.097623E+03                    | -1.685951E+03 | -1.988055E+03 | -87.0038 | 2.694401E+03                    | -2.878723E+03 | 2.378562E+03 | 2.378562E+03 |
| 50          | -2.00000E-02   | 1.547234E+03                     | 1.753674E+03  | 1.897814E+03  | 63.4949  | 2.690917E+03                    | -8.063334E+03 | 2.378127E+03 | 2.378127E+03 |
| 50          | -2.00000E-02   | -1.552481E+03                    | -1.468749E+03 | -1.353611E+03 | -80.9560 | 2.045044E+03                    | -1.087158E+03 | 2.034401E+03 | 2.034401E+03 |
| 51          | -2.00000E-02   | 1.059350E+03                     | -1.204200E+03 | 1.381995E+03  | 68.1940  | 1.991707E+03                    | -2.106407E+03 | 2.003557E+03 | 2.003557E+03 |
| 51          | -2.00000E-02   | -1.061780E+03                    | 1.866785E+03  | -1.080660E+03 | -21.1368 | 1.457002E+03                    | -1.601930E+03 | 1.589507E+03 | 1.589507E+03 |
| 52          | -2.00000E-02   | 9.084793E+02                     | -1.006444E+03 | -8.436906E+02 | 69.2554  | 1.656780E+03                    | -1.461785E+03 | 1.554267E+03 | 1.554267E+03 |
| 52          | -2.00000E-02   | -9.080865E+02                    | 9.938111E+02  | 8.457856E+02  | -70.6888 | 1.227162E+03                    | -1.385177E+03 | 1.278170E+03 | 1.278170E+03 |
| 53          | -2.00000E-02   | 7.600919E+02                     | -8.444304E+02 | -7.185388E+02 | 69.1877  | 1.315302E+03                    | -1.231300E+03 | 1.273301E+03 | 1.273301E+03 |
| 53          | -2.00000E-02   | -7.723681E+02                    | 8.533844E+02  | 7.192034E+02  | -20.8457 | 1.042605E+03                    | -1.118033E+03 | 1.003310E+03 | 1.003310E+03 |
| 54          | -2.00000E-02   | 6.769612E+02                     | -7.345895E+02 | -6.360739E+02 | 69.2484  | 1.185875E+03                    | -1.044859E+03 | 1.005367E+03 | 1.005367E+03 |
| 54          | -2.00000E-02   | -6.813014E+02                    | 7.463837E+02  | 6.388007E+02  | -21.0132 | 9.212958E+02                    | -9.789242E+02 | 9.501106E+02 | 9.501106E+02 |
| 55          | -2.00000E-02   | 6.470916E+02                     | -6.995323E+02 | -5.758900E+02 | 69.0877  | 9.904751E+02                    | -9.253928E+02 | 9.570340E+02 | 9.570340E+02 |
| 55          | -2.00000E-02   | -6.600839E+02                    | 6.821069E+02  | 5.717683E+02  | -20.2818 | 8.604937E+02                    | -9.121245E+02 | 8.863091E+02 | 8.863091E+02 |

77

SUBCASE 1

STRESSES IN GENERAL QUADRILATERAL ELEMENTS (COUAD4)  
(IN STRESS COORDINATE SYSTEM)

| ELEMENT<br>ID. | FIBRE<br>DISTANCE | STRESSES IN STRESS COORD. SYSTEM |               |               | ANGLE    | PRINCIPAL STRESSES (ZERO SHEAR) |               | MAX<br>SHEAR |
|----------------|-------------------|----------------------------------|---------------|---------------|----------|---------------------------------|---------------|--------------|
|                |                   | NORMAL-X                         | NORMAL-Y      | SHEAR-XY      |          | MAJOR                           | MINOR         |              |
| 56             | -2.000000E-02     | 3.753140E+02                     | -8.025401E+02 | -4.610400E+02 | -19.6008 | 5.342802E+02                    | -9.615149E+02 | 7.470010E+02 |
| 57             | 2.000000E-02      | -3.681681E+02                    | 8.196762E+02  | 4.700044E+02  | 70.6411  | 9.863400E+02                    | -5.364303E+02 | 7.611434E+02 |
| 57             | -2.000000E-02     | 1.709560E+02                     | -3.725230E+02 | -1.04237E+02  | -11.2230 | 1.527300E+02                    | -3.932077E+02 | 2.729730E+02 |
| 58             | 2.000000E-02      | -1.334050E+02                    | 3.800010E+02  | 1.031884E+02  | 79.9679  | 4.006327E+02                    | -1.533360E+02 | 2.770843E+02 |
| 58             | -2.000000E-02     | 2.184900E+02                     | -2.190430E+02 | -6.586447E+02 | -3.3732  | 2.281843E+02                    | -2.809373E+02 | 2.205500E+02 |
| 59             | 2.000000E-02      | -2.190430E+02                    | 2.044306E+02  | 6.590113E+02  | 3.3717   | 2.144364E+02                    | -2.200630E+02 | 2.221501E+02 |
| 59             | -2.000000E-02     | 1.404372E+02                     | -1.818581E+02 | -4.700030E+02 | -8.5119  | 1.470650E+02                    | -1.606847E+02 | 1.502753E+02 |
| 60             | 2.000000E-02      | -1.414832E+02                    | 1.694553E+02  | 4.659783E+02  | 81.3802  | 1.748743E+02                    | -1.480220E+02 | 1.628782E+02 |
| 60             | -2.000000E-02     | 1.106164E+02                     | -1.896064E+02 | -4.012653E+02 | -8.9278  | 1.258190E+02                    | -1.359080E+02 | 1.300644E+02 |
| 61             | 2.000000E-02      | -1.203920E+02                    | 1.876870E+02  | 4.076502E+02  | 80.8953  | 1.342040E+02                    | -1.288859E+02 | 1.305184E+02 |
| 61             | -2.000000E-02     | 9.893725E+02                     | -1.081381E+02 | -3.520210E+02 | -9.4112  | 1.047860E+02                    | -1.139870E+02 | 1.003874E+02 |
| 62             | 2.000000E-02      | -9.893725E+02                    | 1.094490E+02  | 3.505900E+02  | 89.8840  | 1.153627E+02                    | -1.065810E+02 | 1.104710E+02 |
| 62             | -2.000000E-02     | 8.745513E+02                     | -9.391750E+02 | -3.230974E+02 | -9.8269  | 9.307540E+02                    | -9.382000E+02 | 9.630170E+02 |
| 63             | 2.000000E-02      | -8.814023E+02                    | 9.519237E+02  | 3.276520E+02  | 80.1555  | 1.008722E+02                    | -9.488603E+02 | 9.134700E+02 |
| 63             | -2.000000E-02     | 8.265627E+02                     | -8.933854E+02 | -3.091230E+02 | -9.8565  | 8.00077E+02                     | -8.938606E+02 | 9.101652E+02 |
| 64             | 2.000000E-02      | -8.406327E+02                    | 8.733512E+02  | 3.065300E+02  | 80.1592  | 9.265244E+02                    | -8.104370E+02 | 7.925466E+02 |
| 64             | -2.000000E-02     | 4.976617E+02                     | -9.093533E+02 | -2.614152E+02 | -9.6294  | 5.414140E+02                    | -5.369568E+02 | 8.020963E+02 |
|                |                   | -4.908633E+02                    | 1.021147E+02  | 2.679753E+02  | 80.2413  | 1.067236E+02                    |               |              |

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0604 COMPOSITE TUBE  
SYMMETRIC LAYUP [45/-45/0/90]S  
TUBE UNDER CONSTANT PRESSURE P

CASE CONTROL DECK ECHO

TITLE : 0604 COMPOSITE TUBE  
SUBTITLE : SYMMETRIC LAYUP [45/-45/0/90]S  
LABEL : TUBE UNDER CONSTANT PRESSURE P

MODEL : SECTION OF A OPEN TUBE RADIUS R, UNDER PRESSURE P.  
SYMMETRIC LAYUP [45/-45/0/90/0/-45/45]

1 : COMPARISON OF HOOP LOADING FY FOR ELEMENT ID 8 X 1

HOOP LOADING FY = P \* R \* 10.5 \* 50 = 5.25E5

| UAI/NASTRAN | MSC/NASTRAN | THEORETICAL |
|-------------|-------------|-------------|
| 5.15E2      | 5.15E2      | 5.25E2      |

2 : COMPARISON OF LAYER STRESSES FOR ELEMENT ID 8 X 1

| UAI/NASTRAN |          |         |           |
|-------------|----------|---------|-----------|
|             | SIG1     | SIG2    | TAUIZ     |
| LAYER 1     | 2.524E2  | 1.741E1 | 2.277E1   |
| LAYER 2     | 2.494E2  | 1.751E1 | -2.278E1  |
| LAYER 3     | -2.253E2 | 3.231E1 | 1.944E-2  |
| LAYER 4     | 2.271E2  | 2.652E0 | 1.556E-2  |
| LAYER 5     | 2.270E2  | 2.660E0 | 5.854E-2  |
| LAYER 6     | -2.253E2 | 3.230E1 | -8.551E-2 |
| LAYER 7     | -2.534E2 | 1.741E1 | -2.273E1  |
| LAYER 8     | 2.477E2  | 1.750E1 | 2.278E1   |

MSC/NASTRAN

0604 COMPOSITE TUBE  
SYMMETRIC LAYUP [45/-45/0/90]S  
TUBE UNDER CONSTANT PRESSURE P

CASE CONTROL DECK ECHO

| MSC/NASTRAN |          |         |           |
|-------------|----------|---------|-----------|
|             | SIG1     | SIG2    | TAUIZ     |
| LAYER 1     | 2.519E2  | 1.740E1 | 2.273E1   |
| LAYER 2     | 2.494E2  | 1.748E1 | -2.273E1  |
| LAYER 3     | -2.257E2 | 3.228E1 | 1.188E-2  |
| LAYER 4     | 2.268E2  | 2.651E0 | 2.221E-2  |
| LAYER 5     | 2.269E2  | 2.657E0 | 5.551E-2  |
| LAYER 6     | -2.255E2 | 3.231E1 | -8.881E-2 |
| LAYER 7     | -2.535E2 | 1.741E1 | -2.274E1  |
| LAYER 8     | 2.478E2  | 1.760E1 | 2.274E1   |

DISP : 1 29,45,61.77  
SET : 2 8.24  
DISP (PRINT) : 1

[illegible]

MARCH 23, 1989 RELEASE 1989 CJC 242

THE INFORMATION MESSAGE 2007, BULK DATA NOT SORTED, X509 WILL RE-ORDER DECK.

SI 06/4/54-577 6/16/57 7/1/58

MARCH 23 1980

| 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | 13 | 14 | 15 | 16 | 17 | 18 | 19 | 20 | 21 | 22 | 23 | 24 | 25 | 26 | 27 | 28 | 29 | 30 | 31 | 32 | 33 | 34 | 35 | 36 | 37 | 38 | 39 | 40 | 41 | 42 | 43 | 44 | 45 | 46 | 47 | 48 | 49 | 50 | 51 | 52 | 53 | 54 | 55 | 56 | 57 | 58 | 59 | 60 | 61 | 62 | 63 | 64 | 65 | 66 | 67 | 68 | 69 | 70 | 71 | 72 | 73 | 74 | 75 | 76 | 77 | 78 | 79 | 80 | 81 | 82 | 83 | 84 | 85 | 86 | 87 | 88 | 89 | 90 | 91 | 92 | 93 | 94 | 95 | 96 | 97 | 98 | 99 | 100 | 101 | 102 | 103 | 104 | 105 | 106 | 107 | 108 | 109 | 110 | 111 | 112 | 113 | 114 | 115 | 116 | 117 | 118 | 119 | 120 | 121 | 122 | 123 | 124 | 125 | 126 | 127 | 128 | 129 | 130 | 131 | 132 | 133 | 134 | 135 | 136 | 137 | 138 | 139 | 140 | 141 | 142 | 143 | 144 | 145 | 146 | 147 | 148 | 149 | 150 | 151 | 152 | 153 | 154 | 155 | 156 | 157 | 158 | 159 | 160 | 161 | 162 | 163 | 164 | 165 | 166 | 167 | 168 | 169 | 170 | 171 | 172 | 173 | 174 | 175 | 176 | 177 | 178 | 179 | 180 | 181 | 182 | 183 | 184 | 185 | 186 | 187 | 188 | 189 | 190 | 191 | 192 | 193 | 194 | 195 | 196 | 197 | 198 | 199 | 200 | 201 | 202 | 203 | 204 | 205 | 206 | 207 | 208 | 209 | 210 | 211 | 212 | 213 | 214 | 215 | 216 | 217 | 218 | 219 | 220 | 221 | 222 | 223 | 224 | 225 | 226 | 227 | 228 | 229 | 230 | 231 | 232 | 233 | 234 | 235 | 236 | 237 | 238 | 239 | 240 | 241 | 242 | 243 | 244 | 245 | 246 | 247 | 248 | 249 | 250 | 251 | 252 | 253 | 254 | 255 | 256 | 257 | 258 | 259 | 260 | 261 | 262 | 263 | 264 | 265 | 266 | 267 | 268 | 269 | 270 | 271 | 272 | 273 | 274 | 275 | 276 | 277 | 278 | 279 | 280 | 281 | 282 | 283 | 284 | 285 | 286 | 287 | 288 | 289 | 290 | 291 | 292 | 293 | 294 | 295 | 296 | 297 | 298 | 299 | 300 | 301 | 302 | 303 | 304 | 305 | 306 | 307 | 308 | 309 | 310 | 311 | 312 | 313 | 314 | 315 | 316 | 317 | 318 | 319 | 320 | 321 | 322 | 323 | 324 | 325 | 326 | 327 | 328 | 329 | 330 | 331 | 332 | 333 | 334 | 335 | 336 | 337 | 338 | 339 | 340 | 341 | 342 | 343 | 344 | 345 | 346 | 347 | 348 | 349 | 350 | 351 | 352 | 353 | 354 | 355 | 356 | 357 | 358 | 359 | 360 | 361 | 362 | 363 | 364 | 365 | 366 | 367 | 368 | 369 | 370 | 371 | 372 | 373 | 374 | 375 | 376 | 377 | 378 | 379 | 380 | 381 | 382 | 383 | 384 | 385 | 386 | 387 | 388 | 389 | 390 | 391 | 392 | 393 | 394 | 395 | 396 | 397 | 398 | 399 | 400 | 401 | 402 | 403 | 404 | 405 | 406 | 407 | 408 | 409 | 410 | 411 | 412 | 413 | 414 | 415 | 416 | 417 | 418 | 419 | 420 | 421 | 422 | 423 | 424 | 425 | 426 | 427 | 428 | 429 | 430 | 431 | 432 | 433 | 434 | 435 | 436 | 437 | 438 | 439 | 440 | 441 | 442 | 443 | 444 | 445 | 446 | 447 | 448 | 449 | 450 | 451 | 452 | 453 | 454 | 455 | 456 | 457 | 458 | 459 | 460 | 461 | 462 | 463 | 464 | 465 | 466 | 467 | 468 | 469 | 470 | 471 | 472 | 473 | 474 | 475 | 476 | 477 | 478 | 479 | 480 | 481 | 482 | 483 | 484 | 485 | 486 | 487 | 488 | 489 | 490 | 491 | 492 | 493 | 494 | 495 | 496 | 497 | 498 | 499 | 500 | 501 | 502 | 503 | 504 | 505 | 506 | 507 | 508 | 509 | 510 | 511 | 512 | 513 | 514 | 515 | 516 | 517 | 518 | 519 | 520 | 521 | 522 | 523 | 524 |
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257- GRID 125 1 50.000 45.000 70.000  
 258- GRID 126 1 50.000 337.500 70.000  
 259- GRID 127 1 50.000 22.500 70.000  
 260- GRID 128 1 50.000 0.000 70.000  
 261- GRID 129 1 50.000 180.000 80.000  
 262- GRID 130 1 50.000 202.500 80.000  
 263- GRID 131 1 50.000 157.500 80.000  
 264- GRID 132 1 50.000 225.000 80.000  
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 267- GRID 135 1 50.000 112.500 80.000  
 268- GRID 136 1 50.000 270.000 80.000  
 269- GRID 137 1 50.000 90.000 80.000  
 270- GRID 138 1 50.000 292.500 80.000  
 271- GRID 139 1 50.000 67.500 80.000  
 272- GRID 140 1 50.000 315.000 80.000

1 QUADA COMPOSITE TUBE  
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CARD COUNT  
 273- GRID 141 1 50.000 45.000 80.000  
 274- GRID 142 1 50.000 337.500 80.000  
 275- GRID 143 1 50.000 22.500 80.000  
 276- GRID 144 1 50.000 0.000 80.000  
 277- MAT8 1 73.8 E+33.75 E+30.4 1.74 E+3  
 278- +M01  
 279- PCOMP 2 -0.96 1680. -229.0 20.9  
 280- +PC1 1 .24 45.0 YES  
 281- +PC2 1 0.0 YES  
 282- PLOAD4 1 1 10.5 9 16 24  
 283- SPC1 1 3 1 1 144  
 284- SPC1 1 6 1 1 136  
 285- SPC1 1 145 9 24 137  
 286- SPC1 1 245 1 16 129 144  
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1 QUADA COMPOSITE TUBE  
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 TUBE UNDER CONSTANT PRESSURE P

SUBCASE 1

DISPLACEMENT VECTOR

| POINT ID. | TYPE | T1           | T2           | T3            | R1            | R2            | R3 |
|-----------|------|--------------|--------------|---------------|---------------|---------------|----|
| 29        | G    | 3.467347E-01 | 3.438213E-01 | -3.005484E-02 | -4.671168E-04 | 2.562122E-04  | .0 |
| 45        | G    | 3.470449E-01 | 3.449179E-01 | -6.224011E-02 | -7.687557E-04 | 2.196043E-04  | .0 |
| 61        | G    | 3.470888E-01 | 3.455627E-01 | -9.453406E-02 | 3.007555E-04  | 5.882263E-04  | .0 |
| 77        | G    | 3.485623E-01 | 3.478297E-01 | -1.268109E-01 | -2.582375E-04 | -6.487657E-04 | .0 |

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MARCH 23, 1989 RELEASE 1989 CBC PAGE 16

SUBCASE 1

FORCES IN COVERED COMPOSITE ELEMENTS (QUADR)

ELEMENT

| FORCES IN COVERED COMPOSITE ELEMENTS (QUADR) | FX         | FY         | FZ          | MX          | MY          | MZ          | UX         | UY         | UZ         |
|--|------------|------------|-------------|-------------|-------------|-------------|------------|------------|------------|
| 1  | 2.2585E-01 | 5.1778E-02 | -2.7865E-08 | -8.5564E-01 | -5.0341E-01 | -3.4703E-01 | 2.5886E-01 | 7.9885E-02 | 2.4473E-01 |
| 2  | 1.7831E-01 | 5.1541E-02 | -4.1714E-08 | 1.6764E-01  | 1.1235E-01  | 1.3633E-01  | 7.1068E-01 | 2.4473E-01 | 2.4473E-01 |

- TRANSLATE SHEAR FORCES -  
 UX  
 UY  
 UZ





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RELEASE 1989 CDC PAGE 3

| DATE | TIME  | FROM | TO  | REMARKS |
|------|-------|------|-----|---------|
| 1989 | 03:00 | 100  | 100 | 100     |
| 1989 | 03:01 | 100  | 100 | 100     |
| 1989 | 03:02 | 100  | 100 | 100     |
| 1989 | 03:03 | 100  | 100 | 100     |
| 1989 | 03:04 | 100  | 100 | 100     |
| 1989 | 03:05 | 100  | 100 | 100     |
| 1989 | 03:06 | 100  | 100 | 100     |
| 1989 | 03:07 | 100  | 100 | 100     |
| 1989 | 03:08 | 100  | 100 | 100     |
| 1989 | 03:09 | 100  | 100 | 100     |
| 1989 | 03:10 | 100  | 100 | 100     |
| 1989 | 03:11 | 100  | 100 | 100     |
| 1989 | 03:12 | 100  | 100 | 100     |
| 1989 | 03:13 | 100  | 100 | 100     |
| 1989 | 03:14 | 100  | 100 | 100     |
| 1989 | 03:15 | 100  | 100 | 100     |
| 1989 | 03:16 | 100  | 100 | 100     |
| 1989 | 03:17 | 100  | 100 | 100     |
| 1989 | 03:18 | 100  | 100 | 100     |
| 1989 | 03:19 | 100  | 100 | 100     |
| 1989 | 03:20 | 100  | 100 | 100     |
| 1989 | 03:21 | 100  | 100 | 100     |
| 1989 | 03:22 | 100  | 100 | 100     |
| 1989 | 03:23 | 100  | 100 | 100     |
| 1989 | 03:24 | 100  | 100 | 100     |
| 1989 | 03:25 | 100  | 100 | 100     |
| 1989 | 03:26 | 100  | 100 | 100     |
| 1989 | 03:27 | 100  | 100 | 100     |
| 1989 | 03:28 | 100  | 100 | 100     |
| 1989 | 03:29 | 100  | 100 | 100     |
| 1989 | 03:30 | 100  | 100 | 100     |
| 1989 | 03:31 | 100  | 100 | 100     |
| 1989 | 03:32 | 100  | 100 | 100     |
| 1989 | 03:33 | 100  | 100 | 100     |
| 1989 | 03:34 | 100  | 100 | 100     |
| 1989 | 03:35 | 100  | 100 | 100     |
| 1989 | 03:36 | 100  | 100 | 100     |
| 1989 | 03:37 | 100  | 100 | 100     |
| 1989 | 03:38 | 100  | 100 | 100     |
| 1989 | 03:39 | 100  | 100 | 100     |
| 1989 | 03:40 | 100  | 100 | 100     |
| 1989 | 03:41 | 100  | 100 | 100     |
| 1989 | 03:42 | 100  | 100 | 100     |
| 1989 | 03:43 | 100  | 100 | 100     |
| 1989 | 03:44 | 100  | 100 | 100     |
| 1989 | 03:45 | 100  | 100 | 100     |
| 1989 | 03:46 | 100  | 100 | 100     |
| 1989 | 03:47 | 100  | 100 | 100     |
| 1989 | 03:48 | 100  | 100 | 100     |
| 1989 | 03:49 | 100  | 100 | 100     |
| 1989 | 03:50 | 100  | 100 | 100     |
| 1989 | 03:51 | 100  | 100 | 100     |
| 1989 | 03:52 | 100  | 100 | 100     |
| 1989 | 03:53 | 100  | 100 | 100     |
| 1989 | 03:54 | 100  | 100 | 100     |
| 1989 | 03:55 | 100  | 100 | 100     |
| 1989 | 03:56 | 100  | 100 | 100     |
| 1989 | 03:57 | 100  | 100 | 100     |
| 1989 | 03:58 | 100  | 100 | 100     |
| 1989 | 03:59 | 100  | 100 | 100     |
| 1989 | 04:00 | 100  | 100 | 100     |

MARCH 23, 1989 RELEASE '938 305 PAGE 7

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121- 15.000 15.625  
 122- 25.000 20.000 15.625  
 123- 25.000 25.000 15.625  
 124- 25.000 30.000 15.625  
 125- 25.000 35.000 15.625  
 126- 25.000 40.000 15.625  
 127- 25.000 45.000 18.750  
 128- 25.000 50.000 18.750  
 129- 25.000 55.000 18.750  
 130- 25.000 60.000 18.750  
 131- 25.000 65.000 18.750  
 132- 25.000 70.000 18.750  
 133- 25.000 75.000 18.750  
 134- 25.000 80.000 18.750  
 135- 25.000 85.000 18.750  
 136- 25.000 90.000 18.750

COMPOS 227  
 UNIT 228 QUAD4 4-MODE SHELL ROOF TEST  
 DATA 229 COMPOSITE SHELL

SORTED BULK DATA ECHO

121- 15.000 15.625  
 122- 25.000 20.000 15.625  
 123- 25.000 25.000 15.625  
 124- 25.000 30.000 15.625  
 125- 25.000 35.000 15.625  
 126- 25.000 40.000 15.625  
 127- 25.000 45.000 18.750  
 128- 25.000 50.000 18.750  
 129- 25.000 55.000 18.750  
 130- 25.000 60.000 18.750  
 131- 25.000 65.000 18.750  
 132- 25.000 70.000 18.750  
 133- 25.000 75.000 18.750  
 134- 25.000 80.000 18.750  
 135- 25.000 85.000 18.750  
 136- 25.000 90.000 18.750

121- 15.000 15.625  
 122- 25.000 20.000 15.625  
 123- 25.000 25.000 15.625  
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 130- 25.000 60.000 18.750  
 131- 25.000 65.000 18.750  
 132- 25.000 70.000 18.750  
 133- 25.000 75.000 18.750  
 134- 25.000 80.000 18.750  
 135- 25.000 85.000 18.750  
 136- 25.000 90.000 18.750

COMPOS 227  
 UNIT 228 QUAD4 4-MODE SHELL ROOF TEST  
 DATA 229 COMPOSITE SHELL

SORTED BULK DATA ECHO

121- 15.000 15.625  
 122- 25.000 20.000 15.625  
 123- 25.000 25.000 15.625  
 124- 25.000 30.000 15.625  
 125- 25.000 35.000 15.625  
 126- 25.000 40.000 15.625  
 127- 25.000 45.000 18.750  
 128- 25.000 50.000 18.750  
 129- 25.000 55.000 18.750  
 130- 25.000 60.000 18.750  
 131- 25.000 65.000 18.750  
 132- 25.000 70.000 18.750  
 133- 25.000 75.000 18.750  
 134- 25.000 80.000 18.750  
 135- 25.000 85.000 18.750  
 136- 25.000 90.000 18.750

DISPLACEMENT VECTOR

| TIME     | 72           | 73           | R1          | R2           | R3          |
|----------|--------------|--------------|-------------|--------------|-------------|
| 0.000000 | 0.000000     | 2.23236E-02  | 0.000000    | 6.81759E-03  | 0.000000    |
| 0.000000 | 0.000000     | 3.00284E-02  | 5.88751E-02 | 1.46810E-05  | 1.75511E-03 |
| 0.000000 | 0.000000     | 3.26187E-02  | 1.10073E-01 | -1.59581E-02 | 2.56899E-03 |
| 0.000000 | 0.000000     | 3.38168E-02  | 1.48168E-01 | -3.95271E-02 | 3.44876E-02 |
| 0.000000 | 0.000000     | 2.94681E-02  | 1.68784E-01 | -6.87239E-02 | 3.96183E-03 |
| 0.000000 | 0.000000     | 1.41951E-02  | 1.65224E-01 | -1.01839E-01 | 4.42134E-03 |
| 0.000000 | 0.000000     | -1.83498E-02 | 1.58147E-01 | -1.36711E-01 | 4.19257E-03 |
| 0.000000 | 0.000000     | -7.52801E-02 | 1.24409E-01 | -1.70373E-01 | 4.61826E-03 |
| 0.000000 | 0.000000     | -1.64448E-01 | 0.000000    | -1.96604E-01 | 2.80910E-03 |
| 0.000000 | 0.000000     | 2.87232E-02  | 0.000000    | 6.78286E-03  | 0.000000    |
| 0.000000 | -2.29266E-03 | 2.38459E-02  | 6.73295E-02 | 3.83766E-03  | 1.57089E-02 |
| 0.000000 | -1.76015E-03 | 3.22452E-02  | 1.03340E-01 | -9.95937E-03 | 5.72463E-02 |
| 0.000000 | 3.89200E-03  | 3.26529E-02  | 1.75598E-01 | -3.88934E-02 | 3.68324E-02 |
| 0.000000 | 1.64288E-02  | 2.38363E-02  | 1.54963E-01 | -5.76124E-02 | 4.45450E-02 |
| 0.000000 | 3.72633E-02  | 1.37138E-02  | 1.53804E-01 | -8.21950E-02 | 4.97117E-02 |
| 0.000000 | 6.70894E-02  | -1.32114E-02 | 1.57363E-01 | -1.20445E-01 | 5.18400E-02 |
| 0.000000 | 1.00239E-01  | -7.36284E-02 | 1.51931E-01 | -1.52552E-01 | 5.05647E-02 |
| 0.000000 | 1.52846E-01  | -1.59212E-01 | 0.000000    | -1.77084E-01 | 4.65827E-02 |
| 0.000000 | 0.000000     | 2.80181E-02  | 0.000000    | 4.97711E-03  | 0.000000    |
| 0.000000 | -4.23572E-03 | 2.38978E-02  | 7.20386E-02 | 1.31631E-03  | 2.85585E-02 |
| 0.000000 | -2.26158E-03 | 3.97866E-02  | 1.16658E-01 | -1.03832E-02 | 5.17690E-02 |
| 0.000000 | 8.34089E-03  | 3.12855E-02  | 1.45486E-01 | -2.93205E-02 | 7.07545E-02 |
| 0.000000 | 3.27882E-02  | 2.56538E-02  | 1.60832E-01 | -5.32339E-02 | 8.53152E-02 |
| 0.000000 | 1.50381E-01  | 1.21080E-02  | 1.60914E-01 | -8.61595E-02 | 9.46336E-02 |
| 0.000000 | 1.00381E-01  | -1.77819E-02 | 1.51621E-01 | -1.08223E-01 | 9.80549E-02 |
| 0.000000 | 2.04540E-01  | -6.86884E-02 | 1.33395E-01 | -1.36088E-01 | 9.62893E-02 |
| 0.000000 | 2.94087E-01  | -1.46061E-01 | 0.000000    | -1.58674E-01 | 9.26475E-02 |
| 0.000000 | 0.000000     | 2.81204E-02  | 0.000000    | 1.28124E-05  | 0.000000    |
| 0.000000 | -2.09236E-03 | 2.67680E-02  | 8.74357E-02 | 5.29531E-04  | 9.93138E-02 |
| 0.000000 | -2.79284E-03 | 2.79818E-02  | 9.96254E-02 | -1.00230E-02 | 7.38939E-02 |
| 0.000000 | 1.33671E-02  | 2.78028E-02  | 1.15470E-01 | -2.57839E-02 | 1.00408E-01 |
| 0.000000 | 4.84272E-02  | 2.30733E-02  | 1.21856E-01 | -4.59617E-02 | 1.20727E-01 |
| 0.000000 | 1.05755E-01  | 9.74782E-03  | 1.20537E-01 | -5.87718E-02 | 1.33281E-01 |
| 0.000000 | 1.87445E-01  | -1.56926E-02 | 1.14344E-01 | -9.12817E-02 | 1.37963E-01 |



| ID | TYPE | DISPLACEMENT VECTOR |              |             |              |             |
|----|------|---------------------|--------------|-------------|--------------|-------------|
|    |      | T1                  | T2           | R1          | R2           | R3          |
| 1  | 0    | 1.26432E+00         | -6.97532E-02 | 1.15367E-01 | -1.17092E-01 | 1.37189E-01 |
| 2  | 0    | 1.26121E+00         | -1.28231E-01 | .0          | -1.37425E-01 | 1.35458E-01 |
| 3  | 0    | 1.52861E+00         | 2.39557E-02  | .0          | 6.03336E-04  | .0          |
| 4  | 0    | 1.16916E+00         | 2.34331E-02  | 5.53408E-02 | -2.26461E-03 | 4.80910E-02 |
| 5  | 0    | 1.66555E+00         | 2.39362E-02  | 1.95623E-01 | 2.66461E-03  | 9.00247E-02 |
| 6  | 0    | 1.66555E+00         | 2.34331E-02  | 1.21108E-01 | -2.11981E-02 | 1.24533E-01 |
| 7  | 0    | 1.42123E+00         | 2.36538E-02  | 1.26426E-01 | 3.65541E-02  | 1.49733E-01 |
| 8  | 0    | 1.42123E+00         | 2.36538E-02  | 1.21977E-01 | -5.21842E-02 | 1.52288E-01 |
| 9  | 0    | 1.33631E+00         | 2.34331E-02  | 1.11931E-01 | 7.49007E-02  | 1.71818E-01 |
| 10 | 0    | 1.33631E+00         | 2.34331E-02  | 9.45355E-02 | -9.52895E-02 | 1.72678E-01 |
| 11 | 0    | 1.33631E+00         | 2.34331E-02  | .0          | -1.13119E-01 | 1.73460E-01 |
| 12 | 0    | 1.33631E+00         | 2.34331E-02  | 9.85430E-02 | -5.23975E-04 | 5.28342E-02 |
| 13 | 0    | 1.33631E+00         | 2.34331E-02  | 7.60708E-02 | -7.01659E-03 | 1.82630E-01 |
| 14 | 0    | 1.33631E+00         | 2.34331E-02  | 7.51005E-02 | -1.55965E-02 | 1.42639E-01 |
| 15 | 0    | 1.33631E+00         | 2.34331E-02  | 7.08596E-02 | -2.72795E-03 | 1.22287E-01 |
| 16 | 0    | 1.33631E+00         | 2.34331E-02  | 6.63188E-02 | -4.11150E-02 | 1.90534E-01 |
| 17 | 0    | 1.33631E+00         | 2.34331E-02  | 5.33582E-02 | -5.33919E-02 | 1.98912E-01 |
| 18 | 0    | 1.33631E+00         | 2.34331E-02  | 7.65989E-02 | -1.36849E-02 | 2.01555E-01 |
| 19 | 0    | 1.33631E+00         | 2.34331E-02  | .0          | -0.55317E-03 | 2.04395E-01 |
| 20 | 0    | 1.33631E+00         | 2.34331E-02  | 3.80582E-02 | 1.19698E-03  | .0          |
| 21 | 0    | 1.33631E+00         | 2.34331E-02  | 5.10407E-02 | -4.52463E-03 | 5.78999E-02 |
| 22 | 0    | 1.33631E+00         | 2.34331E-02  | 1.00372E-01 | -5.31245E-03 | 1.10439E-01 |
| 23 | 0    | 1.33631E+00         | 2.34331E-02  | 1.00372E-01 | -1.05920E-02 | 1.55104E-01 |
| 24 | 0    | 1.33631E+00         | 2.34331E-02  | 9.02432E-01 | -1.78356E-02 | 1.88193E-01 |
| 25 | 0    | 1.33631E+00         | 2.34331E-02  | 9.02432E-01 | -2.69579E-02 | 2.08914E-01 |
| 26 | 0    | 1.33631E+00         | 2.34331E-02  | 7.48577E-02 | -3.72878E-02 | 2.18916E-01 |
| 27 | 0    | 1.33631E+00         | 2.34331E-02  | 4.53184E-02 | -4.75038E-02 | 2.23919E-01 |
| 28 | 0    | 1.33631E+00         | 2.34331E-02  | .0          | -5.91737E-02 | 2.26532E-01 |
| 29 | 0    | 1.33631E+00         | 2.34331E-02  | .0          | -1.64395E-02 | .0          |
| 30 | 0    | 1.33631E+00         | 2.34331E-02  | 1.04337E-01 | 9.22681E-04  | 5.79143E-02 |
| 31 | 0    | 1.33631E+00         | 2.34331E-02  | 4.18160E-02 | -9.22681E-04 | 1.14909E-01 |
| 32 | 0    | 1.33631E+00         | 2.34331E-02  | 1.75592E-02 | -1.82023E-03 | 1.14909E-01 |
| 33 | 0    | 1.33631E+00         | 2.34331E-02  | 1.75592E-02 | -3.82043E-03 | 1.51928E-01 |
| 34 | 0    | 1.33631E+00         | 2.34331E-02  | 1.75592E-02 | -7.42340E-03 | 1.97027E-01 |





UNITED STATES OF AMERICA  
DEPARTMENT OF HEALTH AND HUMAN SERVICES  
CENTERS FOR DISEASE CONTROL AND PREVENTION

LOAD U E C T O R

R3

R2

R1

T3

T2

T1

T0

T-1

T-2

T-3

T-4

T-5

T-6

T-7

T-8

T-9

T-10

T-11

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T-15

T-16

T-17

T-18





|     |         |       |                      |      |      |                              |               |
|-----|---------|-------|----------------------|------|------|------------------------------|---------------|
| 31- | FORCE   | 1     | 24                   | -4.0 | 1.0  | 0.0                          | 0.0           |
| 32- | FORCE   | 1     | 30                   | -4.0 | 1.0  | 0.0                          | 0.0           |
| 33- | FORCE   | 1     | 36                   | -2.0 | 1.0  | 0.0                          | 0.0           |
| 34- | GRID    | 1     |                      | 0.0  | 0.0  |                              |               |
| 35- | GRID    | 2     |                      | 3.0  | 0.0  |                              |               |
| 36- | GRID    | 3     |                      | 6.0  | 0.0  |                              |               |
| 37- | GRID    | 4     |                      | 9.0  | 0.0  |                              |               |
| 38- | GRID    | 5     |                      | 12.0 | 0.0  |                              |               |
| 39- | GRID    | 6     |                      | 15.0 | 0.0  |                              |               |
| 40- | GRID    | 7     |                      | 0.0  | 0.0  |                              |               |
| 41- | GRID    | 8     |                      | 3.0  | 0.0  |                              |               |
| 42- | GRID    | 9     |                      | 6.0  | 0.0  |                              |               |
| 43- | GRID    | 10    |                      | 9.0  | 0.0  |                              |               |
| 44- | GRID    | 11    |                      | 12.0 | 0.0  |                              |               |
| 45- | GRID    | 12    |                      | 15.0 | 0.0  |                              |               |
| 46- | GRID    | 13    |                      | 0.0  | 0.0  |                              |               |
| 47- | GRID    | 14    |                      | 3.0  | 0.0  |                              |               |
| 48- | GRID    | 15    |                      | 6.0  | 0.0  |                              |               |
| 49- | GRID    | 16    |                      | 9.0  | 0.0  |                              |               |
| 50- | GRID    | 17    |                      | 12.0 | 0.0  |                              |               |
| 51- | GRID    | 18    |                      | 15.0 | 0.0  |                              |               |
| 52- | GRID    | 19    |                      | 0.0  | 0.0  |                              |               |
| 53- | GRID    | 20    |                      | 3.0  | 0.0  |                              |               |
| 54- | GRID    | 21    |                      | 6.0  | 0.0  |                              |               |
| 55- | GRID    | 22    |                      | 9.0  | 0.0  |                              |               |
| 56- | GRID    | 23    |                      | 12.0 | 0.0  |                              |               |
| 57- | GRID    | 24    |                      | 15.0 | 0.0  |                              |               |
| 58- | GRID    | 25    |                      | 0.0  | 0.0  |                              |               |
| 59- | GRID    | 26    |                      | 3.0  | 0.0  |                              |               |
| 60- | GRID    | 27    |                      | 6.0  | 0.0  |                              |               |
| 61- | GRID    | 28    |                      | 9.0  | 0.0  |                              |               |
| 62- | GRID    | 29    |                      | 12.0 | 0.0  |                              |               |
| 63- | GRID    | 30    |                      | 15.0 | 0.0  |                              |               |
| 64- | GRID    | 31    |                      | 0.0  | 0.0  |                              |               |
| 65- | GRID    | 32    |                      | 3.0  | 0.0  |                              |               |
| 66- | GRID    | 33    |                      | 6.0  | 0.0  |                              |               |
| 67- | GRID    | 34    |                      | 9.0  | 0.0  |                              |               |
| 68- | GRID    | 35    |                      | 12.0 | 0.0  |                              |               |
| 69- | GRID    | 36    |                      | 15.0 | 0.0  |                              |               |
| 70- | MAT8    | 22    | 18.5E+06 1.6E+06 .25 | 20.0 | 0.0  | .65E+06 4.0E+05 4.0E+05 .005 |               |
| 71- | PARAM   | GRPNT | 0                    |      |      | 22                           | .0236 SYM ABC |
| 72- | +BC     | 1     |                      |      |      |                              |               |
| 73- | SPC1    | 45.   | -45.                 | 90.  | 0.   |                              |               |
| 74- | SPC1    | 1     | 6                    | 1    | THRU | 36                           |               |
| 75- | SPC1    | 1     | 36                   | 1    | THRU | 6                            |               |
| 76- | SPC1    | 1     | 36                   | 6    | 12   | 18                           | 36            |
| 77- | SPC1    | 1     | 136                  | 1    | 7    | 13                           | 31            |
| 78- | SPC1    | 1     | 236                  | 31   | THRU | 19                           | 25            |
|     | ENDDATA |       |                      |      |      |                              |               |

# DISPLACEMENT VECTOR

| POINT ID. | TYPE | T1            | T2            | T3            | R1            | R2            | R3  |
|-----------|------|---------------|---------------|---------------|---------------|---------------|-----|
| 1         | G    | 0.0           | -4.586931E-06 | 0.0           | -3.739354E-22 | 1.350191E-22  | 0.0 |
| 2         | G    | -2.200378E-06 | -4.586931E-06 | 0.0           | -2.377961E-21 | -1.630386E-22 | 0.0 |
| 3         | G    | -4.400757E-06 | -4.586931E-06 | 0.0           | -3.200120E-21 | -2.832612E-22 | 0.0 |
| 4         | G    | -6.601135E-06 | -4.586931E-06 | 0.0           | -2.924989E-21 | -4.927746E-22 | 0.0 |
| 5         | G    | -8.801513E-06 | -4.586931E-06 | 0.0           | -1.749883E-21 | -6.912656E-22 | 0.0 |
| 6         | G    | -1.100189E-05 | -4.586931E-06 | 0.0           | -2.375740E-22 | -6.298611E-22 | 0.0 |
| 7         | G    | 0.0           | -3.669544E-06 | 0.0           | -1.472087E-22 | 2.774235E-21  | 0.0 |
| 8         | G    | -2.200378E-06 | -3.669544E-06 | -7.287472E-21 | -1.450458E-21 | 1.688366E-21  | 0.0 |
| 9         | G    | -4.400757E-06 | -3.669544E-06 | -1.095683E-20 | -2.326285E-21 | 1.925028E-22  | 0.0 |
| 10        | G    | -6.601135E-06 | -3.669544E-06 | -1.044381E-20 | -2.380581E-21 | -1.288238E-21 | 0.0 |
| 11        | G    | -8.801513E-06 | -3.669544E-06 | -6.208032E-21 | -1.622087E-21 | -2.497188E-21 | 0.0 |
| 12        | G    | -1.100189E-05 | -3.669544E-06 | 0.0           | -3.016316E-22 | -2.690360E-21 | 0.0 |
| 13        | G    | 0.0           | -2.752158E-06 | 0.0           | -1.581260E-22 | 3.783715E-21  | 0.0 |
| 14        | G    | -2.200378E-06 | -2.752158E-06 | 1.067713E-20  | -4.566404E-22 | 2.795794E-21  | 0.0 |
| 15        | G    | -4.400757E-06 | -2.752158E-06 | -1.683721E-20 | -8.041368E-22 | 6.983753E-22  | 0.0 |
| 16        | G    | -6.601135E-06 | -2.752158E-06 | -1.660366E-20 | -9.408494E-22 | -1.586746E-21 | 0.0 |
| 17        | G    | -8.801513E-06 | -2.752158E-06 | -1.021450E-20 | -7.443478E-22 | -3.527765E-21 | 0.0 |
| 18        | G    | -1.100189E-05 | -2.752158E-06 | 0.0           | -2.417337E-22 | -4.189631E-21 | 0.0 |
| 19        | G    | 0.0           | -1.834772E-06 | 0.0           | -3.906152E-24 | 3.473266E-21  | 0.0 |
| 20        | G    | -2.200378E-06 | -1.834772E-06 | -1.021954E-20 | 4.793626E-22  | 2.806174E-21  | 0.0 |
| 21        | G    | -4.400757E-06 | -1.834772E-06 | -1.660582E-20 | 6.688025E-22  | 8.647538E-22  | 0.0 |
| 22        | G    | -6.601135E-06 | -1.834772E-06 | -1.683507E-20 | 5.326254E-22  | -1.420836E-21 | 0.0 |
| 23        | G    | -8.801513E-06 | -1.834772E-06 | -1.067168E-20 | 1.922288E-22  | -3.517113E-21 | 0.0 |
| 24        | G    | -1.100189E-05 | -1.834772E-06 | 0.0           | -8.275889E-23 | -4.500022E-21 | 0.0 |
| 25        | G    | 0.0           | -9.173861E-07 | 0.0           | 5.363293E-23  | 1.973869E-21  | 0.0 |
| 26        | G    | -2.200378E-06 | -9.173861E-07 | -6.212500E-21 | 1.355181E-21  | 1.772284E-21  | 0.0 |
| 27        | G    | -4.400757E-06 | -9.173861E-07 | -1.044628E-20 | 2.108294E-21  | 5.667328E-22  | 0.0 |
| 28        | G    | -6.601135E-06 | -9.173861E-07 | -1.095559E-20 | 2.054565E-21  | -9.129689E-22 | 0.0 |
| 29        | G    | -8.801513E-06 | -9.173861E-07 | -7.282610E-21 | 1.187933E-21  | -2.409765E-21 | 0.0 |
| 30        | G    | -1.100189E-05 | -9.173861E-07 | 0.0           | -9.025533E-23 | -3.488668E-21 | 0.0 |
| 31        | G    | 0.0           | 0.0           | 0.0           | -1.634195E-23 | -8.226114E-23 | 0.0 |
| 32        | G    | -2.200378E-06 | 0.0           | 0.0           | 1.483997E-21  | -2.799380E-23 | 0.0 |
| 33        | G    | -4.400757E-06 | 0.0           | 0.0           | 2.653633E-21  | -2.217385E-22 | 0.0 |
| 34        | G    | -6.601135E-06 | 0.0           | 0.0           | 2.928972E-21  | -4.339853E-22 | 0.0 |
| 35        | G    | -8.801513E-06 | 0.0           | 0.0           | 2.114021E-21  | -5.510358E-22 | 0.0 |
| 36        | G    | -1.100189E-05 | 0.0           | 0.0           | 1.404663E-22  | -8.513567E-22 | 0.0 |

\* \* \* END OF JOB \* \* \*



N A S T R A N   E X E C U T I V E   C O N T R O L   D E C K   E C H O

ID   COMPOSITE PLATE   15 BY 20   NASTRAN USERS COLLOQUIUM   1990  
 APP DISPLACEMENT  
 SOL 1  
 TIME 10  
 CEND

C A S E   C O N T R O L   D E C K   E C H O

1   TITLE = PROBLEM 13 CASE 1 INPLANE LOADING  
 2   LOAD = 1  
 3   SPC = 1  
 4   OUTPUT  
 5   CLOAD=ALL  
 6   CRESS=ALL  
 7   CSMPLACEMENTS=ALL  
 8   STRAIN=ALL  
 9   BEGIN BULK

| COUNT | 1      | 2   | 3   | 4    | 5   | 6   | 7   | 8   | 9   | 10   |
|-------|--------|-----|-----|------|-----|-----|-----|-----|-----|------|
| 1-    | CCOR2R | 1   | 0.0 | 0.0  | 0.0 | 0.0 | 0.0 | 0.0 | 1.0 | ..10 |
| 2-    | +XYP   | 1.0 | 0.0 | 0.0  | 0.0 | 0.0 | 0.0 | 0.0 | 1.0 | +XYP |
| 3-    | QUAD4  | 1   | 1   | 2    | 3   | 8   | 7   | 1   |     |      |
| 4-    | QUAD4  | 2   | 1   | 3    | 4   | 9   | 8   | 1   |     |      |
| 5-    | QUAD4  | 3   | 1   | 4    | 5   | 10  | 9   | 1   |     |      |
| 6-    | QUAD4  | 4   | 1   | 5    | 6   | 11  | 10  | 1   |     |      |
| 7-    | QUAD4  | 5   | 1   | 6    | 7   | 12  | 11  | 1   |     |      |
| 8-    | QUAD4  | 6   | 1   | 7    | 8   | 13  | 12  | 1   |     |      |
| 9-    | QUAD4  | 7   | 1   | 8    | 9   | 14  | 13  | 1   |     |      |
| 10-   | QUAD4  | 8   | 1   | 9    | 10  | 15  | 14  | 1   |     |      |
| 11-   | QUAD4  | 9   | 1   | 10   | 11  | 16  | 15  | 1   |     |      |
| 12-   | QUAD4  | 10  | 1   | 11   | 12  | 17  | 16  | 1   |     |      |
| 13-   | QUAD4  | 11  | 1   | 12   | 13  | 18  | 17  | 1   |     |      |
| 14-   | QUAD4  | 12  | 1   | 13   | 14  | 19  | 18  | 1   |     |      |
| 15-   | QUAD4  | 13  | 1   | 14   | 15  | 20  | 19  | 1   |     |      |
| 16-   | QUAD4  | 14  | 1   | 15   | 16  | 21  | 20  | 1   |     |      |
| 17-   | QUAD4  | 15  | 1   | 16   | 17  | 22  | 21  | 1   |     |      |
| 18-   | QUAD4  | 16  | 1   | 17   | 18  | 23  | 22  | 1   |     |      |
| 19-   | QUAD4  | 17  | 1   | 18   | 19  | 24  | 23  | 1   |     |      |
| 20-   | QUAD4  | 18  | 1   | 19   | 20  | 25  | 24  | 1   |     |      |
| 21-   | QUAD4  | 19  | 1   | 20   | 21  | 26  | 25  | 1   |     |      |
| 22-   | QUAD4  | 20  | 1   | 21   | 22  | 27  | 26  | 1   |     |      |
| 23-   | QUAD4  | 21  | 1   | 22   | 23  | 28  | 27  | 1   |     |      |
| 24-   | QUAD4  | 22  | 1   | 23   | 24  | 29  | 28  | 1   |     |      |
| 25-   | QUAD4  | 23  | 1   | 24   | 25  | 30  | 29  | 1   |     |      |
| 26-   | QUAD4  | 24  | 1   | 25   | 26  | 31  | 30  | 1   |     |      |
| 27-   | QUAD4  | 25  | 1   | 26   | 27  | 32  | 31  | 1   |     |      |
| 28-   | QUAD4  | 26  | 1   | 27   | 28  | 33  | 32  | 1   |     |      |
| 29-   | QUAD4  | 27  | 1   | 28   | 29  | 34  | 33  | 1   |     |      |
| 30-   | QUAD4  | 28  | 1   | 29   | 30  | 35  | 34  | 1   |     |      |
| 31-   | QUAD4  | 29  | 1   | 30   | 31  | 36  | 35  | 1   |     |      |
|       | FORCE  | 1   | 6   | -2.0 | 0.0 | 0.0 | 0.0 | 0.0 |     |      |
|       | FORCE  | 1   | 12  | -4.0 | 1.0 | 0.0 | 0.0 | 0.0 |     |      |
|       | FORCE  | 1   | 18  | -4.0 | 1.0 | 0.0 | 0.0 | 0.0 |     |      |
|       | FORCE  | 1   | 24  | -4.0 | 1.0 | 0.0 | 0.0 | 0.0 |     |      |

|     |         |    |    |      |     |     |     |  |
|-----|---------|----|----|------|-----|-----|-----|--|
| 32- | FORCE   | 1  | 30 | -4.0 | 1.0 | 0.0 | 0.0 |  |
| 33- | FORCE   | 1  | 36 | -2.0 | 1.0 | 0.0 | 0.0 |  |
| 34- | GRID    | 1  |    | 0.0  | 0.0 |     |     |  |
| 35- | GRID    | 2  |    | 0.0  | 0.0 |     |     |  |
| 36- | GRID    | 3  |    | 0.0  | 0.0 |     |     |  |
| 37- | GRID    | 4  |    | 0.0  | 0.0 |     |     |  |
| 38- | GRID    | 5  |    | 0.0  | 0.0 |     |     |  |
| 39- | GRID    | 6  |    | 0.0  | 0.0 |     |     |  |
| 40- | GRID    | 7  |    | 0.0  | 0.0 |     |     |  |
| 41- | GRID    | 8  |    | 0.0  | 0.0 |     |     |  |
| 42- | GRID    | 9  |    | 0.0  | 0.0 |     |     |  |
| 43- | GRID    | 10 |    | 0.0  | 0.0 |     |     |  |
| 44- | GRID    | 11 |    | 0.0  | 0.0 |     |     |  |
| 45- | GRID    | 12 |    | 0.0  | 0.0 |     |     |  |
| 46- | GRID    | 13 |    | 0.0  | 0.0 |     |     |  |
| 47- | GRID    | 14 |    | 0.0  | 0.0 |     |     |  |
| 48- | GRID    | 15 |    | 0.0  | 0.0 |     |     |  |
| 49- | GRID    | 16 |    | 0.0  | 0.0 |     |     |  |
| 50- | GRID    | 17 |    | 0.0  | 0.0 |     |     |  |
| 51- | GRID    | 18 |    | 0.0  | 0.0 |     |     |  |
| 52- | GRID    | 19 |    | 0.0  | 0.0 |     |     |  |
| 53- | GRID    | 20 |    | 0.0  | 0.0 |     |     |  |
| 54- | GRID    | 21 |    | 0.0  | 0.0 |     |     |  |
| 55- | GRID    | 22 |    | 0.0  | 0.0 |     |     |  |
| 56- | GRID    | 23 |    | 0.0  | 0.0 |     |     |  |
| 57- | GRID    | 24 |    | 0.0  | 0.0 |     |     |  |
| 58- | GRID    | 25 |    | 0.0  | 0.0 |     |     |  |
| 59- | GRID    | 26 |    | 0.0  | 0.0 |     |     |  |
| 60- | GRID    | 27 |    | 0.0  | 0.0 |     |     |  |
| 61- | GRID    | 28 |    | 0.0  | 0.0 |     |     |  |
| 62- | GRID    | 29 |    | 0.0  | 0.0 |     |     |  |
| 63- | GRID    | 30 |    | 0.0  | 0.0 |     |     |  |
| 64- | GRID    | 31 |    | 0.0  | 0.0 |     |     |  |
| 65- | GRID    | 32 |    | 0.0  | 0.0 |     |     |  |
| 66- | GRID    | 33 |    | 0.0  | 0.0 |     |     |  |
| 67- | GRID    | 34 |    | 0.0  | 0.0 |     |     |  |
| 68- | GRID    | 35 |    | 0.0  | 0.0 |     |     |  |
| 69- | GRID    | 36 |    | 0.0  | 0.0 |     |     |  |
| 70- | MAT8    | 22 |    | 0.0  | 0.0 |     |     |  |
| 71- | PARAM   | 22 |    | 0.0  | 0.0 |     |     |  |
| 72- | PCOMP   | 1  |    | 0.0  | 0.0 |     |     |  |
| 73- | +KKQ    | 22 |    | 0.0  | 0.0 |     |     |  |
| 74- | +KKQ    | 22 |    | 0.0  | 0.0 |     |     |  |
| 75- | +KKS    | 22 |    | 0.0  | 0.0 |     |     |  |
| 76- | +KKR    | 22 |    | 0.0  | 0.0 |     |     |  |
| 77- | SPC1    | 1  |    | 0.0  | 0.0 |     |     |  |
| 78- | SPC1    | 1  |    | 0.0  | 0.0 |     |     |  |
| 79- | SPC1    | 1  |    | 0.0  | 0.0 |     |     |  |
| 80- | SPC1    | 1  |    | 0.0  | 0.0 |     |     |  |
| 81- | SPC1    | 1  |    | 0.0  | 0.0 |     |     |  |
|     | ENDDATA |    |    |      |     |     |     |  |

| POINT ID. | TYPE | T1            | T2            | T3  | R1  | R2  | R3  |
|-----------|------|---------------|---------------|-----|-----|-----|-----|
| 1         | G    | 0.0           | -4.586931E-06 | 0.0 | 0.0 | 0.0 | 0.0 |
| 2         | G    | -2.200378E-06 | -4.586931E-06 | 0.0 | 0.0 | 0.0 | 0.0 |
| 3         | G    | -4.400757E-06 | -4.586931E-06 | 0.0 | 0.0 | 0.0 | 0.0 |
| 4         | G    | -6.601135E-06 | -4.586931E-06 | 0.0 | 0.0 | 0.0 | 0.0 |
| 5         | G    | -8.801513E-06 | -4.586931E-06 | 0.0 | 0.0 | 0.0 | 0.0 |
| 6         | G    | -1.100189E-05 | -4.586931E-06 | 0.0 | 0.0 | 0.0 | 0.0 |
| 7         | G    | 0.0           | -3.669545E-06 | 0.0 | 0.0 | 0.0 | 0.0 |
| 8         | G    | -2.290378E-06 | -3.669545E-06 | 0.0 | 0.0 | 0.0 | 0.0 |
| 9         | G    | -4.400757E-06 | -3.669545E-06 | 0.0 | 0.0 | 0.0 | 0.0 |
| 10        | G    | -6.601135E-06 | -3.669545E-06 | 0.0 | 0.0 | 0.0 | 0.0 |
| 11        | G    | -8.801513E-06 | -3.669545E-06 | 0.0 | 0.0 | 0.0 | 0.0 |
| 12        | G    | -1.100189E-05 | -3.669545E-06 | 0.0 | 0.0 | 0.0 | 0.0 |
| 13        | G    | 0.0           | -2.752158E-06 | 0.0 | 0.0 | 0.0 | 0.0 |
| 14        | G    | -2.200378E-06 | -2.752158E-06 | 0.0 | 0.0 | 0.0 | 0.0 |
| 15        | G    | -4.400757E-06 | -2.752158E-06 | 0.0 | 0.0 | 0.0 | 0.0 |
| 16        | G    | -6.601135E-06 | -2.752158E-06 | 0.0 | 0.0 | 0.0 | 0.0 |
| 17        | G    | -8.801513E-06 | -2.752158E-06 | 0.0 | 0.0 | 0.0 | 0.0 |
| 18        | G    | -1.100189E-05 | -2.752158E-06 | 0.0 | 0.0 | 0.0 | 0.0 |
| 19        | G    | 0.0           | -1.834772E-06 | 0.0 | 0.0 | 0.0 | 0.0 |
| 20        | G    | -2.200378E-06 | -1.834772E-06 | 0.0 | 0.0 | 0.0 | 0.0 |
| 21        | G    | -4.400757E-06 | -1.834772E-06 | 0.0 | 0.0 | 0.0 | 0.0 |
| 22        | G    | -6.601135E-06 | -1.834772E-06 | 0.0 | 0.0 | 0.0 | 0.0 |
| 23        | G    | -8.801513E-06 | -1.834772E-06 | 0.0 | 0.0 | 0.0 | 0.0 |
| 24        | G    | -1.100189E-05 | -1.834772E-06 | 0.0 | 0.0 | 0.0 | 0.0 |
| 25        | G    | 0.0           | -9.173861E-07 | 0.0 | 0.0 | 0.0 | 0.0 |
| 26        | G    | -2.200378E-06 | -9.173861E-07 | 0.0 | 0.0 | 0.0 | 0.0 |
| 27        | G    | -4.400757E-06 | -9.173861E-07 | 0.0 | 0.0 | 0.0 | 0.0 |
| 28        | G    | -6.601135E-06 | -9.173861E-07 | 0.0 | 0.0 | 0.0 | 0.0 |
| 29        | G    | -8.801513E-06 | -9.173861E-07 | 0.0 | 0.0 | 0.0 | 0.0 |
| 30        | G    | -1.100189E-05 | -9.173861E-07 | 0.0 | 0.0 | 0.0 | 0.0 |
| 31        | G    | 0.0           | 0.0           | 0.0 | 0.0 | 0.0 | 0.0 |
| 32        | G    | -2.200378E-06 | 0.0           | 0.0 | 0.0 | 0.0 | 0.0 |
| 33        | G    | -4.400757E-06 | 0.0           | 0.0 | 0.0 | 0.0 | 0.0 |
| 34        | G    | -6.601135E-06 | 0.0           | 0.0 | 0.0 | 0.0 | 0.0 |
| 35        | G    | -8.801513E-06 | 0.0           | 0.0 | 0.0 | 0.0 | 0.0 |
| 36        | G    | -1.100189E-05 | 0.0           | 0.0 | 0.0 | 0.0 | 0.0 |

\* \* \* END OF JOB \* \*



218

# DISPLACEMENT VECTOR

| POINT ID. | TYPE | T1            | T2            | T3            | R1            | R2            | R3  |
|-----------|------|---------------|---------------|---------------|---------------|---------------|-----|
| 1         | G    | 0.0           | -1.77870E-19  | 0.0           | -1.210504E-03 | 1.323176E-03  | 0.0 |
| 2         | G    | -6.450161E-20 | -2.176825E-19 | 0.0           | -1.668269E-02 | -2.038859E-04 | 0.0 |
| 3         | G    | -1.034476E-19 | -2.616967E-19 | 0.0           | -2.286059E-02 | 1.153546E-04  | 0.0 |
| 4         | G    | -1.195788E-19 | -2.633216E-19 | 0.0           | -2.066868E-02 | -5.39371E-04  | 0.0 |
| 5         | G    | -1.218607E-19 | -2.115229E-19 | 0.0           | -1.161163E-02 | -8.255243E-04 | 0.0 |
| 6         | G    | -1.204124E-19 | -1.236314E-19 | 0.0           | 1.190341E-04  | 4.164568E-04  | 0.0 |
| 7         | G    | 0.0           | -1.481314E-19 | 0.0           | 6.716475E-05  | 2.187735E-02  | 0.0 |
| 8         | G    | -2.842967E-20 | -1.810125E-19 | -5.659059E-02 | -9.983173E-03 | 1.411448E-02  | 0.0 |
| 9         | G    | -8.923749E-20 | -2.230359E-19 | -8.471517E-02 | -1.663885E-02 | 3.682787E-03  | 0.0 |
| 10        | G    | -1.576236E-19 | -2.311706E-19 | -8.078058E-02 | -1.696210E-02 | -6.537446E-03 | 0.0 |
| 11        | G    | -2.094548E-19 | -1.946830E-19 | -4.827733E-02 | -1.106920E-02 | -1.479205E-02 | 0.0 |
| 12        | G    | -2.278775E-19 | -1.268147E-19 | 0.0           | -9.165117E-04 | -1.563870E-02 | 0.0 |
| 13        | G    | 0.0           | -1.115810E-19 | 0.0           | -1.869885E-04 | 2.978237E-02  | 0.0 |
| 14        | G    | -1.794663E-20 | -1.250619E-19 | -8.308844E-02 | -2.497156E-03 | 2.273052E-02  | 0.0 |
| 15        | G    | -8.58504E-20  | -1.460906E-19 | -1.305235E-01 | -5.125305E-03 | 7.580590E-03  | 0.0 |
| 16        | G    | -1.763567E-19 | -1.563161E-19 | -1.287307E-01 | -6.098102E-03 | -8.856641E-03 | 0.0 |
| 17        | G    | -2.526314E-19 | -1.483047E-19 | -7.953523E-02 | -4.516714E-03 | -2.280502E-02 | 0.0 |
| 18        | G    | -2.825843E-19 | -1.291995E-19 | 0.0           | -6.436033E-04 | -2.735314E-02 | 0.0 |
| 19        | G    | 0.0           | -7.106225E-20 | 0.0           | 6.436033E-04  | 2.735314E-02  | 0.0 |
| 20        | G    | -1.210820E-20 | -6.589079E-20 | -7.953523E-02 | 4.516714E-03  | 2.280602E-02  | 0.0 |
| 21        | G    | -7.136112E-20 | -6.327438E-20 | -1.287307E-01 | 6.098102E-03  | 8.856641E-03  | 0.0 |
| 22        | G    | -1.546190E-19 | -7.015307E-20 | -1.305235E-01 | 5.125305E-03  | -7.580590E-03 | 0.0 |
| 23        | G    | -2.290863E-19 | -8.759032E-20 | -8.308844E-02 | 2.497156E-03  | -2.273052E-02 | 0.0 |
| 24        | G    | -2.619817E-19 | -1.198988E-19 | 0.0           | 1.869885E-04  | -2.978237E-02 | 0.0 |
| 25        | G    | 0.0           | -3.792629E-20 | 0.0           | 9.165117E-04  | 1.563870E-02  | 0.0 |
| 26        | G    | -1.775468E-20 | -2.082724E-20 | -4.827733E-02 | 1.106920E-02  | 1.479205E-02  | 0.0 |
| 27        | G    | -5.709451E-20 | -6.595195E-21 | -8.078058E-02 | 1.696210E-02  | 6.537446E-03  | 0.0 |
| 28        | G    | -1.057224E-19 | -8.386640E-21 | -8.471517E-02 | 1.663885E-02  | -3.682787E-03 | 0.0 |
| 29        | G    | -1.524044E-19 | -2.939486E-20 | -5.659059E-02 | 9.983173E-03  | -1.411448E-02 | 0.0 |
| 30        | G    | -1.795011E-19 | -8.353070E-20 | 0.0           | -6.716475E-05 | -2.187735E-02 | 0.0 |
| 31        | G    | 0.0           | 0.0           | 0.0           | -1.190341E-04 | -4.164568E-04 | 0.0 |
| 32        | G    | -3.228108E-20 | 0.0           | 0.0           | 1.161163E-02  | 8.255243E-04  | 0.0 |
| 33        | G    | -5.761368E-20 | 0.0           | 0.0           | 2.066868E-02  | 5.39371E-04   | 0.0 |
| 34        | G    | -7.971998E-20 | 0.0           | 0.0           | 2.286059E-02  | -1.153546E-04 | 0.0 |
| 35        | G    | -9.792119E-20 | 0.0           | 0.0           | 1.668269E-02  | 2.038859E-04  | 0.0 |
| 36        | G    | -1.171910E-19 | 0.0           | 0.0           | 1.210504E-03  | -1.323176E-03 | 0.0 |

\*\*\* END OF JOB \*\*\*

N A S T R A N   E X E C U T I V E   C O N T R O L   D E C K   E C H O

ID   COMPOSITE PLATE 15 BY 20   NASTRAN USERS COLLOQUIUM 1990  
 APP DISPLACEMENT  
 SOL 1  
 TIME 10  
 CEND

C A S E   C O N T R O L   D E C K   E C H O

TITLE = PROBLEM 13 CASE 2   PRESSURE LOAD

LOAD = 1

SPC = 1

OUTPUT

OLoad=ALL

STRESS=ALL

DISPLACEMENTS=ALL

STRAIN=ALL

BEGIN BULK

| CARD<br>COUNT | 1      | 2   | 3    | 4   | 5   | 6   | 7   | 8   | 9   | 10   |
|---------------|--------|-----|------|-----|-----|-----|-----|-----|-----|------|
| 1-            | CORD2R | 1   | 0.0  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 1.0 | +XYP |
| 2-            | +XYP   | 1.0 | 0.0  | 1.0 | 2   | 8   | 7   | 1   |     |      |
| 3-            | CQUADA | 1   | 1    | 1   | 3   | 9   | 8   | 1   |     |      |
| 4-            | CQUADA | 2   | 1    | 2   | 4   | 10  | 9   | 1   |     |      |
| 5-            | CQUADA | 3   | 1    | 3   | 5   | 11  | 10  | 1   |     |      |
| 6-            | CQUADA | 4   | 1    | 4   | 6   | 12  | 11  | 1   |     |      |
| 7-            | CQUADA | 5   | 1    | 5   | 7   | 13  | 12  | 1   |     |      |
| 8-            | CQUADA | 6   | 1    | 6   | 8   | 14  | 13  | 1   |     |      |
| 9-            | CQUADA | 7   | 1    | 7   | 9   | 15  | 14  | 1   |     |      |
| 10-           | CQUADA | 8   | 1    | 8   | 10  | 16  | 15  | 1   |     |      |
| 11-           | CQUADA | 9   | 1    | 9   | 11  | 17  | 16  | 1   |     |      |
| 12-           | CQUADA | 10  | 1    | 10  | 12  | 18  | 17  | 1   |     |      |
| 13-           | CQUADA | 11  | 1    | 11  | 13  | 19  | 18  | 1   |     |      |
| 14-           | CQUADA | 12  | 1    | 12  | 14  | 20  | 19  | 1   |     |      |
| 15-           | CQUADA | 13  | 1    | 13  | 15  | 21  | 20  | 1   |     |      |
| 16-           | CQUADA | 14  | 1    | 14  | 16  | 22  | 21  | 1   |     |      |
| 17-           | CQUADA | 15  | 1    | 15  | 17  | 23  | 22  | 1   |     |      |
| 18-           | CQUADA | 16  | 1    | 16  | 18  | 24  | 23  | 1   |     |      |
| 19-           | CQUADA | 17  | 1    | 17  | 19  | 25  | 24  | 1   |     |      |
| 20-           | CQUADA | 18  | 1    | 18  | 20  | 26  | 25  | 1   |     |      |
| 21-           | CQUADA | 19  | 1    | 19  | 21  | 27  | 26  | 1   |     |      |
| 22-           | CQUADA | 20  | 1    | 20  | 22  | 28  | 27  | 1   |     |      |
| 23-           | CQUADA | 21  | 1    | 21  | 23  | 29  | 28  | 1   |     |      |
| 24-           | CQUADA | 22  | 1    | 22  | 24  | 30  | 29  | 1   |     |      |
| 25-           | CQUADA | 23  | 1    | 23  | 25  | 31  | 30  | 1   |     |      |
| 26-           | CQUADA | 24  | 1    | 24  | 26  | 32  | 31  | 1   |     |      |
| 27-           | CQUADA | 25  | 1    | 25  | 27  | 33  | 32  | 1   |     |      |
| 28-           | CQUADA | 26  | 1    | 26  | 28  | 34  | 33  | 1   |     |      |
| 29-           | CQUADA | 27  | 1    | 27  | 29  | 35  | 34  | 1   |     |      |
| 30-           | CQUADA | 28  | 1    | 28  | 30  | 36  | 35  | 1   |     |      |
| 31-           | GRID   | 1   | 0.0  | 0.0 | 0.0 | 0.0 |     |     |     |      |
| 32-           | GRID   | 2   | 3.0  | 0.0 | 0.0 | 0.0 |     |     |     |      |
| 33-           | GRID   | 3   | 6.0  | 0.0 | 0.0 | 0.0 |     |     |     |      |
| 34-           | GRID   | 4   | 9.0  | 0.0 | 0.0 | 0.0 |     |     |     |      |
| 35-           | GRID   | 5   | 12.0 | 0.0 | 0.0 | 0.0 |     |     |     |      |

[illegible]



# DISPLACEMENT VECTOR

| POINT ID. | TYPE | T1  | T2  | T3            | R1            | R2            | R3  |
|-----------|------|-----|-----|---------------|---------------|---------------|-----|
| 1         | G    | 0.0 | 0.0 | 0.0           | -1.659534E-03 | 1.630020E-03  | 0.0 |
| 2         | G    | 0.0 | 0.0 | 0.0           | -1.568235E-02 | -2.224436E-04 | 0.0 |
| 3         | G    | 0.0 | 0.0 | 0.0           | -2.187067E-02 | 3.227942E-05  | 0.0 |
| 4         | G    | 0.0 | 0.0 | 0.0           | -1.995040E-02 | -5.664070E-04 | 0.0 |
| 5         | G    | 0.0 | 0.0 | 0.0           | -1.137580E-02 | -9.744150E-04 | 0.0 |
| 6         | G    | 0.0 | 0.0 | 0.0           | 3.201080E-04  | 5.167378E-04  | 0.0 |
| 7         | G    | 0.0 | 0.0 | 0.0           | 1.627950E-04  | 2.09241E-02   | 0.0 |
| 8         | G    | 0.0 | 0.0 | -5.535023E-02 | -9.825311E-03 | 1.395945E-02  | 0.0 |
| 9         | G    | 0.0 | 0.0 | -8.360141E-02 | -1.605320E-02 | 3.755062E-03  | 0.0 |
| 10        | G    | 0.0 | 0.0 | -8.008110E-02 | -1.632954E-02 | -6.32517E-03  | 0.0 |
| 11        | G    | 0.0 | 0.0 | -4.817439E-02 | -1.057774E-02 | -1.449008E-02 | 0.0 |
| 12        | G    | 0.0 | 0.0 | 0.0           | -1.202052E-03 | -1.558065E-02 | 0.0 |
| 13        | G    | 0.0 | 0.0 | 0.0           | -4.916388E-05 | 2.914780E-02  | 0.0 |
| 14        | G    | 0.0 | 0.0 | -8.209246E-02 | -2.443614E-03 | 2.221869E-02  | 0.0 |
| 15        | G    | 0.0 | 0.0 | -1.268863E-01 | -5.031536E-03 | 7.453628E-03  | 0.0 |
| 16        | G    | 0.0 | 0.0 | -1.272077E-01 | -5.982229E-03 | -8.563215E-03 | 0.0 |
| 17        | G    | 0.0 | 0.0 | -7.872557E-02 | -4.491568E-03 | -2.23347E-02  | 0.0 |
| 18        | G    | 0.0 | 0.0 | 0.0           | -6.876041E-04 | -2.682067E-02 | 0.0 |
| 19        | G    | 0.0 | 0.0 | 0.0           | 6.876041E-04  | 2.682067E-02  | 0.0 |
| 20        | G    | 0.0 | 0.0 | -7.872557E-02 | -4.491568E-03 | 2.23347E-02   | 0.0 |
| 21        | G    | 0.0 | 0.0 | -1.272077E-01 | -5.982229E-03 | 8.563215E-03  | 0.0 |
| 22        | G    | 0.0 | 0.0 | -1.268863E-01 | -5.031536E-03 | -7.453628E-03 | 0.0 |
| 23        | G    | 0.0 | 0.0 | -8.209246E-02 | -2.443614E-03 | -2.221869E-02 | 0.0 |
| 24        | G    | 0.0 | 0.0 | 0.0           | -4.916388E-05 | -2.914780E-02 | 0.0 |
| 25        | G    | 0.0 | 0.0 | 0.0           | 1.202066E-03  | 1.558065E-02  | 0.0 |
| 26        | G    | 0.0 | 0.0 | -4.817439E-02 | -1.067774E-02 | 1.449008E-02  | 0.0 |
| 27        | G    | 0.0 | 0.0 | -8.008110E-02 | -1.632954E-02 | 6.32517E-03   | 0.0 |
| 28        | G    | 0.0 | 0.0 | -8.360141E-02 | -1.605320E-02 | -3.755062E-03 | 0.0 |
| 29        | G    | 0.0 | 0.0 | -5.535023E-02 | 9.825311E-03  | -1.395945E-02 | 0.0 |
| 30        | G    | 0.0 | 0.0 | 0.0           | -1.627950E-04 | -2.09241E-02  | 0.0 |
| 31        | G    | 0.0 | 0.0 | 0.0           | -3.201080E-04 | -5.167378E-04 | 0.0 |
| 32        | G    | 0.0 | 0.0 | 0.0           | 1.137580E-02  | 9.744150E-04  | 0.0 |
| 33        | G    | 0.0 | 0.0 | 0.0           | 1.995040E-02  | 5.664070E-04  | 0.0 |
| 34        | G    | 0.0 | 0.0 | 0.0           | 2.187067E-02  | -3.227942E-05 | 0.0 |
| 35        | G    | 0.0 | 0.0 | 0.0           | 1.568235E-02  | 2.294436E-04  | 0.0 |
| 36        | G    | 0.0 | 0.0 | 0.0           | 1.659534E-03  | -1.630020E-03 | 0.0 |

\* \* \* END OF JOB \* \* \*



[illegible]

# REAL EIGENVALUES

| MODE NO. | EXTRACTION ORDER | EIGENVALUE   | RADIAN FREQUENCY | CYCLIC FREQUENCY | GENERALIZED MASS | GENERALIZED STIFFNESS |
|----------|------------------|--------------|------------------|------------------|------------------|-----------------------|
| 1        | 1                | 2.206153E+04 | 1.485312E+02     | 2.363948E+01     | 8.482909E-02     | 1.871459E+03          |
| 2        | 2                | 9.134000E+04 | 3.022251E+02     | 4.810061E+01     | 8.233906E-02     | 7.520849E+03          |
| 3        | 3                | 1.469552E+05 | 3.833473E+02     | 6.101162E+01     | 7.591807E-02     | 1.130351E+04          |
| 4        | 4                | 2.678455E+05 | 5.175388E+02     | 8.236885E+01     | 6.337978E-02     | 1.697605E+04          |
| 5        | 5                | 3.130239E+05 | 5.595389E+02     | 8.905338E+01     | 6.320361E-02     | 1.978803E+04          |
| 6        | 6                | 5.373900E+05 | 7.330688E+02     | 1.166715E+02     | 5.950838E-02     | 3.197921E+04          |

\*\*\* END OF JOB \*\*\*

## NASTRAN EXECUTIVE CONTROL DECK ECHO

ID COMPOSITE PLATE 15 BY 20 NASTRAN USERS COLLOQUIUM 1990  
 APP DISPLACEMENT  
 SOL 3  
 TIME 10  
 CEND

## CASE CONTROL DECK ECHO

CARD  
COUNT

METHOD = 1  
 SPC = 1  
 OUTPUT  
 DISPLACEMENTS(PRINT,PUNCH)=ALL  
 BEGIN BULK

## SORTED BULK DATA ECHO

| CARD<br>COUNT | 1      | 2   | 3   | 4   | 5    | 6   | 7   | 8   | 9   | 10   |
|---------------|--------|-----|-----|-----|------|-----|-----|-----|-----|------|
| 1-            | CORD2R | 1   | 0.0 | 0.0 | 0.0  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0  |
| 2-            | +ABC   | 1.0 | 0.0 | 1.0 | 2    | 3   | 7   | 1   |     |      |
| 3-            | CQUAD4 | 1   | 1   | 1   | 3    | 9   | 8   | 1   |     |      |
| 4-            | CQUAD4 | 2   | 1   | 2   | 4    | 10  | 9   | 1   |     |      |
| 5-            | CQUAD4 | 3   | 1   | 3   | 5    | 11  | 10  | 1   |     |      |
| 6-            | CQUAD4 | 4   | 1   | 4   | 6    | 12  | 11  | 1   |     |      |
| 7-            | CQUAD4 | 5   | 1   | 5   | 7    | 13  | 12  | 1   |     |      |
| 8-            | CQUAD4 | 6   | 1   | 6   | 8    | 14  | 13  | 1   |     |      |
| 9-            | CQUAD4 | 7   | 1   | 7   | 9    | 15  | 14  | 1   |     |      |
| 10-           | CQUAD4 | 8   | 1   | 8   | 10   | 16  | 15  | 1   |     |      |
| 11-           | CQUAD4 | 9   | 1   | 9   | 11   | 17  | 16  | 1   |     |      |
| 12-           | CQUAD4 | 10  | 1   | 10  | 12   | 18  | 17  | 1   |     |      |
| 13-           | CQUAD4 | 11  | 1   | 11  | 13   | 19  | 18  | 1   |     |      |
| 14-           | CQUAD4 | 12  | 1   | 12  | 14   | 20  | 19  | 1   |     |      |
| 15-           | CQUAD4 | 13  | 1   | 13  | 15   | 21  | 20  | 1   |     |      |
| 16-           | CQUAD4 | 14  | 1   | 14  | 16   | 22  | 21  | 1   |     |      |
| 17-           | CQUAD4 | 15  | 1   | 15  | 17   | 23  | 22  | 1   |     |      |
| 18-           | CQUAD4 | 16  | 1   | 16  | 18   | 24  | 23  | 1   |     |      |
| 19-           | CQUAD4 | 17  | 1   | 17  | 19   | 25  | 24  | 1   |     |      |
| 20-           | CQUAD4 | 18  | 1   | 18  | 20   | 26  | 25  | 1   |     |      |
| 21-           | CQUAD4 | 19  | 1   | 19  | 21   | 27  | 26  | 1   |     |      |
| 22-           | CQUAD4 | 20  | 1   | 20  | 22   | 28  | 27  | 1   |     |      |
| 23-           | CQUAD4 | 21  | 1   | 21  | 23   | 29  | 28  | 1   |     |      |
| 24-           | CQUAD4 | 22  | 1   | 22  | 24   | 30  | 29  | 1   |     |      |
| 25-           | CQUAD4 | 23  | 1   | 23  | 25   | 31  | 30  | 1   |     |      |
| 26-           | CQUAD4 | 24  | 1   | 24  | 26   | 32  | 31  | 1   |     |      |
| 27-           | CQUAD4 | 25  | 1   | 25  | 27   | 33  | 32  | 1   |     |      |
| 28-           | CQUAD4 | 26  | 1   | 26  | 28   | 34  | 33  | 1   |     |      |
| 29-           | CQUAD4 | 27  | 1   | 27  | 29   | 35  | 34  | 1   |     |      |
| 30-           | CQUAD4 | 28  | 1   | 28  | 30   | 36  | 35  | 1   |     |      |
| 31-           | CQUAD4 | 29  | 1   | 29  | 31   | 37  | 36  | 1   |     |      |
| 32-           | CQUAD4 | 30  | 1   | 30  | 32   | 38  | 37  | 1   |     |      |
| 33-           | CQUAD4 | 31  | 1   | 31  | 33   | 39  | 38  | 1   |     |      |
|               | ETGR   | 1   | INV | 0.0 | 100. | 20  | 6   |     |     |      |
|               | +WXY   | MAX |     |     |      |     |     |     |     | +WXY |
|               | GRID   | 1   |     | 0.0 | 0.0  | 0.0 |     |     |     |      |
|               | GRID   | 2   |     | 3.0 | 0.0  | 0.0 |     |     |     |      |
|               | GRID   | 3   |     | 6.0 | 0.0  | 0.0 |     |     |     |      |
|               | GRID   | 4   |     | 9.0 | 0.0  | 0.0 |     |     |     |      |



| MODE<br>NO. | EXTRACTION<br>ORDER | EIGENVALUE   | R E A L   E I G E N V A L U E S<br>RADIAN'S | CYCLES       | GENERALIZED<br>MASS | GENERALIZED<br>STIFFNESS |
|-------------|---------------------|--------------|---|--------------|---------------------|--------------------------|
| 1           | 1                   | 2.232679E+04 | 1.494215E+02                                | 2.378117E+01 | 8.489105E-02        | 1.895345E+03             |
| 2           | 2                   | 9.354261E+04 | 3.058474E+02                                | 4.867712E+01 | 8.149919E-02        | 7.523547E+03             |
| 3           | 3                   | 1.493206E+05 | 3.864202E+02                                | 6.150068E+01 | 7.644512E-02        | 1.141483E+04             |
| 4           | 4                   | 2.815449E+05 | 5.306080E+02                                | 8.444889E+01 | 6.521134E-02        | 1.835992E+04             |
| 5           | 5                   | 3.217625E+05 | 5.672411E+02                                | 9.027923E+01 | 6.557091E-02        | 2.109826E+04             |
| 6           | 6                   | 5.805212E+05 | 7.619194E+02                                | 1.212632E+02 | 7.699291E-02        | 4.469601E+04             |

\* \* \* END OF JOB \* \* \*

## NASTRAN EXECUTIVE CONTROL DECK FCHO

ID COMPOSITE PLATE 15 BY 20 NASTRAN USERS COLLOQUIUM 1990  
 APP DISPLACEMENT  
 SOL 1,0  
 TIME 10  
 CEND

## CASE CONTROL DECK ECHO

TITLE = PROBLEM 14 SANDWICH PLATE COMPOSITE FACE SHEETS  
 LOAD = 1  
 SPC = 1  
 OUTPUT  
 OLOAD=ALL  
 STRESS=ALL  
 DISPLACEMENTS=ALL  
 STRAIN=ALL  
 BEGIN BULK

| CARD<br>COUNT | 1-<br>--- | 2-<br>--- | 3-<br>--- | 4-<br>--- | 5-<br>--- | 6-<br>--- | 7-<br>--- | 8-<br>--- | 9-<br>--- | 10-<br>--- |
|---------------|-----------|-----------|-----------|-----------|-----------|-----------|-----------|-----------|-----------|------------|
| 1             | CORD2R    | 1         | 1.0       | 0.0       | 0.0       | 0.0       | 0.0       | 0.0       | 0.0       | 0.0        |
| 2             | +XYP      | 1         | 0.0       | 0.0       | 0.0       | 0.0       | 0.0       | 0.0       | 0.0       | 0.0        |
| 3             | CQUAD4    | 1         | 72        | 72        | 72        | 72        | 72        | 72        | 72        | 72         |
| 4             | CQUAD4    | 2         | 72        | 72        | 72        | 72        | 72        | 72        | 72        | 72         |
| 5             | CQUAD4    | 3         | 72        | 72        | 72        | 72        | 72        | 72        | 72        | 72         |
| 6             | CQUAD4    | 4         | 72        | 72        | 72        | 72        | 72        | 72        | 72        | 72         |
| 7             | CQUAD4    | 5         | 72        | 72        | 72        | 72        | 72        | 72        | 72        | 72         |
| 8             | CQUAD4    | 6         | 72        | 72        | 72        | 72        | 72        | 72        | 72        | 72         |
| 9             | CQUAD4    | 7         | 72        | 72        | 72        | 72        | 72        | 72        | 72        | 72         |
| 10            | CQUAD4    | 8         | 72        | 72        | 72        | 72        | 72        | 72        | 72        | 72         |
| 11            | CQUAD4    | 9         | 72        | 72        | 72        | 72        | 72        | 72        | 72        | 72         |
| 12            | CQUAD4    | 10        | 72        | 72        | 72        | 72        | 72        | 72        | 72        | 72         |
| 13            | CQUAD4    | 11        | 72        | 72        | 72        | 72        | 72        | 72        | 72        | 72         |
| 14            | CQUAD4    | 12        | 72        | 72        | 72        | 72        | 72        | 72        | 72        | 72         |
| 15            | CQUAD4    | 13        | 72        | 72        | 72        | 72        | 72        | 72        | 72        | 72         |
| 16            | CQUAD4    | 14        | 72        | 72        | 72        | 72        | 72        | 72        | 72        | 72         |
| 17            | CQUAD4    | 15        | 72        | 72        | 72        | 72        | 72        | 72        | 72        | 72         |
| 18            | CQUAD4    | 16        | 72        | 72        | 72        | 72        | 72        | 72        | 72        | 72         |
| 19            | CQUAD4    | 17        | 72        | 72        | 72        | 72        | 72        | 72        | 72        | 72         |
| 20            | CQUAD4    | 18        | 72        | 72        | 72        | 72        | 72        | 72        | 72        | 72         |
| 21            | CQUAD4    | 19        | 72        | 72        | 72        | 72        | 72        | 72        | 72        | 72         |
| 22            | CQUAD4    | 20        | 72        | 72        | 72        | 72        | 72        | 72        | 72        | 72         |
| 23            | CQUAD4    | 21        | 72        | 72        | 72        | 72        | 72        | 72        | 72        | 72         |
| 24            | CQUAD4    | 22        | 72        | 72        | 72        | 72        | 72        | 72        | 72        | 72         |
| 25            | CQUAD4    | 23        | 72        | 72        | 72        | 72        | 72        | 72        | 72        | 72         |
| 26            | CQUAD4    | 24        | 72        | 72        | 72        | 72        | 72        | 72        | 72        | 72         |
| 27            | CQUAD4    | 25        | 72        | 72        | 72        | 72        | 72        | 72        | 72        | 72         |
| 28            | GRID      | 1         | 0.0       | 0.0       | 0.0       | 0.0       | 0.0       | 0.0       | 0.0       | 0.0        |
| 29            | GRID      | 2         | 3.0       | 0.0       | 0.0       | 0.0       | 0.0       | 0.0       | 0.0       | 0.0        |
| 30            | GRID      | 3         | 6.0       | 0.0       | 0.0       | 0.0       | 0.0       | 0.0       | 0.0       | 0.0        |





# DISPLACEMENT VECTOR

| POINT ID. | TYPE | T1            | T2            | T3            | R1            | R2            | R3  |
|-----------|------|---------------|---------------|---------------|---------------|---------------|-----|
| 1         | G    | 0.0           | -1.262361E-21 | 0.0           | -1.143164E-04 | 7.499255E-05  | 0.0 |
| 2         | G    | -4.790143E-22 | -3.355112E-21 | 0.0           | -1.790529E-04 | 7.432398E-05  | 0.0 |
| 3         | G    | -2.955672E-22 | -4.562165E-21 | 0.0           | -2.559781E-04 | 4.713882E-05  | 0.0 |
| 4         | G    | -1.264729E-22 | -3.982578E-21 | 0.0           | -2.455365E-04 | -1.063998E-05 | 0.0 |
| 5         | G    | -3.145644E-22 | -1.929802E-21 | 0.0           | -1.535654E-04 | -5.459381E-05 | 0.0 |
| 6         | G    | -6.515719E-22 | -4.187489E-21 | 0.0           | -9.322596E-05 | -6.597897E-05 | 0.0 |
| 7         | G    | 0.0           | -1.765623E-21 | 0.0           | -1.063590E-04 | 2.292916E-04  | 0.0 |
| 8         | G    | 1.332215E-21  | -3.720921E-21 | -9.309109E-04 | -1.386407E-04 | 1.791623E-04  | 0.0 |
| 9         | G    | 1.130583E-21  | -4.489881E-21 | -1.361827E-03 | -1.983330E-04 | 6.680239E-05  | 0.0 |
| 10        | G    | 6.925907E-23  | -3.550335E-21 | -1.314377E-03 | -1.916281E-04 | -5.423765E-05 | 0.0 |
| 11        | G    | -1.060839E-21 | -1.285317E-21 | -8.131079E-04 | -1.148891E-04 | -1.635653E-04 | 0.0 |
| 12        | G    | -1.756958E-21 | 8.511079E-22  | 0.0           | -7.354622E-05 | -2.062164E-04 | 0.0 |
| 13        | G    | 0.0           | -3.076516E-21 | 0.0           | -6.418965E-05 | 3.436314E-04  | 0.0 |
| 14        | G    | 9.844685E-22  | -4.408565E-21 | -1.309181E-03 | -5.804963E-05 | 2.753212E-04  | 0.0 |
| 15        | G    | 2.347015E-22  | -4.176753E-21 | -2.026409E-03 | -6.767749E-05 | 9.872849E-05  | 0.0 |
| 16        | G    | -1.484854E-21 | -2.611802E-21 | -2.008277E-03 | -5.978833E-05 | -1.001152E-04 | 0.0 |
| 17        | G    | -3.134186E-21 | -2.276627E-23 | -1.272991E-03 | -2.516000E-05 | -2.728444E-04 | 0.0 |
| 18        | G    | -4.084607E-21 | 3.144208E-21  | 0.0           | -1.208915E-05 | -3.363470E-04 | 0.0 |
| 19        | G    | 0.0           | -3.969974E-21 | 0.0           | 1.208915E-05  | 3.363470E-04  | 0.0 |
| 20        | G    | -3.581775E-22 | -4.373653E-21 | -1.272991E-03 | 2.516000E-05  | 2.728444E-04  | 0.0 |
| 21        | G    | -2.106983E-21 | -3.416818E-21 | -2.008277E-03 | 5.978833E-05  | 1.001152E-04  | 0.0 |
| 22        | G    | -4.344522E-21 | -1.542182E-21 | -2.026409E-03 | 6.767749E-05  | -9.872849E-05 | 0.0 |
| 23        | G    | -5.108684E-21 | 1.052242E-21  | -1.309181E-03 | 5.804963E-05  | -2.753212E-04 | 0.0 |
| 24        | G    | -6.847433E-21 | 4.573085E-21  | 0.0           | 6.418965E-05  | -3.436314E-04 | 0.0 |
| 25        | G    | 0.0           | -3.377656E-21 | 0.0           | 7.354622E-05  | 2.062164E-04  | 0.0 |
| 26        | G    | -2.382802E-21 | -2.936029E-21 | -8.131079E-04 | 1.148891E-04  | 1.635653E-04  | 0.0 |
| 27        | G    | -5.172902E-21 | -1.979701E-21 | -1.314377E-03 | 1.916281E-04  | 5.423765E-05  | 0.0 |
| 28        | G    | -7.664944E-21 | -5.526538E-22 | -1.361827E-03 | 1.983330E-04  | -6.680239E-05 | 0.0 |
| 29        | G    | -9.268735E-21 | 1.075791E-21  | -9.309109E-04 | 1.386407E-04  | -1.791623E-04 | 0.0 |
| 30        | G    | -9.495322E-21 | 3.923865E-21  | 0.0           | 1.063590E-04  | -2.292916E-04 | 0.0 |
| 31        | G    | 0.0           | 0.0           | 0.0           | 9.322596E-05  | 6.597897E-05  | 0.0 |
| 32        | G    | -4.600099E-21 | 0.0           | 0.0           | 1.535654E-04  | 5.459381E-05  | 0.0 |
| 33        | G    | -8.168351E-21 | 0.0           | 0.0           | 2.455365E-04  | 1.063998E-05  | 0.0 |
| 34        | G    | -1.056813E-20 | 0.0           | 0.0           | 2.559781E-04  | -4.713882E-05 | 0.0 |
| 35        | G    | -1.165361E-20 | 0.0           | 0.0           | 1.790529E-04  | -7.432398E-05 | 0.0 |
| 36        | G    | -1.132367E-20 | 0.0           | 0.0           | 1.143164E-04  | -7.499255E-05 | 0.0 |

\* \* \* END OF JOB \* \* \*

NASTRAN EXECUTIVE CONTROL DECK ECHO

IG COMPOSITE PLATE 15 BY 20 NASTRAN USERS COLLOQUIUM 1990  
 APP DISPLACEMENT  
 SOL 1  
 TIME 10  
 CEND

CASE CONTROL DECK ECHO

TITLE = PROBLEM 14 SANDWICH PLATE COMPOSITE FACE SHEETS

LOAD = 1

SPC = 1

OUTPUT

LOAD=ALL

STRESS=ALL

DISPLACEMENTS=ALL

STRAIN=ALL

BEGIN BULK

INPUT BULK DATA CARD COUNT = 78

SORTED BULK DATA ECHO

| CARD<br>COUNT | 1      | 2   | 3   | 4   | 5   | 6   | 7   | 8   | 9   | 10  |
|---------------|--------|-----|-----|-----|-----|-----|-----|-----|-----|-----|
| 1-            | CARD2R | 1   | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 1.0 | 1.0 |
| 2-            | +XYP   | 1.0 | 0.0 | 1.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 3-            | QUAD4  | 1   | 72  | 2   | 3   | 8   | 7   | 1   | 1   | 1   |
| 4-            | QUAD4  | 2   | 72  | 4   | 4   | 9   | 8   | 1   | 1   | 1   |
| 5-            | QUAD4  | 3   | 72  | 5   | 5   | 10  | 9   | 1   | 1   | 1   |
| 6-            | QUAD4  | 4   | 72  | 6   | 6   | 11  | 10  | 1   | 1   | 1   |
| 7-            | QUAD4  | 5   | 72  | 7   | 7   | 12  | 11  | 1   | 1   | 1   |
| 8-            | QUAD4  | 6   | 72  | 8   | 8   | 13  | 12  | 1   | 1   | 1   |
| 9-            | QUAD4  | 7   | 72  | 9   | 9   | 14  | 13  | 1   | 1   | 1   |
| 10-           | QUAD4  | 8   | 72  | 10  | 10  | 15  | 14  | 1   | 1   | 1   |
| 11-           | QUAD4  | 9   | 72  | 11  | 11  | 16  | 15  | 1   | 1   | 1   |
| 12-           | QUAD4  | 10  | 72  | 12  | 12  | 17  | 16  | 1   | 1   | 1   |
| 13-           | QUAD4  | 11  | 72  | 13  | 13  | 18  | 17  | 1   | 1   | 1   |
| 14-           | QUAD4  | 12  | 72  | 14  | 14  | 19  | 18  | 1   | 1   | 1   |
| 15-           | QUAD4  | 13  | 72  | 15  | 15  | 20  | 19  | 1   | 1   | 1   |
| 16-           | QUAD4  | 14  | 72  | 16  | 16  | 21  | 20  | 1   | 1   | 1   |
| 17-           | QUAD4  | 15  | 72  | 17  | 17  | 22  | 21  | 1   | 1   | 1   |
| 18-           | QUAD4  | 16  | 72  | 18  | 18  | 23  | 22  | 1   | 1   | 1   |
| 19-           | QUAD4  | 17  | 72  | 19  | 19  | 24  | 23  | 1   | 1   | 1   |
| 20-           | QUAD4  | 18  | 72  | 20  | 20  | 25  | 24  | 1   | 1   | 1   |
| 21-           | QUAD4  | 19  | 72  | 21  | 21  | 26  | 25  | 1   | 1   | 1   |
| 22-           | QUAD4  | 20  | 72  | 22  | 22  | 27  | 26  | 1   | 1   | 1   |
| 23-           | QUAD4  | 21  | 72  | 23  | 23  | 28  | 27  | 1   | 1   | 1   |
| 24-           | QUAD4  | 22  | 72  | 24  | 24  | 29  | 28  | 1   | 1   | 1   |
| 25-           | QUAD4  | 23  | 72  | 25  | 25  | 30  | 29  | 1   | 1   | 1   |
| 26-           | QUAD4  | 24  | 72  | 26  | 26  | 31  | 30  | 1   | 1   | 1   |
| 27-           | QUAD4  | 25  | 72  | 27  | 27  | 32  | 31  | 1   | 1   | 1   |
| 28-           | GRID   | 1   | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |

233

| POINT ID. | TYPE | T1  | T2  | T3            | DISPLACEMENT VECTOR |               |     | R1 | R2 | R3 |
|-----------|------|-----|-----|---------------|---------------------|---------------|-----|----|----|----|
| 1         | G    | 0.0 | 0.0 | 0.0           | -1.571077E-04       | 1.142508E-04  | 0.0 |    |    |    |
| 2         | G    | 0.0 | 0.0 | 0.0           | -1.933766E-04       | 9.752589E-05  | 0.0 |    |    |    |
| 3         | G    | 0.0 | 0.0 | 0.0           | -2.608906E-04       | 3.743675E-05  | 0.0 |    |    |    |
| 4         | G    | 0.0 | 0.0 | 0.0           | -2.607035E-04       | -4.376641E-05 | 0.0 |    |    |    |
| 5         | G    | 0.0 | 0.0 | 0.0           | -1.934224E-04       | -1.008716E-04 | 0.0 |    |    |    |
| 6         | G    | 0.0 | 0.0 | 0.0           | -1.578756E-04       | -1.151408E-04 | 0.0 |    |    |    |
| 7         | G    | 0.0 | 0.0 | 0.0           | -1.300261E-04       | 2.268750E-04  | 0.0 |    |    |    |
| 8         | G    | 0.0 | 0.0 | -1.290094E-03 | -1.522363E-04       | 1.820725E-04  | 0.0 |    |    |    |
| 9         | G    | 0.0 | 0.0 | -1.923577E-03 | -2.075847E-04       | 6.413873E-05  | 0.0 |    |    |    |
| 10        | G    | 0.0 | 0.0 | -1.920106E-03 | -2.083385E-04       | -6.804503E-05 | 0.0 |    |    |    |
| 11        | G    | 0.0 | 0.0 | -1.283585E-03 | -1.548241E-04       | -1.836399E-04 | 0.0 |    |    |    |
| 12        | G    | 0.0 | 0.0 | 0.0           | -1.342848E-04       | -2.265799E-04 | 0.0 |    |    |    |
| 13        | G    | 0.0 | 0.0 | 0.0           | -4.759642E-05       | 3.373894E-04  | 0.0 |    |    |    |
| 14        | G    | 0.0 | 0.0 | -1.845502E-03 | -5.010014E-05       | 2.752919E-04  | 0.0 |    |    |    |
| 15        | G    | 0.0 | 0.0 | -2.815118E-03 | -6.906350E-05       | 1.000706E-04  | 0.0 |    |    |    |
| 16        | G    | 0.0 | 0.0 | -2.813420E-03 | -7.083838E-05       | -1.013518E-04 | 0.0 |    |    |    |
| 17        | G    | 0.0 | 0.0 | -1.842411E-03 | -5.519034E-05       | -2.755732E-04 | 0.0 |    |    |    |
| 18        | G    | 0.0 | 0.0 | 0.0           | -5.532292E-05       | -3.366605E-04 | 0.0 |    |    |    |
| 19        | G    | 0.0 | 0.0 | 0.0           | 5.532292E-05        | 3.366605E-04  | 0.0 |    |    |    |
| 20        | G    | 0.0 | 0.0 | -1.842411E-03 | 5.519034E-05        | 2.755732E-04  | 0.0 |    |    |    |
| 21        | G    | 0.0 | 0.0 | -2.813420E-03 | 7.083838E-05        | 1.013518E-04  | 0.0 |    |    |    |
| 22        | G    | 0.0 | 0.0 | -2.815118E-03 | 6.906350E-05        | -1.000706E-04 | 0.0 |    |    |    |
| 23        | G    | 0.0 | 0.0 | -1.845502E-03 | 5.010014E-05        | -2.752919E-04 | 0.0 |    |    |    |
| 24        | G    | 0.0 | 0.0 | 0.0           | 4.759642E-05        | -3.370894E-04 | 0.0 |    |    |    |
| 25        | G    | 0.0 | 0.0 | 0.0           | 1.342848E-04        | 2.265799E-04  | 0.0 |    |    |    |
| 26        | G    | 0.0 | 0.0 | -1.283585E-03 | 1.548241E-04        | 1.836399E-04  | 0.0 |    |    |    |
| 27        | G    | 0.0 | 0.0 | -1.920106E-03 | 2.083385E-04        | 6.804503E-05  | 0.0 |    |    |    |
| 28        | G    | 0.0 | 0.0 | -1.923577E-03 | 2.075847E-04        | -6.413873E-05 | 0.0 |    |    |    |
| 29        | G    | 0.0 | 0.0 | -1.290094E-03 | 1.522363E-04        | -1.820725E-04 | 0.0 |    |    |    |
| 30        | G    | 0.0 | 0.0 | 0.0           | 1.300261E-04        | -2.268750E-04 | 0.0 |    |    |    |
| 31        | G    | 0.0 | 0.0 | 0.0           | 1.578756E-04        | 1.151408E-04  | 0.0 |    |    |    |
| 32        | G    | 0.0 | 0.0 | 0.0           | 1.934224E-04        | 1.008716E-04  | 0.0 |    |    |    |
| 33        | G    | 0.0 | 0.0 | 0.0           | 2.607035E-04        | 4.376641E-05  | 0.0 |    |    |    |
| 34        | G    | 0.0 | 0.0 | 0.0           | 2.608906E-04        | -3.743675E-05 | 0.0 |    |    |    |
| 35        | G    | 0.0 | 0.0 | 0.0           | 1.933766E-04        | -9.752589E-05 | 0.0 |    |    |    |
| 36        | G    | 0.0 | 0.0 | 0.0           | 1.571077E-04        | -1.142508E-04 | 0.0 |    |    |    |

\* \* \* END OF JOB \* \* \*

# PROBLEM #9

## REGULAR SYMMETRIC CROSS-PLY LAMINATE

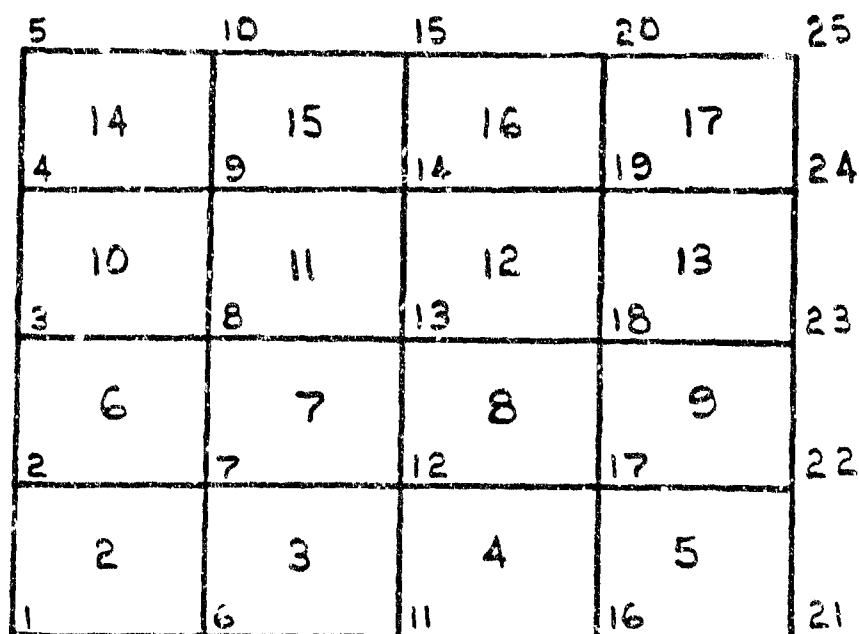


FIGURE 9

A quarter model of a composite square plate modeled as symmetric cross-ply laminate is shown in Fig. 9. The length of each side is 1.0, and the thickness of each ply is .000666. The material properties are given as  $E_1 = 2.0 \times 10^7$ ,  $E_2 = 5.0 \times 10^5$ ,  $\nu_{12} = 0.25$  and  $G_{12} = 2.5 \times 10^5$  and are the same for each ply. The full plate is simply supported and is subjected to a uniform pressure load of  $-1.0 \times 10^{-4}$ . The finite element model contains 25 nodes and 16 elements. The theoretical solution for the Z deflection at Grid 25 (center of the plate) is  $-1.836 \times 10^{-3}$ . Theoretical results are also given for the stresses in layers 1 and 3 in element 17 in Table 4.

| $\sigma_1$ | $\sigma_2$ | $\tau_{12}$ |
|------------|------------|-------------|
| 58.6       | 1.8        | -.06        |

TABLE 4

**PROBLEM 9 REGULAR SYMMETRIC  
CROSS-PLY LAMINATE MODELED WITH  
QUAD4  
ELEMENTS**

# COSMIC INPUT

[illegible]

PROBLEM 9 REGULAR SYMMETRIC  
CROSS-PLY LAMINATE MODELED WITH

QAD4

# ELEMENTS

## COSMIC INPUT (CONTD)

| GRID | 15     | -500        | 1.000   | 0.000     | +PC1 | +PC2 |
|------|--------|-------------|---------|-----------|------|------|
| 31-  | 15     | -500        | 1.000   | 0.000     |      |      |
| 32-  | 16     | .750        | 0.900   | 0.000     |      |      |
| 33-  | 17     | .750        | .250    | 0.000     |      |      |
| 34-  | 18     | .750        | .500    | 0.000     |      |      |
| 35-  | 19     | .750        | .750    | 0.000     |      |      |
| 36-  | 20     | .750        | 1.000   | 0.000     |      |      |
| 37-  | 21     | 1.000       | 0.000   | 0.000     |      |      |
| 38-  | 22     | 1.000       | .250    | 0.000     |      |      |
| 39-  | 23     | 1.000       | .500    | 0.000     |      |      |
| 40-  | 24     | 1.000       | .750    | 0.000     |      |      |
| 41-  | 25     | 1.000       | 1.000   | 0.000     |      |      |
| 42-  | 1      | 20.0E+06.50 | E+.25   | .250 E+.6 |      |      |
| 43-  | PAPAM  |             |         |           |      |      |
| 44-  | PCOMP  | 1           | - .001  |           |      |      |
| 45-  | +PC1   | 1           | .000666 | 0.0       | YES  |      |
| 46-  | +PC2   | 1           | .000666 | 0.0       | YES  |      |
| 47-  | PLAD4  | 1           | 2       | -1.0E-04  |      |      |
| 48-  | SPC1   | 1           | 15      | 23        | 23   | 24   |
| 49-  | SPC1   | 1           | 24      | 10        | 15   | 20   |
| 50-  | SPC1   | 1           | 1234    | 2         | 3    | 4    |
| 51-  | SPC1   | 1           | 1235    | 6         | 11   | 16   |
| 52-  | SPC1   | 1           | 1245    | 25        |      | 21   |
| 53-  | SPC1   | 1           | 12345   | 1         |      |      |
|      | END473 |             |         |           |      |      |



# PROBLEM 9 REGULAR SYMMETRIC CROSS-PLY LAMINATE MODELED WITH

## QUAD4

## ELEMENTS

## COSMIC OUTPUT

| POINT ID. | TYPE | DISPLACEMENT VECTOR |     |               |               |  |              | R2  | R3  |
|-----------|------|---------------------|-----|---------------|---------------|--|--------------|-----|-----|
|           |      | T1                  | T2  | T3            | R1            |  |              |     |     |
| 1         | G    | 0.0                 | 0.0 | 0.0           | 0.0           |  | 0.0          | 0.0 | 0.0 |
| 2         | G    | 0.0                 | 0.0 | 0.0           | 0.0           |  | 1.259793E-03 | 0.0 | 0.0 |
| 3         | G    | 0.0                 | 0.0 | 0.0           | 0.0           |  | 2.153710E-03 | 0.0 | 0.0 |
| 4         | G    | 0.0                 | 0.0 | 0.0           | 0.0           |  | 2.628845E-03 | 0.0 | 0.0 |
| 5         | G    | 0.0                 | 0.0 | 0.0           | 0.0           |  | 2.772239E-03 | 0.0 | 0.0 |
| 6         | G    | 0.0                 | 0.0 | 0.0           | -1.423199E-03 |  | 0.0          | 0.0 | 0.0 |
| 7         | G    | 0.0                 | 0.0 | -3.245146E-04 | -1.171945E-03 |  | 1.144795E-03 | 0.0 | 0.0 |
| 8         | G    | 0.0                 | 0.0 | -5.582579E-04 | -7.010531E-04 |  | 1.969839E-03 | 0.0 | 0.0 |
| 9         | G    | 0.0                 | 0.0 | -6.843077E-04 | -3.124475E-04 |  | 2.410641E-03 | 0.0 | 0.0 |
| 10        | G    | 0.0                 | 0.0 | -7.226401E-04 | 0.0           |  | 2.543014E-03 | 0.0 | 0.0 |
| 11        | G    | 0.0                 | 0.0 | 0.0           | -2.575155E-03 |  | 0.0          | 0.0 | 0.0 |
| 12        | G    | 0.0                 | 0.0 | -5.899490E-04 | -2.141997E-03 |  | 8.477152E-04 | 0.0 | 0.0 |
| 13        | G    | 0.0                 | 0.0 | -1.019073E-03 | -1.294632E-03 |  | 1.476568E-03 | 0.0 | 0.0 |
| 14        | G    | 0.0                 | 0.0 | -1.251502E-03 | -5.713850E-04 |  | 1.817122E-03 | 0.0 | 0.0 |
| 15        | G    | 0.0                 | 0.0 | -1.322001E-03 | 0.0           |  | 1.918498E-03 | 0.0 | 0.0 |
| 16        | G    | 0.0                 | 0.0 | 0.0           | -3.312100E-03 |  | 0.0          | 0.0 | 0.0 |
| 17        | G    | 0.0                 | 0.0 | -7.614346E-04 | -2.775751E-03 |  | 4.475691E-04 | 0.0 | 0.0 |
| 18        | G    | 0.0                 | 0.0 | -1.319674E-03 | -1.693370E-03 |  | 7.857787E-04 | 0.0 | 0.0 |
| 19        | G    | 0.0                 | 0.0 | -1.623366E-03 | -7.425974E-04 |  | 9.717189E-04 | 0.0 | 0.0 |
| 20        | G    | 0.0                 | 0.0 | -1.715300E-03 | 0.0           |  | 1.026845E-03 | 0.0 | 0.0 |
| 21        | G    | 0.0                 | 0.0 | 0.0           | -3.565219E-03 |  | 0.0          | 0.0 | 0.0 |
| 22        | G    | 0.0                 | 0.0 | -8.206014E-04 | -2.995503E-03 |  | 0.0          | 0.0 | 0.0 |
| 23        | G    | 0.0                 | 0.0 | -1.423963E-03 | -1.834283E-03 |  | 0.0          | 0.0 | 0.0 |
| 24        | G    | 0.0                 | 0.0 | -1.752818E-03 | -8.027037E-04 |  | 0.0          | 0.0 | 0.0 |
| 25        | G    | 0.0                 | 0.0 | -1.852310E-03 | 0.0           |  | 0.0          | 0.0 | 0.0 |

# PROBLEM 9 REGULAR SYMMETRIC CROSS-PLY LAMINATE MODELED WITH TRIA3 ELEMENTS

**COSMIC**

| CARD | COUNT  | --- | 1 | ++2++ | ---3--- | ++4++ | ---5--- | ++6++ | ---7--- | ++8++ | ---9--- | ++10++ |
|------|--------|-----|---|-------|---------|-------|---------|-------|---------|-------|---------|--------|
| 1-   | CTRIA3 | 1   | 1 |       |         | 1     | 6       | 2     |         | 45.0  |         |        |
| 2-   | CTRIA3 | 2   | 1 |       |         | 2     | 6       | 7     |         |       |         |        |
| 3-   | CTRIA3 | 3   | 1 |       |         | 6     | 11      | 7     |         | 45.0  |         |        |
| 4-   | CTRIA3 | 4   | 1 |       |         | 7     | 11      | 12    |         |       |         |        |
| 5-   | CTRIA3 | 5   | 1 |       |         | 11    | 16      | 12    |         | 45.0  |         |        |
| 6-   | CTRIA3 | 6   | 1 |       |         | 12    | 16      | 17    |         | 45.0  |         |        |
| 7-   | CTRIA3 | 7   | 1 |       |         | 16    | 21      | 17    |         |       |         |        |
| 8-   | CTRIA3 | 8   | 1 |       |         | 17    | 21      | 22    |         | 45.0  |         |        |
| 9-   | CTRIA3 | 9   | 1 |       |         | 2     | 7       | 3     |         |       |         |        |
| 10-  | CTRIA3 | 10  | 1 |       |         | 3     | 7       | 8     |         | 45.0  |         |        |
| 11-  | CTRIA3 | 11  | 1 |       |         | 7     | 12      | 8     |         |       |         |        |
| 12-  | CTRIA3 | 12  | 1 |       |         | 8     | 12      | 13    |         | 45.0  |         |        |
| 13-  | CTRIA3 | 13  | 1 |       |         | 12    | 17      | 13    |         |       |         |        |
| 14-  | CTRIA3 | 14  | 1 |       |         | 13    | 17      | 18    |         | 45.0  |         |        |
| 15-  | CTRIA3 | 15  | 1 |       |         | 17    | 22      | 18    |         |       |         |        |
| 16-  | CTRIA3 | 16  | 1 |       |         | 18    | 22      | 23    |         | 45.0  |         |        |
| 17-  | CTRIA3 | 17  | 1 |       |         | 3     | 8       | 4     |         |       |         |        |
| 18-  | CTRIA3 | 18  | 1 |       |         | 4     | 8       | 9     |         | 45.0  |         |        |
| 19-  | CTRIA3 | 19  | 1 |       |         | 8     | 13      | 9     |         |       |         |        |
| 20-  | CTRIA3 | 20  | 1 |       |         | 9     | 13      | 14    |         | 45.0  |         |        |
| 21-  | CTRIA3 | 21  | 1 |       |         | 13    | 18      | 14    |         |       |         |        |
| 22-  | CTRIA3 | 22  | 1 |       |         | 14    | 18      | 19    |         | 45.0  |         |        |
| 23-  | CTRIA3 | 23  | 1 |       |         | 18    | 23      | 19    |         |       |         |        |
| 24-  | CTRIA3 | 24  | 1 |       |         | 19    | 23      | 24    |         | 45.0  |         |        |
| 25-  | CTRIA3 | 25  | 1 |       |         | 4     | 9       | 5     |         |       |         |        |
| 26-  | CTRIA3 | 26  | 1 |       |         | 5     | 9       | 10    |         | 45.0  |         |        |
| 27-  | CTRIA3 | 27  | 1 |       |         | 9     | 14      | 10    |         |       |         |        |
| 28-  | CTRIA3 | 28  | 1 |       |         | 10    | 14      | 15    |         | 45.0  |         |        |
| 29-  | CTRIA3 | 29  | 1 |       |         | 14    | 19      | 15    |         |       |         |        |
| 30-  | CTRIA3 | 30  | 1 |       |         | 15    | 19      | 20    |         | 45.0  |         |        |
| 31-  | CTRIA3 | 31  | 1 |       |         | 19    | 24      | 20    |         |       |         |        |
| 32-  | CTRIA3 | 32  | 1 |       |         | 20    | 4       | 25    |         | 45.0  |         |        |
| 33-  | GRID   | 1   |   |       |         | 0.000 | 0.000   | 0.000 |         |       |         |        |
| 34-  | GRID   | 2   |   |       |         | 0.000 | .250    | 0.000 |         |       |         |        |
| 35-  | GRID   | 3   |   |       |         | 0.000 | .500    | 0.000 |         |       |         |        |

# PROBLEM 9 REGULAR SYMMETRIC CROSS-PLY LAMINATE MODELED WITH TRIA3 ELEMENTS

## COSMIC INPUT (CONTD)

|     |       |    |            |        |       |    |  |  |      |
|-----|-------|----|------------|--------|-------|----|--|--|------|
| 36- | GRID  | 4  | 5.000      | .750   | 0.000 |    |  |  |      |
| 37- | GRID  | 5  | 0.000      | 1.000  | 0.000 |    |  |  |      |
| 38- | GRID  | 6  | .250       | 0.000  | 0.000 |    |  |  |      |
| 39- | GRID  | 7  | .250       | .250   | 0.000 |    |  |  |      |
| 40- | GRID  | 8  | .250       | .500   | 0.000 |    |  |  |      |
| 41- | GRID  | 9  | .250       | .750   | 0.000 |    |  |  |      |
| 42- | GRID  | 10 | .250       | 1.000  | 0.000 |    |  |  |      |
| 43- | GRID  | 11 | .500       | 0.000  | 0.000 |    |  |  |      |
| 44- | GRID  | 12 | .500       | .250   | 0.000 |    |  |  |      |
| 45- | GRID  | 13 | .500       | .500   | 0.000 |    |  |  |      |
| 46- | GRID  | 14 | .500       | .750   | 0.000 |    |  |  |      |
| 47- | GRID  | 15 | .500       | 1.000  | 0.000 |    |  |  |      |
| 48- | GRID  | 16 | .750       | 0.000  | 0.000 |    |  |  |      |
| 49- | GRID  | 17 | .750       | .250   | 0.000 |    |  |  |      |
| 50- | GRID  | 18 | .750       | .500   | 0.000 |    |  |  |      |
| 51- | GRID  | 19 | .750       | .750   | 0.000 |    |  |  |      |
| 52- | GRID  | 20 | .750       | 1.000  | 0.000 |    |  |  |      |
| 53- | GRID  | 21 | 1.000      | 0.000  | 0.000 |    |  |  |      |
| 54- | GRID  | 22 | 1.000      | .250   | 0.000 |    |  |  |      |
| 55- | GRID  | 23 | 1.000      | .500   | 0.000 |    |  |  |      |
| 56- | GRID  | 24 | 1.000      | .750   | 0.000 |    |  |  |      |
| 57- | GRID  | 25 | 1.000      | 1.000  | 0.000 |    |  |  |      |
| 58- | MAT8  | 1  | 20.0E+06   | .50E+6 | .25   |    |  |  |      |
| 59- | FCOMP | 1  | .001       |        |       |    |  |  |      |
| 60- | +PC1  | 1  | .000666    | 90.0   | YES   | 1  |  |  | +PC1 |
| 61- | +PC2  | 1  | .000666    | 90.0   | YES   | 1  |  |  | +PC2 |
| 62- | FLOAD | 1  | -1.0E-0417 | 21     | YES   | 22 |  |  |      |
| 63- | FLOAD | 1  | -1.0E-0419 | 24     | YES   | 20 |  |  |      |
| 64- | FLOAD | 1  | -1.0E-043  | 7      | YES   | 9  |  |  |      |
| 65- | FLOAD | 1  | -1.0E-042  | 7      | YES   | 5  |  |  |      |
| 66- | FLOAD | 1  | -1.0E-0410 | 14     | YES   | 15 |  |  |      |
| 67- | FLOAD | 1  | -1.0E-0414 | 19     | YES   | 15 |  |  |      |
| 68- | FLOAD | 1  | -1.0E-0415 | 19     | YES   | 20 |  |  |      |
| 69- | FLOAD | 1  | -1.0E-0420 | 24     | YES   | 25 |  |  |      |
| 70- | FLOAD | 1  | -1.0E-0413 | 18     | YES   | 14 |  |  |      |
| 71- | FLOAD | 1  | -1.0E-0414 | 18     | YES   | 19 |  |  |      |
| 72- | FLOAD | 1  | -1.0E-0418 | 23     | YES   | 19 |  |  |      |
| 73- | FLOAD | 1  | -1.0E-049  | 13     | YES   | 14 |  |  |      |
| 74- | FLOAD | 1  | -1.0E-0419 | 23     | YES   | 24 |  |  |      |

# PROBLEM 9 REGULAR SYMMETRIC CROSS-PLY LAMINATE MODELED WITH TRIA3 ELEMENTS

## COSMIC INPUT (CONTD)

|      |   |            |      |    |
|------|---|------------|------|----|
| 75-  | 1 | -1.0E-044  | 9    | 9  |
| 76-  | 1 | -1.0E-045  | 9    | 10 |
| 77-  | 1 | -1.0E-049  | 14   | 10 |
| 78-  | 1 | -1.0E-0411 | 16   | 12 |
| 79-  | 1 | -1.0E-0413 | 17   | 18 |
| 80-  | 1 | -1.0E-046  | 11   | 7  |
| 81-  | 1 | -1.0E-047  | 11   | 12 |
| 82-  | 1 | -1.0E-0417 | 22   | 18 |
| 83-  | 1 | -1.0E-048  | 13   | 9  |
| 84-  | 1 | -1.0E-0418 | 22   | 23 |
| 85-  | 1 | -1.0E-047  | 12   | 8  |
| 86-  | 1 | -1.0E-0416 | 21   | 7  |
| 87-  | 1 | -1.0E-048  | 12   | 13 |
| 88-  | 1 | -1.0E-0412 | 16   | 17 |
| 89-  | 1 | -1.0E-0412 | 17   | 13 |
| 90-  | 1 | -1.0E-044  | 9    | 9  |
| 91-  | 1 | -1.0E-042  | 6    | 7  |
| 92-  | 1 | -1.0E-043  | 8    | 4  |
| 93-  | 1 | -1.0E-041  | 6    | 2  |
| 94-  | 1 | 6          | THRU | 25 |
| 95-  | 1 | 15         | 23   | 24 |
| 96-  | 1 | 24         | 15   | 20 |
| 97-  | 1 | 1234       | 3    | 4  |
| 98-  | 1 | 1235       | 11   | 16 |
| 99-  | 1 | 1245       |      | 21 |
| 100- | 1 | 12345      |      |    |
|      |   | ENCDATA    |      |    |

# PROBLEM 3 REGULAR SYMMETRIC CROSS-PLY LAMINATE MODELED WITH TRIA3 ELEMENTS

## COSMIC OUTPUT

| POINT ID. | TYPE | T1  | T2  | T3            | DISPLACEMENT VECTOR |               |              | R1           | R2  | R3  |
|-----------|------|-----|-----|---------------|---------------------|---------------|--------------|--------------|-----|-----|
| 1         | G    | 0.0 | 0.0 | 0.0           | 0.0                 | 0.0           | 0.0          | 0.0          | 0.0 | 0.0 |
| 2         | G    | 0.0 | 0.0 | 0.0           | 0.0                 | 0.0           | 0.0          | 1.174108E-03 | 0.0 | 0.0 |
| 3         | G    | 0.0 | 0.0 | 0.0           | 0.0                 | 0.0           | 0.0          | 2.080508E-03 | 0.0 | 0.0 |
| 4         | G    | 0.0 | 0.0 | 0.0           | 0.0                 | 0.0           | 0.0          | 2.331759E-03 | 0.0 | 0.0 |
| 5         | G    | 0.0 | 0.0 | 0.0           | 0.0                 | 0.0           | 0.0          | 2.112162E-03 | 0.0 | 0.0 |
| 6         | G    | 0.0 | 0.0 | 0.0           | 0.0                 | -1.205491E-03 | 0.0          | 0.0          | 0.0 | 0.0 |
| 7         | G    | 0.0 | 0.0 | -2.635227E-04 | -1.039763E-03       | -1.039763E-03 | 1.355696E-03 | 0.0          | 0.0 | 0.0 |
| 8         | G    | 0.0 | 0.0 | -4.983343E-04 | -6.172197E-04       | -6.172197E-04 | 1.355620E-03 | 0.0          | 0.0 | 0.0 |
| 9         | G    | 0.0 | 0.0 | -6.108230E-04 | -2.823236E-04       | -2.823236E-04 | 2.317570E-03 | 0.0          | 0.0 | 0.0 |
| 10        | C    | 0.0 | 0.0 | -6.506284E-04 | 0.0                 | 0.0           | 2.33258E-03  | 0.0          | 0.0 | 0.0 |
| 11        | G    | 0.0 | 0.0 | 0.0           | -2.270027E-03       | -2.270027E-03 | 0.0          | 0.0          | 0.0 | 0.0 |
| 12        | G    | 0.0 | 0.0 | -5.257187E-04 | -1.911599E-03       | -1.911599E-03 | 2.29751E-04  | 0.0          | 0.0 | 0.0 |
| 13        | G    | 0.0 | 0.0 | -9.090877E-04 | -1.163293E-03       | -1.163293E-03 | 2.18127E-03  | 0.0          | 0.0 | 0.0 |
| 14        | G    | 0.0 | 0.0 | -1.113404E-03 | -5.145338E-04       | -5.145338E-04 | 1.780501E-03 | 0.0          | 0.0 | 0.0 |
| 15        | G    | 0.0 | 0.0 | -1.197048E-03 | 0.0                 | 0.0           | 1.899247E-03 | 0.0          | 0.0 | 0.0 |
| 16        | G    | 0.0 | 0.0 | 0.0           | -3.017362E-03       | -3.017362E-03 | 0.0          | 0.0          | 0.0 | 0.0 |
| 17        | G    | 0.0 | 0.0 | -6.307842E-04 | -2.467892E-03       | -2.467892E-03 | 4.628815E-04 | 0.0          | 0.0 | 0.0 |
| 18        | G    | 0.0 | 0.0 | -1.183425E-03 | -1.520105E-03       | -1.520105E-03 | 7.172556E-04 | 0.0          | 0.0 | 0.0 |
| 19        | G    | 0.0 | 0.0 | -1.453424E-03 | -6.558617E-04       | -6.558617E-04 | 5.274826E-04 | 0.0          | 0.0 | 0.0 |
| 20        | G    | 0.0 | 0.0 | -1.556301E-03 | 0.0                 | 0.0           | 5.334608E-04 | 0.0          | 0.0 | 0.0 |
| 21        | G    | 0.0 | 0.0 | 0.0           | -3.372215E-03       | -3.372215E-03 | 0.0          | 0.0          | 0.0 | 0.0 |
| 22        | G    | 0.0 | 0.0 | -7.507661E-04 | -2.62528E-03        | -2.62528E-03  | 0.0          | 0.0          | 0.0 | 0.0 |
| 23        | G    | 0.0 | 0.0 | -1.281222E-03 | -1.624640E-03       | -1.624640E-03 | 0.0          | 0.0          | 0.0 | 0.0 |
| 24        | G    | 0.0 | 0.0 | -1.570615E-03 | -6.878410E-04       | -6.878410E-04 | 0.0          | 0.0          | 0.0 | 0.0 |
| 25        | G    | 0.0 | 0.0 | -1.653666E-03 | 0.0                 | 0.0           | 0.0          | 0.0          | 0.0 | 0.0 |

PROBLEM #12

COMPOSITE SHELL ROOF

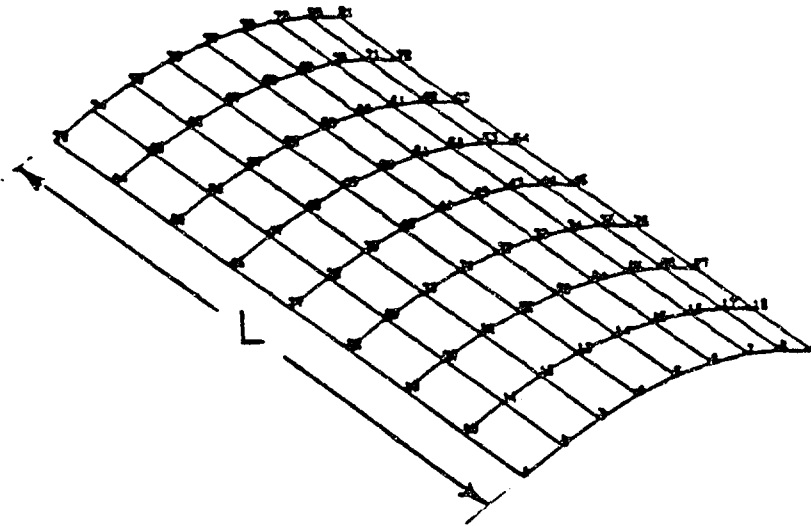


FIGURE 12

A finite element model of a composite shell roof modeled with the symmetric lay-up  $[45, -45, 15, -15, -15, 15, -45, 45]$  is shown in Fig. 12. The length and radius of the shell are 25, and the thickness of each layer is .03125. The material properties are given as  $E_1=2.0 \times 10^6$ ,  $E_2=0.5 \times 10^6$ ,  $\nu_{12}=0.25$ ,  $G_{12}=2.5 \times 10^6$ , and  $G_{17}=G_{27}=2.5 \times 10^6$ . The shell is subjected to a uniform pressure of 90.0. Results from MSC/NASTRAN are given in Table 8 for the radial deflection at selected nodes.

| GRID | T1      |
|------|---------|
| 34   | -1.0662 |
| 35   | -1.3441 |
| 36   | -1.6074 |
| 43   | -1.3267 |
| 44   | -1.6739 |
| 45   | -2.0079 |

TABLE 8

# PROBLEM 12 COMPOSITE SHELL ROOF - CTRIA3 ELEMENTS

## COSMIC OUTPUT

### DISPLACEMENT VECTOR

| POINT ID. | TYPE | T1            | T2             | T3            | R1           | R2            | R3            |
|-----------|------|---------------|----------------|---------------|--------------|---------------|---------------|
| 1         | G    | 0.0           | 0.0            | 1.476633E-02  | 0.0          | 1.361570E-02  | 0.0           |
| 2         | G    | 0.0           | 0.0            | 1.612430E-02  | 3.259866E-02 | 7.457397E-03  | 1.813537E-03  |
| 3         | G    | 0.0           | 0.0            | 1.942165E-02  | 1.313583E-01 | -7.714272E-03 | -5.444880E-01 |
| 4         | G    | 0.0           | 0.0            | 2.215707E-02  | 1.976693E-01 | -2.931339E-02 | -1.551425E-03 |
| 5         | G    | 0.0           | 0.0            | 2.054040E-02  | 2.512435E-01 | -5.650782E-02 | -2.533445E-03 |
| 6         | G    | 0.0           | 0.0            | 9.672505E-03  | 2.694153E-01 | -6.757029E-02 | -3.402307E-03 |
| 7         | G    | 0.0           | 0.0            | -1.624594E-02 | 3.007707E-01 | -1.200357E-01 | -4.131101E-03 |
| 8         | G    | 0.0           | 0.0            | -6.365565E-02 | 2.776152E-01 | -1.504282E-01 | -4.490627E-03 |
| 9         | G    | 0.0           | 0.0            | -1.393812E-01 | 0.0          | -1.643894E-01 | -5.448329E-03 |
| 10        | G    | 0.0           | 0.0            | 1.470382E-02  | 0.0          | 1.151773E-02  | 0.0           |
| 11        | G    | 2.326459E-02  | 3.264060E-03   | 1.605109E-02  | 8.350852E-02 | 8.155208E-03  | 1.493468E-02  |
| 12        | G    | -1.482768E-02 | -3.972457E-03  | 1.922503E-02  | 1.259123E-01 | -5.081122E-03 | 2.591116E-02  |
| 13        | G    | -7.95509E-02  | 4.956841E-05   | 2.184063E-02  | 1.634297E-01 | -2.528226E-02 | 3.501230E-02  |
| 14        | G    | -1.617397E-01 | 1.051910E-02   | 2.020436E-02  | 1.832148E-01 | -5.096502E-02 | 4.241037E-02  |
| 15        | G    | -2.545453E-01 | 2.873136E-02   | 9.543329E-03  | 1.904393E-01 | -9.039026E-02 | 4.734479E-02  |
| 16        | G    | -3.567530E-01 | 5.541377E-02   | -1.533482E-02 | 1.797236E-01 | -1.114484E-01 | 4.872809E-02  |
| 17        | G    | -4.531212E-01 | 9.051105E-02   | -6.169981E-02 | 1.563861E-01 | -1.420324E-01 | 4.565815E-02  |
| 18        | G    | -5.343321E-01 | 1.378572E-01   | -1.344102E-01 | 0.0          | -1.650616E-01 | 3.715051E-02  |
| 19        | G    | 8.354098E-02  | 0.0            | 1.460465E-02  | 0.0          | 8.452272E-03  | 0.0           |
| 20        | G    | 5.485842E-02  | -6.135858E-03  | 1.576311E-02  | 8.552643E-02 | 6.857764E-03  | 2.687922E-02  |
| 21        | G    | -2.71054E-02  | -7.1536237E-03 | 1.850582E-02  | 1.249557E-01 | -4.624485E-03 | 4.874576E-02  |
| 22        | G    | -1.524426E-01 | 4.739278E-04   | 2.065561E-02  | 1.543775E-01 | -2.283172E-02 | 6.676597E-02  |
| 23        | G    | -3.120429E-01 | 2.086395E-02   | 1.880291E-02  | 1.765106E-01 | -4.615085E-02 | 8.686339E-02  |
| 24        | G    | -4.952009E-01 | 5.623504E-02   | 8.573793E-03  | 1.721470E-01 | -7.276709E-02 | 9.305839E-02  |
| 25        | G    | -6.886350E-01 | 1.078973E-01   | -1.504325E-02 | 1.632250E-01 | -1.009693E-01 | 9.333315E-02  |
| 26        | G    | -8.776217E-01 | 1.757794E-01   | -5.733753E-02 | 1.534643E-01 | -1.296031E-01 | 9.074838E-02  |
| 27        | G    | -1.042164E+00 | 2.584161E-01   | -1.236248E-01 | 0.0          | -1.518993E-01 | 8.408472E-02  |
| 28        | G    | 1.165276E-01  | 0.0            | 1.401622E-02  | 0.0          | 5.516557E-03  | 0.0           |
| 29        | G    | 7.638276E-02  | -8.429410E-03  | 1.489281E-02  | 9.887142E-02 | 5.185639E-03  | 3.709026E-02  |
| 30        | G    | -3.927651E-02 | -9.976234E-03  | 1.701572E-02  | 1.290838E-01 | -4.660827E-03 | 6.886443E-02  |
| 31        | G    | -2.181162E-01 | 1.456814E-03   | 1.851162E-02  | 1.488879E-01 | -2.028534E-02 | 9.518006E-02  |
| 32        | G    | -4.443317E-01 | 3.076088E-02   | 1.643791E-02  | 1.590755E-01 | -4.046500E-02 | 1.153411E-01  |
| 33        | G    | -7.071198E-01 | 8.141620E-02   | 7.045491E-03  | 1.533746E-01 | -6.342431E-02 | 1.283439E-01  |
| 34        | G    | -9.838973E-01 | 1.552550E-01   | -1.391042E-02 | 1.444555E-01 | -3.753313E-02 | 1.336524E-01  |
| 35        | G    | -1.256662E+00 | 2.523485E-01   | -5.089922E-02 | 1.320263E-01 | -1.130087E-01 | 1.322963E-01  |
| 36        | G    | -1.512646E+00 | 3.712141E-01   | -1.084606E-01 | 0.0          | -1.328700E-01 | 1.278532E-01  |
| 37        | G    | 1.416514E-01  | 0.0            | 1.214414E-02  | 0.0          | 2.540008E-03  | 0.0           |
| 38        | G    | 9.209161E-02  | -1.039582E-02  | 1.328759E-02  | 1.049389E-01 | 3.458103E-03  | 4.545156E-02  |
| 39        | G    | -5.137531E-02 | -1.167172E-02  | 1.472694E-02  | 1.227285E-01 | -4.346098E-03 | 8.551660E-02  |
| 40        | G    | -2.747474E-01 | 2.839282E-03   | 1.556273E-02  | 1.562093E-01 | -1.685280E-02 | 1.189154E-01  |
| 41        | G    | -5.604917E-01 | 3.978024E-02   | 1.343068E-02  | 1.344321E-01 | -3.318006E-02 | 1.443206E-01  |

# PROBLEM 12 COMPOSITE SHELL ROOF - CTRIA3 ELEMENTS

## COSMIC OUTPUT (CONTD)

|      |   |               |               |               |               |               |              |
|------|---|---------------|---------------|---------------|---------------|---------------|--------------|
| 42   | G | -8.878149E-01 | 1.033431E-01  | 5.308650E-03  | 1.329748E-01  | -5.205030E-02 | 1.606955E-01 |
| 43   | G | -1.234267E+00 | 1.959281E-01  | -1.222983E-02 | 1.214679E-01  | -7.233550E-02 | 1.680550E-01 |
| 44   | G | -1.579237E+00 | 3.178312E-01  | -4.279044E-02 | 1.141284E-01  | -9.345143E-02 | 1.583716E-01 |
| 45   | G | -1.909306E+00 | 4.676751E-01  | -9.003448E-02 | 0.0           | -1.103693E-01 | 1.663368E-01 |
| 46   | G | 1.585377E-01  | 0.0           | 1.970312E-02  | 0.0           | -3.328194E-04 | 0.0          |
| 47   | G | 1.020915E-01  | -1.115413E-02 | 1.092877E-02  | 9.868991E-02  | 1.837580E-03  | 5.151105E-02 |
| 48   | G | -6.221560E-02 | 1.256683E-02  | 1.171812E-02  | 1.181452E-01  | -3.483323E-03 | 9.817950E-02 |
| 49   | G | -3.200940E-01 | 4.571111E-03  | 1.201176E-02  | 1.233944E-01  | 1.296991E-02  | 1.375043E-01 |
| 50   | G | -6.512789E-01 | 4.746799E-02  | 1.006675E-02  | 1.199054E-01  | -2.532743E-02 | 1.673537E-01 |
| 51   | G | -1.031257E+00 | 1.212685E-01  | 3.596504E-03  | 1.095177E-01  | -3.989144E-02 | 1.866436E-01 |
| 52   | G | -1.434333E+00 | 2.287851E-01  | -9.991137E-03 | 9.726910E-02  | -5.572497E-02 | 1.959045E-01 |
| 53   | G | -1.838202E+00 | 3.705621E-01  | -3.339332E-02 | 8.642013E-02  | -7.228899E-02 | 1.979190E-01 |
| 54   | G | -2.229472E+00 | 5.453926E-01  | -6.932452E-02 | 0.0           | -8.547089E-02 | 1.961847E-01 |
| 55   | G | 1.668758E-01  | 0.0           | 7.928718E-03  | 0.0           | -1.660479E-03 | 0.0          |
| 56   | G | 1.080008E-01  | -1.169142E-02 | 7.812442E-03  | 1.191570E-01  | 1.410222E-03  | 5.540280E-02 |
| 57   | G | -7.047822E-02 | 1.286581E-02  | 8.058028E-03  | 1.048704E-01  | -2.586255E-03 | 1.070393E-01 |
| 58   | G | -3.529197E-01 | 6.135389E-03  | 8.046384E-03  | 1.046178E-01  | -8.673640E-03 | 1.509624E-01 |
| 59   | G | -7.173166E-01 | 5.337095E-02  | 6.576133E-03  | 9.723432E-02  | -1.716859E-02 | 1.842807E-01 |
| 60   | G | -1.136189E+00 | 1.346256E-01  | 2.061663E-03  | 8.429071E-02  | -2.724381E-02 | 2.058199E-01 |
| 61   | G | -1.551390E+00 | 2.530920E-01  | -7.243007E-03 | 7.151520E-02  | -3.817935E-02 | 2.166686E-01 |
| 62   | G | -2.029629E+00 | 4.095345E-01  | -2.309458E-02 | 7.429327E-02  | -5.004312E-02 | 2.203675E-01 |
| 63   | G | -2.467970E+00 | 6.029455E-01  | -4.722725E-02 | 0.0           | -6.007985E-02 | 2.227727E-01 |
| 64   | G | 1.701813E-01  | 0.0           | 4.341663E-03  | 0.0           | -6.951285E-03 | 0.0          |
| 65   | G | 1.092746E-01  | -1.175825E-02 | 4.012782E-03  | 5.440501E-02  | -1.511942E-03 | 5.643111E-02 |
| 66   | G | -7.531671E-02 | 1.281390E-02  | 4.029745E-03  | 1.019095E-01  | -9.911236E-04 | 1.113292E-01 |
| 67   | G | -3.725972E-01 | 7.206568E-03  | 3.976561E-03  | 9.881937E-02  | -4.479322E-03 | 1.592970E-01 |
| 68   | G | -7.591181E-01 | 5.706112E-02  | 3.171894E-03  | 8.436854E-02  | -1.003476E-02 | 1.954991E-01 |
| 69   | G | -1.204372E+00 | 1.429999E-01  | 7.929730E-04  | 6.339596E-02  | -1.680591E-02 | 2.186134E-01 |
| 70   | G | -1.677765E+00 | 2.684475E-01  | -4.026215E-03 | 4.313026E-02  | -2.403195E-02 | 2.302631E-01 |
| 71   | G | -2.155000E+00 | 4.342831E-01  | -1.209223E-02 | 1.261471E-02  | -3.079130E-02 | 2.346718E-01 |
| 72   | G | -2.624267E+00 | 6.396778E-01  | -2.407040E-02 | 0.0           | -3.432402E-02 | 2.395364E-01 |
| 73   | G | 1.669936E-01  | 0.0           | 0.0           | -6.855840E-02 | 0.0           | 0.0          |
| 74   | G | 1.072161E-01  | -1.189289E-02 | 0.0           | 1.943815E-01  | 0.0           | 5.693102E-02 |
| 75   | G | -7.771776E-02 | 1.293488E-02  | 0.0           | 1.544966E-01  | 0.0           | 1.154741E-01 |
| 76   | G | -3.814610E-01 | 7.504007E-03  | 0.0           | 1.241305E-01  | 0.0           | 1.650839E-01 |
| 77   | G | -7.779930E-01 | 5.849575E-02  | 0.0           | 8.962283E-02  | 0.0           | 2.022918E-01 |
| 78   | G | -1.235319E+00 | 1.464317E-01  | 0.0           | 5.829034E-02  | 0.0           | 2.257639E-01 |
| 79   | G | -1.721251E+00 | 2.747352E-01  | 0.0           | 4.354954E-02  | 0.0           | 2.374775E-01 |
| 80   | G | -2.211148E+00 | 4.441634E-01  | 0.0           | 9.365131E-02  | 0.0           | 2.441724E-01 |
| 81   | G | -2.697874E+00 | 6.38941E-01   | 0.0           | -3.394089E-01 | 0.0           | 2.429941E-01 |
| 5001 |   | 0.0           | 0.0           | 0.0           | 0.0           | 0.0           | 0.0          |



# PROBLEM 12 COMPOSITE SHELL ROOF - CTRIA3 ELEMENTS

## MSC OUTPUT

### DISPLACEMENT VECTOR

| POINT ID. | TYPE | T1            | T2            | T3            | R1           | R2            | R3            |
|-----------|------|---------------|---------------|---------------|--------------|---------------|---------------|
| 1         | G    | 0.0           | 0.0           | 1.488159E-02  | 0.0          | 1.297565E-02  | 0.0           |
| 2         | G    | 0.0           | 0.0           | 1.623873E-02  | 3.260004E-02 | 6.302241E-03  | 1.105459E-03  |
| 3         | G    | 0.0           | 0.0           | 1.949219E-02  | 1.259453E-01 | -8.333838E-03 | -7.068854E-04 |
| 4         | G    | 0.0           | 0.0           | 2.214893E-02  | 1.881285E-01 | -2.981238E-02 | -1.720419E-03 |
| 5         | G    | 0.0           | 0.0           | 2.044334E-02  | 2.376534E-01 | -5.672706E-02 | -2.624311E-03 |
| 6         | G    | 0.0           | 0.0           | 5.512377E-03  | 2.705136E-01 | -8.735808E-02 | -3.386547E-03 |
| 7         | G    | 0.0           | 0.0           | -1.639316E-02 | 2.789256E-01 | -1.191964E-01 | -3.942670E-03 |
| 8         | G    | 0.0           | 0.0           | -6.364493E-02 | 2.570321E-01 | -1.489690E-01 | -4.086468E-03 |
| 9         | G    | 0.0           | 0.0           | -1.389790E-01 | 0.0          | -1.634667E-01 | -5.355548E-03 |
| 10        | G    | 4.370032E-02  | 0.0           | 1.482224E-02  | 0.0          | 1.071695E-02  | 0.0           |
| 11        | G    | 2.754780E-02  | -3.231469E-03 | 1.615973E-02  | 7.232695E-02 | 7.423943E-03  | 1.472248E-02  |
| 12        | G    | -1.543843E-02 | -3.879277E-03 | 1.929014E-02  | 1.160193E-01 | -5.489751E-03 | 2.558949E-02  |
| 13        | G    | -7.970110E-02 | 1.803154E-04  | 2.183047E-02  | 1.517032E-01 | -2.534725E-02 | 3.470048E-02  |
| 14        | G    | -1.612722E-01 | 1.063959E-02  | 2.010849E-02  | 1.739005E-01 | -5.067214E-02 | 4.208433E-02  |
| 15        | G    | -2.551457E-01 | 2.877427E-02  | 9.386890E-03  | 1.803120E-01 | -7.968822E-02 | 4.636886E-02  |
| 16        | G    | -3.542117E-01 | 5.529228E-02  | -1.587797E-02 | 1.691137E-01 | -1.102678E-01 | 4.829599E-02  |
| 17        | G    | -4.492348E-01 | 9.010967E-02  | -6.169318E-02 | 1.497871E-01 | -1.404172E-01 | 4.528507E-02  |
| 18        | G    | -5.294844E-01 | 1.320778E-01  | -1.340410E-01 | 0.0          | -1.630579E-01 | 3.724567E-02  |
| 19        | G    | 8.300618E-02  | 0.0           | 1.472343E-02  | 0.0          | 7.897929E-03  | 0.0           |
| 20        | G    | 5.366356E-02  | -6.061701E-03 | 1.586366E-02  | 7.629420E-02 | 6.358043E-03  | 2.677065E-02  |
| 21        | G    | -2.822683E-02 | -7.221833E-03 | 1.855773E-02  | 1.131810E-01 | -4.757439E-02 | 4.851977E-02  |
| 22        | G    | -1.528755E-01 | 7.142373E-04  | 2.063585E-02  | 1.418970E-01 | -2.264605E-02 | 6.542760E-02  |
| 23        | G    | -5.112799E-01 | 2.108605E-02  | 1.870670E-02  | 1.575670E-01 | -4.568077E-02 | 8.038319E-02  |
| 24        | G    | -4.927777E-01 | 5.631252E-02  | 8.427706E-03  | 1.589888E-01 | -7.203154E-02 | 8.939063E-02  |
| 25        | G    | -6.840820E-01 | 1.076638E-01  | -1.516993E-02 | 1.496903E-01 | -9.991183E-02 | 9.246780E-02  |
| 26        | G    | -8.704751E-01 | 1.750302E-01  | -5.732376E-02 | 1.383166E-01 | -1.280042E-01 | 8.974052E-02  |
| 27        | G    | -1.039088E+00 | 2.589164E-01  | -1.232866E-01 | 0.0          | -1.501012E-01 | 8.310465E-02  |
| 28        | G    | 1.152777E-01  | 0.0           | 1.413831E-02  | 0.0          | 5.148641E-03  | 0.0           |
| 29        | G    | 7.472901E-02  | -8.300558E-03 | 1.498932E-02  | 8.993725E-02 | 4.714099E-03  | 3.693191E-02  |
| 30        | G    | -4.067949E-02 | -9.711580E-03 | 1.706040E-02  | 1.168659E-01 | -4.884850E-03 | 6.547274E-02  |
| 31        | G    | -2.185333E-01 | 1.802567E-03  | 1.848873E-02  | 1.389650E-01 | -2.031904E-02 | 9.453129E-02  |
| 32        | G    | -4.450763E-01 | 3.107015E-02  | 1.634832E-02  | 1.457215E-01 | -4.305970E-02 | 1.144156E-01  |
| 33        | G    | -7.041574E-01 | 8.151313E-02  | 6.915911E-03  | 1.424343E-01 | -6.306600E-02 | 1.271481E-01  |
| 34        | G    | -9.774102E-01 | 1.549098E-01  | -1.401832E-02 | 1.303859E-01 | -8.721155E-02 | 1.322146E-01  |
| 35        | G    | -1.246591E+00 | 2.512781E-01  | -5.088077E-02 | 1.166845E-01 | -1.116734E-01 | 1.306890E-01  |
| 36        | G    | -1.496247E+00 | 3.690781E-01  | -1.081577E-01 | 0.0          | -1.313339E-01 | 1.261592E-01  |
| 37        | G    | 1.395788E-01  | 0.0           | 1.285421E-02  | 0.0          | 2.296834E-03  | 0.0           |
| 38        | G    | 8.994594E-02  | -9.909747E-03 | 1.337338E-02  | 9.763631E-02 | 3.088094E-03  | 4.519776E-02  |
| 39        | G    | -5.298783E-02 | -1.131881E-02 | 1.476189E-02  | 1.140346E-01 | -4.611195E-03 | 8.497571E-02  |
| 40        | G    | -2.751050E-01 | 3.339947E-03  | 1.553642E-02  | 1.237372E-01 | -1.706274E-02 | 1.180168E-01  |
| 41        | G    | -5.587978E-01 | 4.016229E-02  | 1.334898E-02  | 1.281200E-01 | -3.329806E-02 | 1.430467E-01  |
| 42        | G    | -8.832884E-01 | 1.034509E-01  | 5.196754E-03  | 1.209811E-01 | -5.198572E-02 | 1.590799E-01  |
| 43        | G    | -1.226153E+00 | 1.934803E-01  | -1.231892E-02 | 1.083166E-01 | -7.191026E-02 | 1.661673E-01  |

# PROBLEM 12 COMPOSITE SHELL ROOF - CTRIA3 ELEMENTS

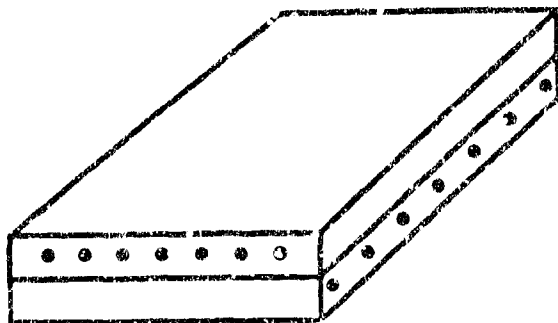
## MSC OUTPUT (CONTD)

|      |   |               |               |               |               |               |              |
|------|---|---------------|---------------|---------------|---------------|---------------|--------------|
| 44   | G | -1.566740E+00 | 3.164809E-01  | -4.276921E-02 | 9.131392E-02  | -9.236200E-02 | 1.662851E-01 |
| 45   | G | -1.891454E+00 | 4.650046E-01  | -8.977510E-02 | 0.0           | -1.090747E-01 | 1.641039E-01 |
| 46   | G | 1.557078E-01  | 0.0           | 1.079951E-02  | 0.0           | -3.798279E-04 | 0.0          |
| 47   | G | 9.953328E-02  | -1.031489E-02 | 1.099804E-02  | 9.500884E-02  | 1.645601E-03  | 5.121743E-02 |
| 48   | G | -6.400058E-02 | -1.213516E-02 | 1.174218E-02  | 1.101552E-01  | -3.827984E-03 | 9.746330E-02 |
| 49   | G | -3.203450E-01 | 5.092703E-03  | 1.198581E-02  | 1.145105E-01  | -1.322338E-02 | 1.363789E-01 |
| 50   | G | -6.491557E-01 | 4.790815E-02  | 9.936753E-03  | 1.098994E-01  | -2.567515E-02 | 1.657968E-01 |
| 51   | G | -1.025912E+00 | 1.213806E-01  | 3.505278E-03  | 9.835787E-02  | -4.009001E-02 | 1.847004E-01 |
| 52   | G | -1.424911E+00 | 2.282475E-01  | -1.005018E-02 | 8.471708E-02  | -5.155328E-02 | 1.936597E-01 |
| 53   | G | -1.823768E+00 | 3.689791E-01  | -3.337160E-02 | 7.237545E-02  | -7.143791E-02 | 1.954772E-01 |
| 54   | G | -2.208994E+00 | 5.422893E-01  | -6.911366E-02 | 0.0           | -8.449339E-02 | 1.955358E-01 |
| 55   | G | 1.651378E-01  | 0.0           | 8.004886E-03  | 0.0           | -1.534491E-03 | 0.0          |
| 56   | G | 1.050687E-01  | -1.140391E-02 | 7.865760E-03  | 1.161919E-01  | 1.292911E-03  | 5.495435E-02 |
| 57   | G | -7.231621E-02 | -1.237000E-02 | 8.075285E-03  | 1.011229E-01  | -2.874816E-03 | 1.062388E-01 |
| 58   | G | -3.530848E-01 | 6.19551E-03   | 8.021737E-03  | 9.764549E-02  | -9.239995E-03 | 1.496353E-01 |
| 59   | G | -7.149160E-01 | 5.385436E-02  | 6.520945E-03  | 8.794370E-02  | -1.782336E-02 | 1.824604E-01 |
| 60   | G | -1.130271E+00 | 1.347380E-01  | 1.996592E-03  | 7.388631E-02  | -2.777240E-02 | 2.035848E-01 |
| 61   | G | -1.571003E+00 | 2.24819E-01   | -7.286683E-03 | 5.962552E-02  | -3.835132E-02 | 2.141181E-01 |
| 62   | G | -2.013773E+00 | 4.077701E-01  | -2.306932E-02 | 5.969225E-02  | -4.963985E-02 | 2.175228E-01 |
| 63   | G | -2.445431E+00 | 5.995078E-01  | -4.706422E-02 | 0.0           | -5.927353E-02 | 2.197395E-01 |
| 64   | G | 1.658712E-01  | 0.0           | 4.412975E-03  | 0.0           | -5.981099E-03 | 0.0          |
| 65   | G | 1.067296E-01  | -1.143024E-02 | 4.039316E-03  | 7.359419E-02  | -2.180564E-04 | 5.660340E-02 |
| 66   | G | -7.722478E-02 | -1.227560E-02 | 4.021035E-03  | 1.047540E-01  | -1.248734E-03 | 1.105871E-01 |
| 67   | G | -3.726837E-01 | 7.833989E-03  | 3.947961E-03  | 9.417622E-02  | -5.160896E-03 | 1.580029E-01 |
| 68   | G | -7.564764E-01 | 5.757254E-02  | 3.136728E-03  | 7.860163E-02  | -1.075205E-02 | 1.906642E-01 |
| 69   | G | -1.197937E+00 | 1.431109E-01  | 7.645905E-04  | 5.680722E-02  | -1.726504E-02 | 2.163555E-01 |
| 70   | G | -1.666436E+00 | 2.677848E-01  | -4.036724E-03 | 3.941693E-02  | -2.394976E-02 | 2.277070E-01 |
| 71   | G | -2.137921E+00 | 4.323893E-01  | -1.207806E-02 | 1.250840E-02  | -2.978523E-02 | 2.323784E-01 |
| 72   | G | -2.600875E+00 | 6.360321E-01  | -2.400697E-02 | 0.0           | -3.367934E-02 | 2.367507E-01 |
| 73   | G | 1.609050E-01  | 0.0           | 0.0           | -1.104910E-01 | 0.0           | 0.0          |
| 74   | G | 1.037114E-01  | -1.149914E-02 | 0.0           | 2.032132E-01  | 0.0           | 5.624982E-02 |
| 75   | G | -7.952528E-02 | -1.232689E-03 | 0.0           | 1.294580E-01  | 0.0           | 1.141604E-01 |
| 76   | G | -3.810907E-01 | 8.167893E-03  | 0.0           | 1.086516E-01  | 0.0           | 1.633870E-01 |
| 77   | G | -7.748047E-01 | 5.900292E-02  | 0.0           | 7.534310E-02  | 0.0           | 2.004330E-01 |
| 78   | G | -1.228686E+00 | 1.465074E-01  | 0.0           | 4.785991E-02  | 0.0           | 2.238440E-01 |
| 79   | G | -1.710430E+00 | 2.740414E-01  | 0.0           | 2.020216E-02  | 0.0           | 2.351301E-01 |
| 80   | G | -2.194278E+00 | 4.422544E-01  | 0.0           | 8.441722E-02  | 0.0           | 2.411606E-01 |
| 81   | G | -2.673986E+00 | 6.502062E-01  | 0.0           | -2.788148E-01 | 0.0           | 2.413830E-01 |
| 5001 | G | 0.0           | 0.0           | 0.0           | 0.0           | 0.0           | 0.0          |

Appendix F: NASTRAN Input and Output for Problem #15

# PROBLEM #15

## COMPOSITE RECTANGULAR PLATE - UNSYMMETRIC CROSS-PLY LAMINATE



| 3 | 6 | 9 | 12 | 15 |
|---|---|---|----|----|
| 2 | 5 | 8 | 11 | 14 |
| 1 | 4 | 7 | 10 | 13 |

UNSYMMETRIC  
CROSS-PLY LAMINATE

FIGURE A1

One half of a composite rectangular plate modeled with the unsymmetric cross-ply layup ( $0^\circ$ ,  $90^\circ$ ) is shown in Fig. A1. The plate is modeled with 15 nodes and 8 elements. The length of the plate is 5", the width is 1", and the thickness of each layer is .005". The material properties matrix on the MAT2 card is given as  $G_{11} = 1.94 \times 10^7$ ,  $G_{12} = 4.66 \times 10^5$ ,  $G_{22} = 1.33 \times 10^5$  and  $G_{33} = 8.3 \times 10^5$ . The thermal expansion coefficient vector is defined by  $A_1 = -1 \times 10^{-6}$ ,  $A_2 = 15 \times 10^{-6}$  and  $A_{12} = 0$ . A temperature field was defined for the plate by an average temperature of  $230^\circ$  over the cross-section of each element on the TEMPP1 card. The input data, the calculated thermal loading, and the displacements due to the thermal loading are also provided.

1. PLATE, SANDHU  
 APP DISP  
 SOL  
 TIME 5  
 CEND

# PROBLEM 15

2. TEST PLATE FOR THERMAL LOAD FOR SANDHU

APRIL 6, 1992 CSA/NASTRAN 3/31/91

CASE : CONTROL DECK ECHO

CARD

COUNT

1. TEST PLATE FOR THERMAL LOAD FOR SANDHU

2. PROPERTIES = ALL  
 3. CLOUT = ALL  
 4. DISPERINT, PUNCH) = ALL  
 5. STRESS = ALL  
 6. SUBCASE 1  
 7. TEMPERATURE = 100  
 8. BEGIN BULK  
 9. TOTAL INPUT BULK DATA

CARD COUNT BETWEEN BEGIN BULK AND ENDDATA = 31  
 TOTAL NUMBER OF BULK DATA CARDS PROCESSED = 32

3. TEST PLATE FOR THERMAL LOAD FOR SANDHU

APRIL 6, 1992 CSA/NASTRAN 3/31/91

SORTED BULK DATA ECHO

CARD

COUNT

| CARD                                       | COUNT | 1     | 2  | 3    | 4   | 5   | 6   | 7 | 8 | 9 | 10 |
|--|-------|-------|----|------|-----|-----|-----|---|---|---|----|
| 1. TEST PLATE FOR THERMAL LOAD FOR SANDHU  | 1     | 10001 | 1  | 4    | 5   | 2   | 0.0 |   |   |   |    |
| 2. PROPERTIES = ALL                        | 2     | 10001 | 2  | 5    | 6   | 3   | 0.0 |   |   |   |    |
| 3. CLOUT = ALL                             | 3     | 10001 | 4  | 7    | 8   | 5   | 0.0 |   |   |   |    |
| 4. DISPERINT, PUNCH) = ALL                 | 4     | 10001 | 5  | 6    | 9   | 6   | 0.0 |   |   |   |    |
| 5. STRESS = ALL                            | 5     | 10001 | 7  | 10   | 11  | 8   | 0.0 |   |   |   |    |
| 6. SUBCASE 1                               | 6     | 10001 | 8  | 11   | 12  | 9   | 0.0 |   |   |   |    |
| 7. TEMPERATURE = 100                       | 7     | 10001 | 10 | 13   | 14  | 11  | 0.0 |   |   |   |    |
| 8. BEGIN BULK                              | 8     | 10001 | 11 | 14   | 15  | 12  | 0.0 |   |   |   |    |
| 9. TOTAL INPUT BULK DATA                   | 9     | 10001 | 11 | 14   | 15  | 12  | 0.0 |   |   |   |    |
| 10. TEST PLATE FOR THERMAL LOAD FOR SANDHU | 10    | GRID  | 1  | 0.0  | 0.0 | 0.0 |     |   |   |   |    |
| 11. PROPERTIES = ALL                       | 11    | GRID  | 2  | 0.0  | 0.0 | 0.0 |     |   |   |   |    |
| 12. CLOUT = ALL                            | 12    | GRID  | 3  | 0.0  | 0.0 | 0.0 |     |   |   |   |    |
| 13. DISPERINT, PUNCH) = ALL                | 13    | GRID  | 4  | 1.25 | 0.0 | 0.0 |     |   |   |   |    |
| 14. STRESS = ALL                           | 14    | GRID  | 5  | 1.25 | 0.0 | 0.0 |     |   |   |   |    |
| 15. SUBCASE 1                              | 15    | GRID  | 6  | 1.25 | 1.0 | 0.0 |     |   |   |   |    |
| 16. TEMPERATURE = 100                      | 16    | GRID  | 7  | 2.50 | 0.0 | 0.0 |     |   |   |   |    |
| 17. BEGIN BULK                             | 17    | GRID  | 8  | 2.50 | 0.0 | 0.0 |     |   |   |   |    |
| 18. TOTAL INPUT BULK DATA                  | 18    | GRID  | 9  | 2.50 | 1.0 | 0.0 |     |   |   |   |    |
| 19. TEST PLATE FOR THERMAL LOAD FOR SANDHU | 19    | GRID  | 10 | 3.75 | 0.0 | 0.0 |     |   |   |   |    |
| 20. PROPERTIES = ALL                       | 20    | GRID  | 11 | 3.75 | 0.0 | 0.0 |     |   |   |   |    |
| 21. CLOUT = ALL                            | 21    | GRID  | 12 | 3.75 | 1.0 | 0.0 |     |   |   |   |    |

|     |        |       |        |        |        |       |     |      |
|-----|--------|-------|--------|--------|--------|-------|-----|------|
| 21- | GRID   | 13    | 5.0    | 0.0    | 0.0    |       |     |      |
| 22- | GRID   | 14    | 5.0    | .50    | 0.0    |       |     |      |
| 23- | GRID   | 15    | 5.0    | 1.0    | 0.0    |       |     |      |
| 24- | MAT2   | 30001 | 1.94+7 | 4.66+5 | 1.33+6 | 8.3+5 |     | +DEF |
| 25- | +DEF   | -1.-6 | 15.-6  |        |        |       |     |      |
| 26- | PCOMP1 | 10001 |        | 0.     | 60000. | 30001 | .01 | +ABC |
| 27- | +ABC   | 0.0   | 90.    |        |        |       |     |      |
| 28- | SPC1   | 16    | 6      | 4      | THRU   | 15    |     |      |
| 29- | SPC1   | 16    | 123456 | 1      | THRU   | 3     |     |      |
| 30- | TEMPPI | 100   | 1      | 230.   |        |       |     |      |
| 31- | +TPA   | 2     | THRU   | 8      |        |       |     | +TPA |

ENDDATA

# LOAD VECTOR

| POINT ID. | TYPE | T1            | T2            | T3  | R1            | R2            | R3  |
|-----------|------|---------------|---------------|-----|---------------|---------------|-----|
| 1         | G    | -4.067550E+00 | -1.016887E+01 | 0.0 | -2.292381E-01 | -9.169525E-02 | 0.0 |
| 2         | G    | -8.135150E+00 | 1.705303E-13  | 0.0 | 0.0           | -1.833905E-01 | 0.0 |
| 3         | G    | -4.067550E+00 | 1.016887E+01  | 0.0 | 2.292381E-01  | -9.169525E-02 | 0.0 |
| 4         | G    | 4.547474E-13  | -2.033775E+01 | 0.0 | -4.584762E-01 | 1.776357E-15  | 0.0 |
| 5         | G    | 0.0           | -5.684342E-14 | 0.0 | 0.0           | 4.440892E-16  | 0.0 |
| 6         | G    | -4.547474E-13 | 2.033775E+01  | 0.0 | 4.584762E-01  | -1.776357E-15 | 0.0 |
| 7         | G    | 4.547474E-13  | -2.033775E+01 | 0.0 | -4.584762E-01 | 1.776357E-15  | 0.0 |
| 8         | G    | 0.0           | -5.684342E-14 | 0.0 | 0.0           | 4.440892E-16  | 0.0 |
| 9         | G    | -4.547474E-13 | 2.033775E+01  | 0.0 | 4.584762E-01  | -1.776357E-15 | 0.0 |
| 10        | G    | 4.547474E-13  | -2.033775E+01 | 0.0 | -4.584762E-01 | 1.776357E-15  | 0.0 |
| 11        | G    | 0.0           | -5.684342E-14 | 0.0 | 0.0           | 4.440892E-16  | 0.0 |
| 12        | G    | -4.547474E-13 | 2.033775E+01  | 0.0 | 4.584762E-01  | -1.776357E-15 | 0.0 |
| 13        | G    | 4.067550E+00  | -1.016887E+01 | 0.0 | -2.292381E-01 | 9.169525E-02  | 0.0 |
| 14        | G    | 4.351500E+00  | -2.273737E-13 | 0.0 | 0.0           | 1.833905E-01  | 0.0 |
| 15        | G    | 4.067550E+00  | 1.016887E+01  | 0.0 | 2.292381E-01  | -9.169525E-02 | 0.0 |

# DISPLACEMENT VECTOR

| POINT NO. | T1           | T2            | T3            | R1            | R2           | R3  |
|-----------|--------------|---------------|---------------|---------------|--------------|-----|
| 1         | 0.0          | 0.0           | 0.0           | 0.0           | 0.0          | 0.0 |
| 2         | 0.0          | 0.0           | 0.0           | 0.0           | 0.0          | 0.0 |
| 3         | 0.0          | 0.0           | 0.0           | 0.0           | 0.0          | 0.0 |
| 4         | 8.262617E-04 | -3.794536E-04 | -1.075627E-01 | -8.041327E-02 | 1.728277E-01 | 0.0 |
| 5         | 9.649792E-04 | -1.712008E-12 | -1.275678E-01 | 2.652152E-11  | 2.033811E-01 | 0.0 |
| 6         | 6.282617E-04 | 3.794536E-04  | -1.075627E-01 | 8.041327E-02  | 1.728277E-01 | 0.0 |
| 7         | 1.853320E-03 | -3.837750E-04 | -4.625432E-01 | -8.305494E-02 | 3.947364E-01 | 0.0 |
| 8         | 1.227469E-03 | -7.303463E-12 | -4.633479E-01 | 1.825518E-11  | 3.662719E-01 | 0.0 |
| 9         | 1.853320E-03 | 3.837749E-04  | -4.625432E-01 | 8.305494E-02  | 3.947364E-01 | 0.0 |
| 10        | 2.635011E-03 | -3.380588E-04 | -1.059582E+00 | -7.217816E-02 | 5.607316E-01 | 0.0 |
| 11        | 2.36763E-03  | -1.580492E-11 | -1.077608E+00 | 1.814007E-10  | 5.843386E-01 | 0.0 |
| 12        | 2.635011E-03 | 3.380588E-04  | -1.059582E+00 | 7.217816E-02  | 5.607316E-01 | 0.0 |
| 13        | 3.624009E-03 | -3.779266E-04 | -1.094176E+00 | -7.982263E-02 | 7.745631E-01 | 0.0 |
| 14        | 3.554427E-03 | -3.137500E-11 | -1.914137E+00 | -4.224974E-10 | 7.541634E-01 | 0.0 |
| 15        | 3.624009E-03 | 3.779266E-04  | -1.094176E+00 | 7.982263E-02  | 7.745631E-01 | 0.0 |



